

## Chapter NR 110

### SEWERAGE SYSTEMS

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**History:** Chapter NR 110 as it existed on November 30, 1974 was repealed and a new chapter NR 110 was created effective December 1, 1974.

**NR 110.01 Applicability.** This chapter is applicable to all new or modified sewerage systems, excluding only industrial waste treatment facilities. This chapter also applies to sewerage systems employing land disposal of sewage effluent, except those systems defined as plumbing within the purview of s. 145.01 (10) (a) 2., Stats.

**Note:** The authority to enact these rules is contained in ch. 281, Stats. Pursuant to s. 299.97, Stats., any person who violates this chapter shall forfeit not less than \$10 nor more than \$5,000 for each violation. Each day of continued violation is a separate offense.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; correction was made under s. 13.93 (2m) (b) 7., Stats., Register, May, 2001, No. 545; correction made under s. 13.92 (4) (b) 7., Stats., Register July 2010 No. 655, eff. 8-1-10.

**NR 110.03 Definitions.** For purposes of this chapter:

**(1)** “Approved areawide waste treatment management plan” means a plan or element thereof developed pursuant to Section 208 of the Federal Water Pollution Control Act Amendments of 1972 as amended by the Clean Water Act Amendments of 1977 (33 USC 1251 et seq.) and approved by the state of Wisconsin.

**(2)** “Approval” means the written approval of the department for any project requiring approval pursuant to s. 281.41, Stats., and s. NR 108.03.

**(4)** “ASTM” means standards developed by ASTM International, 100 Barr Harbor Drive, P.O. Box C700, West Conshohocken, PA 19428-2959.

**Note:** Copies of ASTM standards referenced in this chapter are available for inspection at the office of the department of natural resources, the secretary of state’s office, and the legislative reference bureau, and may be obtained for personal use from ASTM International, 100 Barr Harbor Drive, P.O. Box C700, West Conshohocken, PA 19428-2959.

**(6)** “AWWA” means the American Water Works Association, 6666 West Quincy Avenue, Denver, CO 80235.

**Note:** Copies of AWWA standards referenced in this chapter are available for inspection at the offices of the department of natural resources, the secretary of state’s offices, and the legislative reference bureau, and may be obtained for personal use from the American Water Works Association, 6666 West Quincy Avenue, Denver, CO 80235.

**(6e)** “Bedrock” means the rocks that underlie soil material or where weathered in-place consolidated material larger than 2 millimeters in size is greater than 50% by volume.

**(6m)** “Blending” has the meaning specified under s. NR 210.03 (2e).

**Note:** Section NR 210.03 (2e) reads: “Blending” means the routing of untreated or partially treated wastewater around a biological treatment process, or a portion of a biological treatment process, within a sewage treatment facility. The routing of untreated or partially treated wastewater around a portion of a biological treatment

process is considered to be blending only if the entire wastewater flow has not received biological treatment.

**(6s)** “Building sewer” means that part of the drain system not within or under a building which conveys its discharge to a public sewer, private interceptor main sewer, private onsite wastewater treatment system, or other point of discharge or dispersal.

**Note:** This is the same definition as contained in s. SPS 381.01 (44). A building sewer may also be referred to as a building lateral.

**(7)** “Bypass” has the meaning specified in s. NR 205.03 (5).

**Note:** Section NR 205.03 (5) reads: “Bypass” means the intentional diversion of waste streams from any portion of a sewage treatment facility or a wastewater treatment facility. A bypass does not include a building back-up or a combined sewer overflow.

**(7e)** “Combined sewer overflow” has the meaning specified under s. NR 210.03 (3h).

**Note:** Section NR 210.03 (3h) reads: “Combined sewer overflow” means a release of wastewater from a combined sewer system directly into a water of the state or to the land surface.

**(7m)** “Combined sewer system” has the meaning specified under s. NR 210.03 (3p).

**Note:** Section NR 210.03 (3p) reads: “Combined sewer system” means a wastewater collection system owned by a municipality that conveys domestic, commercial, and industrial wastewater and storm water runoff through a single pipe system to a publicly owned treatment works.

**(7s)** “Combined sewer treatment facility” means all the structures, pipes and other equipment that constitute the various treatment processes and treatment units employed to reduce pollutants in wastewater from combined sewer systems.

**(9)** “Controlled diversion” has the meaning specified under s. NR 205.03 (9m).

**Note:** Section NR 205.03 (9m) and Note reads: “Controlled diversion” means the routing of untreated or partially treated wastewater around any treatment unit within a sewage or wastewater treatment facility which is then recombined with undiverted wastewater prior to the effluent sampling location and prior to effluent discharge.

**Note:** Controlled diversions at a sewage treatment facility do not include blending and may occur only in compliance with s. NR 205.07 (1) (v).

**(11)** “Cost-effective analysis” means a systematic comparison of alternative means of meeting state water quality standards, effluent limitations or other treatment standards in order to identify the alternative which will minimize the total resources costs over the planning period. These resources costs include monetary costs and environmental as well as other non-monetary costs.

**(12)** “Department” means the department of natural resources.

**(12g)** “Design flow” means the anticipated wastewater discharge rate to a sewerage system component, which is used to design the sewerage system component to provide compliance with

WPDES permit limits and other performance objectives, during the most critical operating conditions anticipated within the design planning period. Specific design flow terms used in this chapter, include the following:

(a) “Average daily base flow” means the average of the daily flow volumes anticipated to occur for a continuous 12-month period, less infiltration and inflow, and expressed as a daily average.

(b) “Average design flow” means the average of the daily flow volumes anticipated to occur for a continuous 12-month period, expressed as a daily average.

(c) “Maximum month design flow” means the largest volume of flow anticipated to occur during a continuous 30-day period, expressed as a daily average.

(d) “Maximum week design flow” means the largest volume of flow anticipated to occur during a continuous 7-day period, expressed as a daily average.

(e) “Maximum day design flow” means the largest volume of flow anticipated to occur during a one-day period, expressed as a daily average.

(f) “Maximum hour design flow” means the largest volume of flow anticipated to occur during a one-hour period, expressed as a daily or hourly average.

(g) “Peak instantaneous design flow” means the maximum anticipated instantaneous flow.

(h) “Peak design flow” and “maximum design flow” mean the largest volume of flow anticipated to occur on an infrequent basis, expressed as a daily average. The “peak design flow” or “maximum design flow” may be equal to any one of the design flows defined in pars. (c) to (g).

**(12m)** “Design management zone” or “DMZ” means a 3-dimensional area, bounded by a set horizontal distance from the application or containment area, as specified in Table 4, ch. NR 140, and by variable vertical distance which extends from the land surface downward through all saturated formations.

**Note:** The size of the DMZ may be altered by the department based on the criteria in s. NR 140.22 (3).

**(13)** “Dry land access” means a sewage treatment facility service road which has a minimum elevation of at least one foot above the regional flood elevation.

**(13c)** “Excessive infiltration/inflow” means the quantities of infiltration/inflow which can be economically eliminated from a sewerage system by rehabilitation, as determined in a cost-effectiveness analysis that compares the cost of correcting the infiltration/inflow conditions to the total costs for transportation and treatment of the infiltration/inflow.

**(13e)** “High groundwater level” means the higher of either the elevation to which the soil is saturated as observed as a free water surface in an unlined hole or the elevation to which the soil has been seasonally or periodically saturated as indicated by soil color patterns throughout the soil profile.

**(13t)** “Highest anticipated groundwater elevation” means the sum of the calculated mounding effects of the disposal discharge and the seasonal high groundwater level.

**(14e)** “Hydraulic application rate” means the average daily volume of effluent discharged to a designed acreage of the land application system during a calendar month or other period of time specified in a WPDES permit. The rate is calculated by dividing the total discharge volume for the month or period of time by the acreage of land and by the number of days in the month or period of time, usually expressed in units of gallons per acre per day. For overland flow systems, the hydraulic application rate is expressed as a flow rate per unit width of slope per day.

**(14t)** “Hydrogeologist” means a person who is a graduate of an accredited institution of higher education and who has successfully completed 30 semester hours or 45 quarter hours of course work in geology. At least 6 semester hours or 9 quarter hours of the geology course work must be in hydrogeology, geo-hydrology or groundwater geology. This person shall also have acquired through education and actual field experience the ability to direct the drilling of borings, and the installation and development of wells; describe and classify geology samples and evaluate and interpret geologic and hydrogeologic data in accordance with the requirements of chs. NR 110 and 206.

**(15)** “Industrial user” means:

(a) Any nongovernmental, nonresidential user of a municipally owned sewerage system which discharges more than the equivalent of 25,000 gallons per day (gpd) of sanitary wastes and which is identified in the Standard Industrial Classification Manual, 1972, United States Office Management and Budget, as amended and supplemented as of October 1, 1978 under one of the following divisions:

*Division A.* Agriculture, Forestry, and Fishing

*Division B.* Mining

*Division D.* Manufacturing

*Division E.* Transportation, Communications, Electric, Gas, and Sanitary Services

*Division I.* Services.

1. In determining the amount of a user’s discharge, domestic wastes or discharges from sanitary conveniences may be excluded.

2. After applying the sanitary waste exclusion in subd. 1., discharges in the above divisions that have a volume exceeding 25,000 gpd or the weight of biochemical oxygen demand (BOD) or suspended solids (SS) equivalent to that weight found in 25,000 gpd of sanitary waste are considered industrial users. Sanitary wastes, for purposes of this calculation of equivalency, are the wastes discharged from residential users. The municipality shall, with the department’s approval, define the strength of the residential waste discharges in terms of parameters including biochemical oxygen demand (BOD) and suspended solids (SS) per volume of flow as a minimum. Dischargers with a volume exceeding 25,000 gpd or the weight of BOD or SS equivalent to that weight found in 25,000 gpd of sanitary waste are considered as industrial users.

(b) Any nongovernmental user of a municipally owned sewerage system which discharges wastewater to the sewerage system which contains toxic pollutants or poisonous solids, liquids, or gases in sufficient quantity either singly or by interaction with other wastes, to contaminate the sludge of any municipal system, or injure or interfere with any sewage treatment process, constitutes a hazard to humans or animals, creates a public nuisance, or creates any hazard in or has an adverse effect on the waters receiving any discharge from the treatment works;

(c) All commercial users of an individual system constructed with grant assistance under s. 281.57, Stats.

**(16)** “Infiltration” means water other than wastewater that enters a sewerage system (including sewer service connections) from the ground through such sources as defective pipes, pipe joints, connections, or manholes. Infiltration does not include, and is distinguished from, inflow.

**(17)** “Inflow” means water other than wastewater that enters a sewerage system (including sewer service connections) from sources such as roof leaders, cellar drains, yard drains, area drains, foundation drains, sump pumps, drains from springs and swampy areas, manhole covers, cross connections between storm

sewers and sanitary sewers, catch basins, cooling towers, storm waters, surface runoff, street wash waters, or drainage. Inflow does not include, and is distinguished from, infiltration.

**(18)** “Interceptor sewer” means a sewer whose primary purpose is to transport wastewaters from collector sewers to a treatment facility.

**(18m)** “Intermediate sludge storage” means the storage of sludge for a period of more than 24 hours and no more than 3 months.

**(19)** “Lagoon” means those sewage treatment facilities where the wastewater or sludge containment structure is constructed primarily of earthen materials.

**(19m)** “Long-term sludge storage” means the storage of sludge for a period exceeding 3 months.

**(21)** “Municipality” means any city, town, village, county, utility district, town sanitary district, public inland lake protection and rehabilitation district or metropolitan sewage district.

**(22)** “NEC” means the NFPA 70 National Electrical Code. Copies of the National Electrical Code are available for inspection at the offices of the department of natural resources, the secretary of state’s office, and the legislative reference bureau. Copies may be obtained for personal use from the National Fire Protection Association, 1 Batterymarch Park, Quincy, MA 02169-7471.

**(23)** “Owner” means the state, county, town, town sanitary district, city, village, firm, company, institution, association, utility district, school district, metropolitan sewerage district, or individual owning or operating a sewerage system.

**(25)** “Planning area” means that area under study as part of a facilities plan.

**(26)** “Planning period” means the period over which sewerage system alternatives are evaluated for cost-effectiveness. The planning period begins with the initiation of the operation of the proposed facilities.

**(26m)** “Private interceptor main sewer” means a sewer serving two or more buildings and not part of the municipal sewer system.

**Note:** This is the same definition as contained in s. SPS 381.01 (193).

**(27)** “Reviewable project” means any construction or installation project for which department approval is required, pursuant to s. 281.41, Stats., including any new sewerage system; and, any improvements, extensions, or alterations of existing sewerage systems which may effect the quality or quantity of effluent or the location of any outfall.

**(27e)** “Sanitary sewer overflow” has the meaning specified under s. NR 210.03 (10).

**Note:** Section NR 210.03 (10) reads: “Sanitary sewer overflow” means a release of wastewater from a sewage collection system or an interceptor sewer directly into a water of the state or to the land surface.

**(27m)** “Sanitary sewer overflow structure” means the physical structure, hydraulic mechanisms, and piping specifically constructed to convey a sanitary sewer overflow.

**(27s)** “Satellite sewage collection system” has the meaning specified under s. NR 205.03 (31r).

**Note:** Section NR 205.03 (31r) reads: “Satellite sewage collection system” means a municipally owned or a privately owned sewage collection system that conveys wastewater to another satellite sewage collection system or to another sewerage system that provides wastewater treatment and discharges under a separate WPDES permit.

**(28)** “Sewage collection system” means the common sanitary sewers, interceptor sewers, and appurtenant equipment, such as lift stations, within a sewerage system which are primarily installed to receive wastewaters directly from facilities which convey wastewater from individual structures or from private property, and which include service connection “Y” fittings designed

for connection with those facilities. The facilities which convey wastewater from individual structures, such as building sewers and private interceptor sewers, from private property to the public sanitary sewer, or its equivalent, are specifically excluded from the definition of “sewage collection system”; except that pumping units and pressurized lines for individual structures or groups of structures are included as part of a “sewage collection system” when such units are cost effective and are owned and maintained by the sewerage system owner.

**(29)** “Sewage treatment facility” means all the structures, pipes, and other equipment that constitute the various treatment processes and treatment units employed to reduce pollutants in sewage.

**(29d)** “Sewage treatment facility overflow” has the meaning specified under s. NR 210.03 (13).

**Note:** Section NR 210.03 (13) reads: “Sewage treatment facility overflow” means a release of wastewater from a location within a sewage treatment facility, other than permitted effluent outfall structures, directly to a water of the state or to the land surface. A sewage treatment facility overflow does not include blending, controlled diversions or discharges from permitted combined sewage treatment facility effluent outfall structures.

**(29h)** “Sewage treatment facility overflow structure” means the physical structure, hydraulic mechanisms, and piping specifically constructed to convey a sewage treatment facility overflow.

**(29m)** “Sewer extension” means installation of a sewer or interceptor sewer, or extension thereof, to provide additional conveyance capacity and service to development within the existing or proposed tributary area of the extension. Alterations or modifications of existing sewerage systems designed to replace inadequate existing structures or installed because of inadequate hydraulic sewer capacity and that do not extend sanitary sewer service to areas previously not served are not sewer extensions.

**(29t)** “Sewer service area” means that area served or anticipated to be served by a sewage collection system.

**(30)** “Sewerage system” means all structures, conduits and pipes, by which sewage is collected, treated, and disposed of, except plumbing inside and in connection with buildings served, and service pipes, from building to street main.

**(31c)** “Short-term sludge storage” means the storage of sludge for a period of no more than 24 hours.

**(31e)** “Sludge storage” means the retention of sludge at a treatment plant or at an approved off-site facility.

**(32)** “Staging period” means the period of time during which reserve capacity will be provided in the sewerage system for future domestic, commercial, and industrial flows.

**(32g)** “Treatment process” means a physical, biological or chemical action that is applied to wastewater to remove or reduce pollutants. A treatment process may consist of multiple individual treatment units. “Treatment process” includes screening, chemical treatment, sedimentation, biological treatment, filtration, disinfection, and sludge digestion.

**(32i)** “Treatment unit” means individual structures or equipment within a sewage or wastewater treatment facility that are part of a treatment process. Typical treatment units are screens, clarifiers, aeration tanks, filters, digesters, and lagoons.

**(32m)** “Water table observation well” means any groundwater monitoring well whose screen intersects the water, which is installed for the specific purpose of determining either the elevation of the water table or the physical, chemical, biological, or radiological properties of groundwater at the water table, or both.

**(34)** “WPDES permit” means the Wisconsin pollutant discharge elimination system permit issued by the department under ch. 283, Stats., for the discharge of pollutants.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, December, 1978, No. 276, eff. 1-1-79; cr. (20), Register, August, 1981, No. 308,



eff. 9-1-81; renum. (3) to (20) to be (11), (12), (14) to (18), (21), (23), (25) to (32) and (34) and am. (29) and (34), cr. (4) to (10), (13), (19), (20), (22), (24) and (33), Register, February, 1983, No. 326, eff. 3-1-83; cr. (6m), (12m), (13e), (13i), (14e), (14i), (18m), (19m), (31e), (31i) and (32m), am. (19), Register, November, 1990, No. 419, eff. 12-1-90; corrections in (2), (27), and (34) were made under s. 13.93 (2m) (b) 7., Stats., Register, May, 2001, No. 545; corrections in (3), (4), (6), (22) and (33) made under s. 13.92 (4) (b) 6., Stats., Register February 2010 No. 650; CR 09-123: r. (3), (5), (20), (24) and (33), r. and recr. (4) and (6), am. (9) and (22), cr. (12g) Register July 2010 No. 655, eff. 8-1-10; CR 12-027: renum. (6m) to (6e), cr. (6m), (6s), r. and recr. (7), cr. (7e), (7m), (7s), r. (8), r. and recr. (9), r. (10), am. (17), cr. (26m), (27e), (27m), (27s), am. (28), r. and recr. (29), cr. (29d), (29h), (30p), renum. (31) to (29t), cr. (32g), (32r) Register July 2013 No. 691, eff. 8-1-13; renum. (14), (30p), (31t), (32r) to (13c), (29m), (31c), (32i) under s. 13.92 (4) (b) 1., Stats., Register July 2013 No. 691; correction in (15) (c) made under s. 13.92 (4) (b) 7., Stats., Register June 2020 No. 774.

**NR 110.04 Alternative requirements.** (1) If the owner of a proposed reviewable project feels that compliance with the design requirements of this chapter is impracticable, the reasons therefor shall be fully communicated in writing to the department prior to the submission of final plans. This communication must set forth alternative requirements for which department approval is sought and all pertinent facts, data, reports and studies supporting the imposition of such alternative requirements.

(2) If the department determines that compliance with the design requirements of this chapter would be impracticable in specific cases, it may approve alternative requirements which, in its opinion, are in substantial compliance with the requirements of this chapter.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74.

**NR 110.05 Sewer extensions.** (1) **PURPOSE.** The purpose of this section is to insure that department approval of applications for sanitary sewer extensions are consistent with and enhance the policy of the state to restore and maintain the chemical, physical and biological integrity of its waters to protect public health, safeguard fish and aquatic life and scenic and ecological values and enhance the domestic, municipal, recreational, industrial, agricultural and other uses of water.

(3) **PERMISSIVE APPROVALS RELATED TO PERMITTED EFFLUENT LIMITATIONS.** (a) Unless an approval would be contrary to the purpose of this section, applications for sanitary sewer extensions that comply with all applicable requirements of this chapter shall be approved if the sewer will be tributary to a sewage treatment facility in compliance with the monthly average effluent limitations for biochemical oxygen demand (BOD) and total suspended solids contained in its WPDES permit.

(b) In the event that the WPDES permit for a sewage treatment facility currently discharging an effluent in accordance with ch. NR 210 establishes a compliance schedule for achievement of any more stringent water quality related effluent limitations for biochemical oxygen demand and total suspended solids applicable to such treatment facility, compliance with the schedule of compliance in the discharge permit will be deemed to be compliance with the applicable water quality related effluent limitations.

(c) In determining whether a discharged effluent is in compliance with the monthly average effluent limitations for biochemical oxygen demand (BOD) and total suspended solids contained in a WPDES permit, the following procedure shall apply:

1. Compliance shall be determined by department review of the previous 12 months of discharge monitoring data. If 12 months of data are not available, the review shall be based on the data that are available.

2. More than a total of 3 months of violations of the monthly average limitations for either BOD or total suspended solids or both in the previous 12 months (or the equivalent ratio for the

number of months of data available) shall cause denial, subject to the following additional considerations:

a. Recognition of the inherent inaccuracy of the BOD and total suspended solids tests shall be given by multiplying the monthly average effluent limitations as specified in the permit by a factor of 1.3 for BOD and 1.2 for total suspended solids for purposes of determining whether monthly average effluent results are in compliance.

b. The department may grant approval if it determines that, due to a demonstrable action by the permittee, the sewage treatment facility has been in compliance for four or more consecutive months, thus demonstrating a trend toward better operation.

c. The department may grant approval in those instances where the permittee demonstrates that noncompliance with the effluent limitations has been caused by algae growth in a sewage treatment facility utilizing lagoons as the principal treatment process.

d. The department may grant approval if it determines that noncompliance with the effluent limitations has been caused by operating difficulties associated with startup for those sewage treatment facilities which have recently been constructed or undergone major modification or expansion. The period described as startup may be no longer than 12 consecutive months.

(4) **DENIAL RELATED TO PERMITTED EFFLUENT LIMITATIONS.** (a) Taking into account the factors in sub. (3) (c), applications for sanitary sewer extensions shall be denied if the sewer will be tributary to a sewage treatment facility not in compliance with the monthly average effluent limitations for biochemical oxygen demand (BOD<sub>5</sub>) and total suspended solids contained in its WPDES permit.

(b) If the WPDES permit for a sewage treatment facility establishes a compliance schedule for achievement of any more stringent water quality related effluent limitations for biochemical oxygen demand and total suspended solids applicable to such treatment plant, compliance with the schedule of compliance in the discharge permit shall be deemed to be compliance with the applicable water quality related effluent limitations.

(5) **EXCEPTIONS.** Sewer extensions otherwise prohibited by sub. (4) may be granted by the department upon the determination of any of the following:

(a) That construction of the subdivision, commercial establishment, institutional facility or industrial plant had commenced prior to May 24, 1976, as evidenced by the issuance of a building permit;

(b) That the area to be served was developed prior to May 24, 1976, and that the sewer extension will eliminate use of existing private sewage systems which pose a threat to the public health or safety, provided that connections to the sewer are allowed only for the existing development;

(d) That the proposed extension is a modification of a sewer extension previously approved by the department, providing that the modification results in no increase in the anticipated waste discharge to the sewer system;

(e) That the facilities to be served are intended primarily to provide educational, humanitarian, or charitable community services;

(f) That the program, time schedule, and the commitment to proceed are established in a court-approved stipulation, order, or judgment.

(6) **CONNECTION RESTRICTIONS.** As a condition of any approval granted under sub. (5), the department may require that an applicant for a sewer extension restrict the number of connections

made to the sewer system in accordance with a prescribed schedule.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; emerg. r. and recr. eff. 5-24-76; r. and recr. Register, September, 1976, No. 249, eff. 10-1-76; am. (3) (b) and (4) (b), Register, April, 1980, No. 292, eff. 5-1-80; am. Register, August, 1981, No. 308, eff. 9-1-81; am. (4), r. (7), renum. (8) to be (7), Register, June, 1982, No. 318, eff. 7-1-82; emerg. cr. (5) (g), eff. 4-15-83; cr. (5) (g) Register, September, 1983, No. 333, eff. 10-1-83; corrections in (2) (c) and (g) were made under s. 13.93 (2m) (b) 7., Stats., Register, May, 2001, No. 545; CR 09-123; am. (2) (c), (5) (c) and (6), r. and recr. (4), r. (5) (g) Register July 2010 No. 655, eff. 8-1-10; CR 12-027: r. (2), am. (3), (4), r. (5) (c), r. (7) Register July 2013 No. 691, eff. 8-1-13.

**NR 110.06 Construction plans for reviewable projects.**

(1) All construction plans for reviewable projects submitted to the department shall be in conformance with ch. NR 108, and shall bear a suitable title block which includes the name of the owner, the scale and the date. The north point shall be shown on each plan. All plans shall be clear and legible. Blueprints will not be accepted. The datum used shall be indicated and shall be related to U.S.G.S. datum.

(2) Detailed construction plans shall contain appropriate plan views, elevations, necessary sections and supplemental views which together with the specifications provide all necessary information for construction of the project. Manufacturers' drawings shall not be accepted.

(3) All construction plans shall be in conformance with an approved facilities plan as required in s. NR 110.08 (1).

(4) If the construction of a proposed project requires, or may require, any permit under s. 30.12, 30.19, 30.195, or 30.20, Stats., application for the necessary permits shall be made to the department at the same time the project plans and specifications are submitted for review. Failure to apply for the necessary permits shall be cause for denial or rejection of the plans and specifications.

**Note:** Applicable state and local codes, including those of the department of industry, labor and human relations, the public service commission and the department of health and social services, should be consulted for other requirements.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, December, 1978, No. 276, eff. 1-1-79; cr. (4), Register, February, 1983, No. 326, eff. 3-1-83.

**NR 110.07 Specifications for reviewable projects.**

(1) Complete technical specifications for all reviewable projects shall accompany the construction plans. Where feasible the specifications shall contain provisions for maintaining the same degree of wastewater treatment during construction as that which existed prior to the start of construction.

(2) The specifications accompanying the detailed construction drawing shall include, wherever applicable:

- (a) All construction information not shown on the plans;
- (b) The complete requirements for all mechanical and electrical equipment;
- (c) The type and operating characteristics of all equipment;
- (d) The laboratory fixtures and equipment;
- (e) The construction materials to be used;
- (f) The identification of the chemicals to be used; and
- (g) The instructions for testing materials and equipment to meet design standards.

(3) Specifications reproduced from manufacturers' data and bearing the manufacturers' labels will not be accepted.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, December, 1978, No. 276, eff. 1-1-79.

**NR 110.08 Facilities plans for reviewable projects.**

(1) **APPLICABILITY.** A facilities plan shall be prepared for each reviewable project submitted to the department for approval. Facilities plans for sewage treatment facilities or new sewage collection systems shall be submitted to and approved by the depart-

ment prior to submittal of the construction plans and specifications. The department may accept construction plans and specifications for review prior to facilities plan approval provided that all substantive issues of the facilities plan review have been resolved.

(2) **CONTENT.** The facilities plan for municipally owned sewage treatment facilities, sewage collection systems, and interceptors shall contain all of the information required by s. NR 110.09 (1) through (6), 110.10 (1) and (2), or 110.11 (1), whichever are applicable. The following facilities planning requirements do not apply to nonmunicipally owned facilities: s. NR 110.09 (1) (b) 3., 5. and 8., and (2) (b) to (e) and (j) to (m). The level of detail necessary to fulfill the requirements of this subsection may vary depending on the size and complexity of the project.

(3) **WISCONSIN ENVIRONMENTAL POLICY ACT REVIEW.** Facilities plans shall be reviewed by the department in accordance with ch. NR 150.

(4) **CONFORMANCE WITH APPROVED AREAWIDE WASTE TREATMENT MANAGEMENT PLANS.** All approvable sewerage system facility plans must be in conformance with approved areawide waste treatment management plans unless the department determines that such plans conflict with the department's responsibilities to protect, maintain, and improve the quality and management of the waters of the state, ground and surface, public and private. In the absence of an approved areawide waste treatment management plan, no determination of such conformance is required.

(5) **APPROVAL OF NEW SEWAGE TREATMENT FACILITIES.** It is the policy of the department to restrict the construction of new sewage treatment facilities in order to preserve and protect the quality of the waters of the state. The department may deny requests for approval of new sewage treatment facilities unless they meet the following criteria:

(a) *Treatment facilities to serve existing residential development.* Proposals for new treatment facilities to serve existing residential development may not be approved unless:

- 1. They are necessary to solve a documented and severe existing water quality (groundwater or surface water) or public health problem related to inadequate existing residential sewage disposal; or, are needed to replace an existing treatment facility which is not in compliance with its WPDES permit;
- 2. They are the cost-effective alternative solution in accordance with s. NR 110.09 (1) (a); and
- 3. They are municipally owned, operated and maintained.

(b) *Interim treatment facilities.* An interim treatment facility is one which would serve areas which are within the future sewer service area of another existing facility as delineated in an approved areawide water quality management plan as provided for in ch. NR 121. Proposals for new interim treatment facilities may not be approved unless:

- 1. They are necessary to solve a documented and severe existing water quality (groundwater or surface water) or public health problem related to inadequate existing residential sewage disposal; or, are needed to replace an existing treatment facility which is not in compliance with its WPDES permit;
- 2. They are the cost-effective alternative solution in accordance with s. NR 110.09 (1) (a);
- 3. They are municipally owned, operated and maintained;
- 4. The sewage collection system is designed so that it can be easily connected to the regional system in the future;
- 5. The sewer service area of the proposed system lies entirely within the planned service area of the regional system as delin-

eated in an approved areawide water quality management plan; and

6. An agreement is signed by all involved municipalities which provides for a specified date of abandonment and connection. This intermunicipal agreement may be reviewed and approved by the department prior to facilities plan approval. The WPDES permits may contain schedules for facilities abandonment and connection.

(c) *Treatment facilities serving isolated nonresidential development.* Nonresidential development includes things such as parks and recreational facilities, airports, highway oriented commercial facilities and institutions such as hospitals, nursing homes, prisons and schools. Proposals for new treatment facilities to serve nonresidential development may not be approved unless:

1. Joint treatment with other wastewater treatment systems is not feasible;
2. The proposed facilities are designed to treat only wastes generated by the proposed nonresidential development; and
3. The WPDES permit limits service to the proposed nonresidential development.

(d) *Treatment facilities to serve new residential development.* Proposals for new treatment facilities intended to serve new residential development such as facilities for residential subdivisions, mobile home parks and condominium developments may be denied.

(dg) *Variiances.* Variances to this general prohibition may be granted:

1. Only after the department has considered:
  - a. The general public interest;
  - b. Environmental impacts;
  - c. Socioeconomic impacts; and
  - d. The impact on orderly development and provision of general governmental services within the service area; and
2. Only after a finding that all of the following criteria are met:
  - a. The proposal is consistent with the department's responsibility to protect, maintain and improve the quality and management of the waters of the state;
  - b. The proposed facilities will be municipally owned, operated and maintained;
  - c. The proposed facilities will be more cost-effective in accordance with s. NR 110.09 (1) (a) than other treatment and discharge alternatives; and
  - d. All other federal, state and local approvals and permits have been obtained.

(dm) *Treatment facilities to serve existing mobile home parks and condominium developments.* Proposals for new treatment facilities to serve existing residential developments at mobile home parks and condominium developments may not be approved unless:

1. The conditions of par. (a) are met; or
2. The conditions of par. (a) 1. and 2. are met and the owner submits the following:
  - a. Adequate proof that sufficient funds to operate, maintain and abandon the facility, if necessary, will be available for the life of the facility;
  - b. Documentation showing that the new treatment facilities are being proposed as a replacement of a failing septic tank/soil absorption system which has been in use for at least 10 years; and
  - c. Proof of the inability to form a town sanitary district or other appropriate municipal entity to oversee the facility.

(e) *Conformance with areawide water quality management plans.* In addition to the requirements of pars. (a) through (dg), the new sewage treatment facilities shall also be in conformance with any approved areawide water quality management plan. These plans may be consistent with the criteria in pars. (a) through (dg). These plans as approved by the department may also contain additional criteria necessary to address regional or local considerations.

(6) **COST EFFECTIVENESS.** A cost-effectiveness analysis shall be performed as part of the evaluation of alternatives in each facilities plan. The cost-effectiveness analysis shall be prepared in accordance with s. NR 110.09 (2). Except as provided for in s. NR 110.09 (2) (j) 4. c., the most cost-effective alternative shall be selected for implementation.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, December, 1978, No. 276, eff. 1-1-79; cr. (5), Register, August, 1981, No. 308, eff. 9-1-81; am. (1) and (2), cr. (6), Register, February, 1983, No. 326, eff. 3-1-83; cr. (5) (dm), Register, November, 1990, No. 419, eff. 12-1-90; correction in (5) made under s. 13.93 (2m) (b) 1. and 7., Stats., Register, September, 1995, No. 477; CR 09-123; renum. (3) (a) to be (3) and am., r. (3) (b) Register July 2010 No. 655, eff. 8-1-10.

### NR 110.09 Sewage treatment facilities projects. (1)

**FACILITIES PLANS FOR SEWAGE TREATMENT FACILITIES PROJECTS.** (a) Facilities plans consist of those necessary plans and studies which directly relate to the construction of the proposed facilities. Facilities planning shall demonstrate the need for the proposed facilities. Through a systematic evaluation of feasible alternatives facilities planning shall also demonstrate that the selected alternative is the most cost-effective means of meeting established effluent limitations and water quality standards. The most cost-effective alternative is that which will result in the minimum total resources costs over the planning period. The planning period of the facilities plan shall be 20 years. The total resources costs include monetary costs, environmental and social considerations, and other nonmonetary factors. The interest (discount) rate to be used in calculating present worth shall be obtained from the department when beginning facilities planning.

(b) Facilities planning shall include the following information in such detail as the department deems appropriate for the specific project:

1. A description of the sewerage system for which construction drawings and specifications are to be prepared. This description shall include preliminary engineering data, cost estimates for design and construction of the sewerage system, and a schedule for completion of design and construction. The preliminary engineering data shall include, to the extent appropriate, information such as a schematic flow diagram, unit processes, design data regarding detention times, flow rates, sizing of units, and so forth. This is commonly referred to as the Unit Equipment and Design Report.

2. A description of the selected complete sewerage system of which the proposed facilities will be a part using maps, diagrams and plans as appropriate. This description shall include:

- a. The delineation of a sewer service area for the complete sewerage system based on a 20-year population projection and density assumptions;
- b. A description of the collection system including existing and proposed trunk sewers and interceptors;
- c. A description of the existing and proposed sewerage treatment system including ultimate disposal of wastewater and sludge;
- d. A planning area map showing individual systems, if individual systems are to be a part of the cost-effective solution proposed for state or EPA funding.



3. Infiltration/inflow documentation in accordance with subs. (5) and (6).

4. A cost-effectiveness analysis of alternatives for the sewerage system prepared in accordance with sub. (2). The most cost-effective alternatives shall be selected for implementation in accordance with s. NR 110.08 (6).

5. For facilities plans for state funded projects, parallel cost estimates shall be provided for the facilities necessary to transport and/or treat the fundable capacity, as well as a cost estimate for the total proposed sewerage system.

6. An identification of effluent discharge limitations including water quality related effluent limitations, and where a Wisconsin Pollution Discharge Elimination System (WPDES) permit has been issued, a copy of the permit for the proposed sewerage system.

7. Required comments or approvals of relevant state, interstate, regional, and local agencies.

8. An estimate of the anticipated cost to the average user of the system. This cost shall be presented at the public hearing required under sub. (4).

9. A brief summary of the public hearing required under sub. (4) or any other public meeting or hearing held during the planning process including a summary of the views expressed.

10. A brief statement demonstrating that the authorities who will be implementing the plan have the necessary legal, financial, institutional, and managerial resources available to insure the construction, operation, and maintenance of the proposed treatment works.

11. A description of potential opportunities for recreation, open space, and access to bodies of water analyzed in planning the proposed sewerage system and the recommended actions. The facility plan shall also describe measures taken to coordinate with federal, state and local recreational programs and with recreational elements of applicable approved areawide waste treatment management plans.

**Note:** Facilities plans for projects subject to the requirements of this section may include results from a system evaluation and capacity assurance plan under s. NR 110.10 (4).

**(2) CONTENT OF THE COST-EFFECTIVENESS ANALYSIS.** The cost-effectiveness analysis shall include:

(a) The relationship of the size and capacity of alternative systems to the needs to be served, including reserve capacity;

(b) An evaluation of alternative flow and waste reduction measures, including non-structural methods;

(c) An evaluation of improved effluent quality attainable by upgrading the operation and maintenance and efficiency of existing facilities as an alternative or supplement to construction of new facilities;

(d) An evaluation of the capability of each alternative to meet secondary treatment standards or applicable water quality related effluent limitations. The sewerage system design must be based upon achievement of not less than secondary treatment standards as defined by ch. NR 210;

(e) An identification of and provision for applying technologies included under each of the following waste treatment management techniques:

1. Biological or physical-chemical treatment and discharge to receiving waters;

2. Systems employing the reuse of wastewater and recycling of pollutants;

3. Land application techniques;

4. Systems including revenue generating applications; and

5. On-site and non-conventional systems.

(f) All construction of publicly-owned sewerage systems and privately owned domestic sewerage systems discharging to surface waters shall be based upon application of secondary treatment as a minimum. Where application of secondary treatment would not provide for attainment of water quality standards, the facilities plan shall provide for attaining the applicable standards by designing to meet appropriate water quality related effluent limitations. Sewerage systems discharging to the ground water shall comply with the applicable discharge requirements of ch. NR 206. The alternative of treating combined sewer overflows shall also be considered.

(g) An evaluation to determine the cost-effective means of disposing of treated effluent.

(h) An evaluation of the most cost-effective means of treating, handling, and disposing of sludge. This evaluation shall include at a minimum the following items:

1. A description of the current sludge treatment, handling and disposal operations including a discussion of current quantities being produced, a description of current sludge quality including a sludge analysis, a description of any problems associated with the existing operations, and a description of industrial contributions that may affect the quantity and quality of sludge;

2. An analysis of the anticipated quantity and quality of the sludge from the proposed facility;

3. A brief description of alternative technologies applicable to the proposed facility improvements, such as, thickening, stabilization, dewatering, storage, transportation, and ultimate disposal;

4. A cost-effectiveness analysis of the feasible alternatives including an assessment of the environmental impacts as specified in sub. (3);

5. An evaluation of the storage requirements either at the sewage treatment facility or at an offsite location. The evaluation shall include an estimate of the maximum period of time necessary to store sludge, and a description of the location, accessibility, soils, necessary local permits, depth to groundwater, distance to residential homes, type of facility, topography and any other appropriate information. The storage recommendations shall comply with s. NR 110.26 (10).

6. An estimate of the amount of land required for each alternative shall be made. Land requirements for landfilling of sludge shall be based upon accepted landfill design practices. Department approval in accordance with chs. NR 500 to 520, is required for construction of sludge landfills and prior to disposal of sludge at an existing licensed landfill.

7. A discussion of the procedures and timing for abandonment of the existing sludge facilities, if appropriate. This shall include, but not be limited to, the types of sludge wastes to be disposed of during abandonment, ultimate disposal location, possible construction scheduling, quantity of wastes, quality of wastes and any special problems associated with the disposal of these wastes; and

8. A summary describing the selected plan and its anticipated environmental impacts. Those actions necessary for implementing and operating the sludge management plan shall be presented. This shall include, but not be limited to, the estimated sludge treatment and disposal costs, operator time, discussion of applicable federal and state laws, necessary local permits, public participation programs, training of operators and any other actions necessary to provide for an environmentally sound sludge management program.

(i) An adequate assessment of the expected environmental impacts of the alternatives (including sites) in accordance with sub. (3). This assessment shall be an integral part of the analysis of al-

ternatives for cost-effectiveness. The assessment shall be revised as necessary to include information developed during subsequent project steps.

(j) An analysis of the most cost-effective design staging and sizing. The staging and sizing of treatment works shall be based upon the following:

1. 'Population projections.' Population projections for facilities planning shall be in conformance with those contained in applicable approved areawide waste treatment management plans and rules adopted pursuant to ss. 16.96 and 281.57 (4) (b), Stats. If such projections are not available, the engineer shall project future population growth based on trends in the recent past.

2. 'Wastewater flow estimates.' In determining total average flow for the design of sewerage systems, the flows to be considered include the average daily base flows (ADBF) expected from each of the following: residential sources, commercial sources, institutional sources, and industries the system will serve plus allowances for future industries and nonexcessive infiltration or inflow.

2m. 'Estimation methods.' The estimation of existing and future ADBF from combined residential, commercial, and institutional sources, shall be based upon one of the following methods:

a. Existing ADBF shall be estimated based upon a fully documented analysis of water use records adjusted for consumption and losses or on records of wastewater flows for extended dry periods less estimated dry weather infiltration. Future flows for the sewerage system design shall be estimated by determining the existing per capita flows, subtracting any projected per capita water conservation flow reduction and multiplying this figure by the future projected population to be served. Seasonal population can be converted to equivalent full-time residents using the following multipliers:

- Day-use visitor (0.1-0.2).
- Seasonal visitor (0.5-0.8).

The preferred method shall be used wherever water supply records or wastewater flow data exist. Allowances for future increases of per capita flow over time will not be approved.

b. Where water supply and wastewater flow data are lacking, existing and future ADBF shall be estimated by multiplying a gallon per capita per day (gpcd) allowance not exceeding those in the following table by the estimated total of the existing and future resident populations to be served. The tabulated ADBF allowances include estimates for commercial and institutional sources as well as residential sources. The department may approve exceptions to the tabulated allowances where large commercial and institutional flows (more than 25% of total estimated ADBF) are documented.

| Description   | Gallons per capita per day (gpcd) |
|---|-----------------------------------|
| Non-SMSA cities and towns with projected total 10-yr population of 5,000 or less..... | 60-70                             |
| Other cities and towns.....   | 65-80                             |

3. 'Industrial flows.' a. The sewerage system total design flow capacity may include allowances for industrial flows. The allowances may include capacity needed for industrial flows which the existing sewerage system presently serves. However, these flows shall be carefully reviewed and means of reducing them shall be considered. Capacity needs for existing flows from industrial users and for future flows from all industries intending to increase their flows or relocate in the area must be documented.

b. While many uncertainties accompany forecasting future industrial flows, there is still a need to allow for some unforeseeable future industrial growth. Thus, design capacity of the treatment works may include (in addition to the existing industrial flows and future documented industrial flows) a nominal flow allowance for future nonidentifiable industries or for unplanned industrial expansions, provided that areawide waste treatment management plans, land-use plans and zoning provide for the industrial growth. This additional allowance for future unplanned industrial flow may not normally exceed 5%, or 10% for towns with less than 10,000 population, of the total average design flow of the treatment works exclusive of the allowance or 25% of the total industrial flow, existing plus documented future, whichever is greater.

4. 'Staging of sewage treatment facilities.' For municipally owned projects the design capacity of new, upgraded or expanded sewage treatment facilities shall not exceed that necessary for wastewater flows projected during the initial staging period. Privately owned domestic waste treatment facilities shall provide design capacity for estimated flows 20 years from the estimated time of start-up of the facilities unless the cost-effectiveness staging analysis is done to justify a lesser design staging period. The staging period for municipally owned waste treatment facilities shall be determined by either of the following methods:

a. The owner shall analyze at least 3 alternative staging periods (10 years, 15 years and 20 years) and the least costly (i.e., total present worth or average annual cost) staging period shall be selected.

b. The staging period may not exceed the period which is appropriate according to the following table.

**Staging Periods for Treatment Facilities**

| Flow Growth Factors (20 yrs)                                       | Maximum Initial Staging Period |
|--|--------------------------------|
| 1. Average design flow less than 1.3 times initial average flow    | 20 years                       |
| 2. Average design flow 1.3 to 1.8 times initial average flow       | 15 years                       |
| 3. Average design flow greater than 1.8 times initial average flow | 10 years                       |

c. A municipality may stage the construction of a treatment plant for a shorter period than the maximum allowed under this subdivision. A shorter staging period might be based upon environmental factors (secondary impacts, compliance with other environmental laws, energy conservation, water supply), an objective concerning planned modular construction, the utilization of temporary treatment plants, or attainment of consistency with locally adopted plans including comprehensive and capital improvement plans. However, the staging period may in no case be less than 10 years, because of associated cost penalties and the time necessary to plan and construct later stages.

(k) An evaluation of the costs, cost-savings, and effects of flow reduction measures unless the existing average daily base flow from the area is less than 70 gpcd, or the current population of the municipality is under 5,000, or the area is exempted by the department for having an effective existing flow reduction program. A flow reduction program shall be adopted by municipalities which shall include those measures determined to be cost effective.

(L) An analysis of innovative and alternative treatment processes and techniques that reclaim and reuse water, productively recycle wastewater constituents, eliminate the discharge of pollutants or recover energy. Where certain categories of alternative technologies may not be generally applicable because of prevail-



ing climatic or geological conditions, a detailed analysis of these categories of alternative technologies is not required. However, the reason for such a rejection must be fully substantiated in the facilities plan.

(m) An analysis of the primary energy requirements (operational energy inputs) for each system considered. The alternative selected shall propose adoption of measures to reduce energy consumption or to increase recovery as long as such measures are cost effective.

(n) A flood analysis for the selected treatment facility site if the site is in, or suspected to be in, a floodplain. The analysis shall meet the requirements of s. NR 116.07. The analysis shall determine the limits of the floodplain and the floodway, the regional flood elevation, and the effects on floodstage of constructing the sewage treatment facility, including dry land access and flood protection. The flood velocities at the sewage treatment facility site, and the duration of the regional flood shall also be determined. If a dry land access waiver is requested in accordance with s. NR 110.15 (3) (c), the flood analysis shall also include the information necessary to support the request.

(o) An assessment of the location of the sewage treatment facilities relative to commercial establishments and to buildings which are occupied or intended for residential use, and from land which is being actively developed for commercial or residential use. The location of sewage treatment facilities shall comply with the provisions of s. NR 110.15 (3) (d).

(p) An assessment of the location of land disposal systems relative to public water supply wells. The location and horizontal separation from the proposed land disposal site and any public water supply well shall be shown. The assessment shall discuss the hydrogeologic conditions of the area, the direction of groundwater movement, the depth of the public well casing, and any other appropriate information. The department will determine whether the separation distance between the land disposal system and the public well is sufficient to protect the public health and quality of the public water supply.

(q) Soil boring logs if the selected treatment alternative includes lagoons or land disposal of effluent. The borings shall supply accurate information about the soil conditions, and groundwater and bedrock elevations at the proposed treatment facility site.

(r) Any facility plan which recommends the abandonment of a wastewater treatment, sludge or septage storage lagoon, or land disposal system shall include an abandonment plan. An abandonment plan outlining the proposed method of abandonment of the facility shall be submitted as part of the facility plan to the department for approval. This abandonment plan shall provide for the removal and proper recycling, treatment or disposal of any accumulated solid matter, solid or liquid wastes or wastes in combination with soil. All recycling, treatment and disposal shall be conducted so as to protect public health and the environment. Unless otherwise directed by the department, all abandonment plans for wastewater treatment, sludge or septage storage lagoons, or high rate land disposal systems shall comply with ch. NR 720 for soils that have been contaminated by the contents of the lagoon or system. The abandonment plan shall address relandscaping necessary to prevent accumulation of standing water or runoff and shall provide for completion of the relandscaping within 2 years of the date on which the structure was last used as it was originally intended. The department shall require groundwater monitoring for a minimum of one year at a quarterly frequency after the abandonment of facilities which have an existing groundwater monitoring system. Groundwater monitoring may be required on a case-by-case basis for facilities which do not

have existing groundwater monitoring systems. The monitoring data shall be reviewed after 1 year and the department shall determine whether groundwater monitoring should be continued or not. Any groundwater monitoring wells which are no longer necessary shall be abandoned in accordance with ch. NR 141 and documentation of well abandonment shall be provided to the department.

(3) CONTENT OF AN ENVIRONMENTAL ANALYSIS. An adequate environmental analysis must be an integral, though identifiable, part of any facilities plan submitted to the department under sub. (1). The analyses that constitute an adequate environmental analysis shall include:

(a) *Description of the existing environment without the project.* This shall include for the delineated planning area a description of the present environmental conditions relevant to the analysis of alternatives or determinations of the environmental impacts of the proposed action. This description shall include, but not be limited to, discussions of the following topics where applicable to a particular study: surface and ground water quality; water supply and use; general hydrology; air quality; noise levels; energy production and consumption; land use trends; population projections, wetlands, floodplains, coastal zones and other environmentally sensitive areas; historic and archaeological sites; other related federal or state projects in the area; and plant and animal communities which may be affected, especially those containing threatened or endangered species.

(b) *Description of the future environment without the project.* The future environmental conditions with the no project alternative shall be forecast, covering the same areas listed in par. (a).

(c) *Evaluation of alternatives.* This discussion shall include a comparative analysis of feasible options and a systematic development of wastewater treatment alternatives. The alternatives shall be screened with respect to capital and operating costs; significant primary and secondary environmental effects; physical, legal or institutional constraints; and whether or not they meet regulatory requirements. Special and induced impacts such as development. The reasons for rejecting any alternatives shall be presented in addition to any significant environmental benefits precluded by rejection of an alternative. The analysis should consider, when relevant to the project:

1. Flow and waste reduction measures, including infiltration/inflow reduction;
2. Alternative locations, capacities, and construction phasing of facilities;
3. Alternative waste management techniques, including treatment and discharge, wastewater reuse and land application;
4. Alternative methods for disposal of sludge and other residual waste, including process options and final disposal options;
5. Improving effluent quality through more efficient operation and maintenance;

(d) *Environmental impacts of the proposed action.* Primary and secondary impacts of the proposed action shall be described, giving special attention to unavoidable impacts, steps to mitigate adverse impacts, any irreversible or irretrievable commitments of resources to the project and the relationship between local short term uses of the environment and the maintenance and enhancement of long term productivity. The significance of land use impacts shall be evaluated, based on current population of the planning area; design year population for the service area; percentage of the service area currently vacant; and plans for staging facilities. Special attention should be given to induced changes in population patterns and growth, particularly if a project involves some degree of regionalization.

(e) *Steps to minimize adverse effects.* This section shall describe structural and nonstructural measures, if any, in the facilities plan to mitigate or eliminate significant adverse effects on the human and natural environments. Structural provisions include but are not limited to changes in facility design, size, and location; nonstructural provisions include but are not limited to staging facilities as well as developing and enforcing land use regulations and environmentally protective regulations.

(f) *Documentation.* Sources of information used to describe the existing environment and to assess future environmental impacts should be documented. In addition to the department, these sources should include regional, state and federal agencies with responsibility or interest in the types of impacts listed in par. (a). In particular, the following agencies should be consulted:

1. Local, and regional land use planning agencies and areawide waste treatment management planning agencies for assessments of land use trends and population projections, especially those affecting size, timing, and location of facilities;

2. The HUD regional office if a project involves a flood risk area identified under the Flood Disaster Protection Act of 1973 (Pub. L. 93-234);

3. The state coastal zone management agency, if a coastal zone is affected;

4. The secretary of the interior or secretary of agriculture, if a wild and scenic river is affected;

5. The secretary of the interior or secretary of commerce, if a threatened or endangered species is affected;

6. The fish and wildlife service (department of the interior), the department of commerce, and the U.S. army corps of engineers, if a wetland is affected.

(4) **PUBLIC HEARING.** Municipalities shall hold at least one public hearing before a facilities plan is adopted. A copy of the facilities plan should be available for public review before the hearing and at the hearing, since these hearings provide an opportunity for public comment on the issues associated with the facilities plan.

(5) **CONTENT OF AN INFILTRATION/INFLOW ANALYSIS.** (a) The infiltration/inflow analysis shall demonstrate whether or not excess infiltration/inflow exists in the sewer system. The analysis shall identify the presence, flow rate, and type of infiltration/inflow conditions, which exist in the sewer systems.

(b) For determination of the possible existence of excessive infiltration/inflow, the analysis shall include an estimate of the cost of eliminating the infiltration/inflow conditions. These costs shall be compared with estimated total costs for transportation and treatment of the infiltration/inflow. This determination shall be made at several levels of infiltration/inflow removal.

(c) If the infiltration/inflow analysis demonstrates the existence or possible existence of excessive infiltration/inflow and the specific sources of excessive infiltration/inflow have not been adequately identified, a sewer system evaluation survey shall be conducted in accordance with sub. (6). A detailed plan for the sewer system evaluation survey shall be included in the infiltration/inflow analysis. The plan shall outline the tasks to be performed in the survey and their estimated costs.

(d) The department may waive the requirements of pars. (a) through (c) if the owner can demonstrate to the department's satisfaction the obvious existence or nonexistence of excessive infiltration or inflow, or both. The information necessary for this demonstration may include infiltration and inflow estimates, per capita design flows, ratio of total flow to dry weather flow, cubic meters of infiltration per centimeter diameter per kilometer of pipe per day (gallons of infiltration per inch diameter per mile per

day), bypassing, and other hydrological and geological factors. The department may require the information be expanded to meet the requirements of pars. (a) through (c) if this demonstration is inconclusive.

(6) **CONTENT OF A SEWER SYSTEM EVALUATION SURVEY.** (a) The sewer system evaluation survey shall determine the location, estimated flow rate, method of rehabilitation and cost of rehabilitation versus cost of transportation and treatment for each defined source of infiltration/inflow.

(b) The report shall summarize the results of the sewer system evaluation survey. In addition, the report shall include:

1. A justification for each sewer section cleaned and internally inspected; and

2. A proposed rehabilitation program for the sewer system to eliminate all defined excessive infiltration/inflow.

(7) **CONSTRUCTION PLANS AND SPECIFICATIONS FOR SEWAGE TREATMENT PLANT PROJECTS.** In addition to the requirements of ch. NR 108 and ss. NR 110.06 and 110.07, the following requirements shall be adhered to for submission of plans for sewage treatment plants.

(a) *Overall plan.* A plan shall be submitted which shows the sewage treatment plant in relation to the remainder of the system. Sufficient topographic features shall be included to indicate its location with respect to streams and the point of discharge of treated effluent.

(b) *Layout.* A general layout plan shall be submitted which includes:

1. A contour map of the site;

2. The size and location of plant structures;

3. A schematic flow diagram indicating the various plant units;

4. Piping details including piping arrangements for bypassing individual units;

5. The materials handled and the direction of flow through each pipe;

6. The hydraulic profiles for sewage and sludge flows;

7. Soil conditions at the site.

(c) *Detailed plans.* Detailed construction plans shall be submitted which include:

1. The location, dimensions, elevations and details of all existing and proposed plant units;

2. The elevation of high and low water level in the receiving stream;

3. An adequate description of all features not covered in the specifications.

(8) **ADDITIONAL FACILITY PLANNING REQUIREMENTS FOR LAND DISPOSAL SYSTEM ALTERNATIVES.** (a) *General requirements.* In addition to the requirements of sub. (1), a report including a soil investigation and a hydrogeologic evaluation shall be submitted as part of the facilities plan for a land disposal discharge alternative. The report shall detail the soil types, characteristics, variability and permeability, topography, groundwater conditions and quality, and other characteristics of the disposal site. Soil boring and test pit logs and soil analyses shall be provided. Wastewater characteristics which may influence the design of the disposal system shall also be discussed. Water supply quality, local groundwater use, and potential impacts of the facility on groundwater quality shall be included.

(b) *Hydrogeological investigation.* 1. A hydrogeological investigation shall be included as part of the facilities plan. The analysis of the hydrogeological information shall be done by a hydrogeologist, or other qualified person. The investigation shall

include both regional and site-specific hydrogeological information.

**Note:** The skills and knowledge required of a hydrogeologist making submittals under this chapter include: the ability to apply hydrogeologic principles and practices to the siting, design and operation of land disposal systems; knowledge of contaminants associated with land disposal of wastewater, their transport mechanisms and fate in the environment; familiarity standards; and proficiency in the design of groundwater monitoring systems for defining the physical and chemical characteristics of groundwater flow. A soil scientist or other environmental scientist who can demonstrate the above skills and knowledge, as reflected in submittals made under this chapter, shall be deemed a "qualified person".

2. The following site-specific groundwater information may be required as part of the facilities plan for land disposal facilities:

- a. Depth to highest anticipated groundwater elevation.
- b. Groundwater flow directions and rates of flow.
- c. Vertical and horizontal gradients.
- d. Groundwater quality.
- e. Presence of groundwater divides and barriers.
- f. Presence and extent of perched groundwater.
- g. Mounding calculations.

(c) *Soil investigation.* The soil evaluation may be performed in conjunction with the hydrogeological evaluation; however, each evaluation shall be performed by a person who is qualified to perform the evaluation. The following site-specific soil information shall be submitted as a part of the facilities plan for land disposal systems;

1. Soil borings and sampling performed in accordance with ss. NR 110.24 (3) (d) and 110.24 (4) (d), and test pit analyses. The one boring per acre minimum of s. NR 110.24 (3) (d) 4. does not apply to spray irrigation, ridge and furrow, or overland flow systems. A soil analysis may be required on a case-by-case basis for land disposal systems. The USDA soil classification system shall be used for spray irrigation systems, ridge and furrow, and overland flow systems.

2. Soil descriptions, including soil profile, stratification, slope, soil moisture content, continuity, structure, texture, relative density and depth to groundwater and bedrock.

3. Soil analyses shall be performed on the zone of soil which will provide treatment of the wastewater. The department may require that the analyses include any or all of the following: grain size analyses, hydrometer analyses, field and laboratory horizontal and vertical permeabilities, Atterberg limits, soil pH, cation exchange capacity, bulk density and relative density, porosity, soil nutrient content, and organic matter content.

4. In-field infiltration rates (measured at the proposed elevation of application).

5. A description of the soil testing methods used.
6. Depth to bedrock.
7. Type and nature of bedrock.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register December, 1978, No. 276, eff. 1-1-79; am. (1) (b) 4. and (2) (h), cr. (2) (n) to (q) and (5) (d), Register, February, 1983, No. 326, eff. 3-1-83; am. (2) (h) 5. and 6., cr. (2) (r) and (8), Register, November, 1990, No. 419, eff. 12-1-90; am. (2r), Register, April, 1994, No. 460, eff. 5-1-94; am. (2) (r), Register, March, 1995, No. 471, eff. 4-1-95; correction in (1) (j) 2. made under s. 13.93 (2m) (b) 1., Stats., Register, September, 1995, No. 477; corrections in (2) (f), (j) 1., 4. (intro.), and (4) (c) 1., were made under s. 13.93 (2m) (b) 7., Stats., Register, May, 2001, No. 545; CR 09-123: am. (2) (j) 2., 3. b., 4. b. and (8) (b) 2. Register July 2010 No. 655, eff. 8-1-10; CR 13-022: am. (3) (title), (intro.) Register March 2014 No. 699, eff. 4-1-14.

**NR 110.10 Sewage collection system projects. (1) FACILITIES PLANS FOR SEWER PROJECTS.** For sewer projects the facilities plan shall include the following information:

(a) *Description.* A brief description of the project; including its geographic location and any necessary reference maps or exhibits.

(b) *Topography.* A brief description of the topography of the general area with specific reference to the area serviced by the proposed sewer.

(c) *Soil investigations.* A description of the extent of soil investigations, including information on rock likely to be encountered. In addition, that portion of the proposed sewer which is below high ground water level shall be indicated.

(d) *Flooding.* A designation of any portion of the proposed sewer which is located within the floodway or floodplain as defined in ch. NR 116. All projects shall conform to the requirements of ch. NR 116.

(e) *Wetlands.* A statement indicating whether the proposed sewer will pass through a wetlands area, and the approximate acreage of the wetland.

(f) *Population.* Population growth rate (annual) based on the most recent data for the municipality.

(g) *Sewer service area.* If the sewer project is tributary to a treatment plant for which a service area that has been delineated as a part of an approved areawide waste treatment management plan, indicate the location of the sewer on a map of the service area.

(h) *Downstream overflows.* A description of the number and location of sanitary sewer overflow structures and a description of the occurrence of sanitary sewer overflow events at any location within the sewerage system.

(i) *Description of treatment facilities.* A brief description of the type of treatment facility indicating the ability of the facility to handle the sewage of the proposed project during both wet and dry weather conditions.

(j) *Costs.* A discussion of the estimated capital costs and where an entire sewerage system is being installed, the estimated annual cost to the user of the system.

(k) *Basis of design.* The following data shall be provided for the proposed project:

1. Design period.
2. Population densities per acre and total population served.
3. Area served by proposed sewers in acres.
4. Per capita sewage contribution expressed as an average and maximum value, include basis for this determination.
5. Infiltration.
6. Industrial waste contribution.
7. Design flow rates as expressed as average and maximum values.
8. Size of pipe, grade, velocity and maximum capacity.

(L) *Environmental analysis.* The department may require the submittal of an environmental analysis meeting the requirements of s. NR 110.09 (3) for large or complex sewer projects, for those projects which are proposed to be constructed in environmentally sensitive areas, or for projects which involve significant public controversy.

**(2) STAGING OF INTERCEPTORS.** (a) Since the location and length of interceptors will influence growth, interceptor routes and staging of construction shall be planned carefully and shall be consistent with approved areawide waste treatment management plans, growth management plans and other environmental laws.

(b) The staging period for interceptor construction and interceptor pipe sizes shall be cost-effective. A 20 year staging period shall be analyzed. Other staging periods, not to exceed 40 years, may also be evaluated. The cost-effectiveness analysis shall consider the following factors:

1. Cost.



- a. Capital cost (present and future).
- b. Operation and maintenance cost.
- c. Salvage value.
2. Primary environmental impacts.
  - a. Short-term disruption of traffic, business and other activities.
  - b. Destruction of flora and fauna.
  - c. Noise, erosion and sedimentation.
  - d. Destruction of, or impact on, wetlands and floodplains.
3. Secondary impacts.
  - a. Pressure to rezone or otherwise stimulate unplanned development.
  - b. Pressure to accelerate growth for quicker recovery of the nonfederal share of the interceptor investments.
  - c. Effects on air quality and environmentally sensitive areas by cultural changes.
4. Other nonmonetary impacts including implementation capability, operability, performance reliability and flexibility.
  - (c) The estimation of peak flows in interceptors shall be based upon the following considerations:

1. Daily and seasonal variations of pipe flows, the timing of flows from the various parts of the tributary area and pipe storage effects.
2. The feasibility of off-pipe storage to reduce peak flows.
3. The use of an appropriate peak flow factor that decreases as the average daily flow to be conveyed increases.

**(3) CONSTRUCTION PLANS AND SPECIFICATIONS FOR SEWER PROJECTS.** In addition to the requirements of ch. NR 108 and ss. NR 110.06 and 110.07 plans and specifications for proposed sewer projects shall include a plan and profile view of all proposed construction. The plans and profiles shall show:

- (a) *Location.* The location of existing or proposed streets and sewers;
- (b) *Streams and water surfaces.* The location and 100 year flood elevation of all streams and water surfaces relevant to the project;
- (c) *Elevations.* The line of the ground surface, the invert and surface elevation at each manhole and the grade of the sewer between each adjacent manhole. Basement elevations shall be noted on the plans or the designing engineer shall state that all sewers are sufficiently deep to serve adjacent basements except where otherwise noted on the plans. Where gravity basement drainage to the proposed sewer will not be possible for existing buildings, the buildings' owners shall be so advised prior to construction of the sewers;
- (d) *Pipe size and material.* The pipe size, material, pipe strength and bedding class shall be shown on the plans or in the specifications;
- (e) *Manhole spacing.* The length of sewer between the manholes shall be shown on the plans;
- (f) *Special features.* The locations of all special features including inverted siphons, concrete encasements, elevated sewers, and other features as appropriate;
- (g) *Existing structures.* The location of all known existing structures and utilities which might interfere with the proposed construction, particularly all water mains, gas mains, storm drains, and other pertinent structures;
- (h) *Special drawings.* Special detail drawings made to a scale to clearly show the nature of the design shall be furnished to show the following:

1. Stream crossings with elevations of the stream bed and of normal and extreme high and low water levels;

2. Details of all special sewer joints and cross-sections;
3. Details of all sewer appurtenances such as manholes, lampholes, inspection chambers, inverted siphons and elevated sewers.

**(4) SYSTEM EVALUATION AND CAPACITY ASSURANCE PLAN.** A system evaluation and capacity assurance plan shall include all the following:

(a) An evaluation of those portions of the sewage collection system that may contribute to sewage treatment facility overflows or other noncompliance at a sewage treatment facility, or that are experiencing or contributing to a sanitary sewer overflow caused by excessive infiltration and inflow or a system hydraulic deficiency. The evaluation must provide estimates of peak flows, including the amount from sanitary sewer overflows and sewage treatment facility overflows, provide estimates of the capacity of key system components, identify hydraulic deficiencies, and identify the sources (including private property sources) of infiltration and inflow that contribute to the peak flows associated with sanitary sewer overflow or sewage treatment facility overflow occurrences.

(b) An analysis to identify actions that will eliminate sanitary sewer overflows and sewage treatment facility overflows or abate their occurrence and effects on public health and the environment to the extent technically and economically feasible. The analysis shall consider alternatives such as providing improved operation and maintenance, infiltration and inflow reduction, and removal from all sources, wastewater equalization or storage facilities, sewer and lift station replacement or rehabilitation, the treatment of overflows, peak flow treatment schemes at sewage treatment facilities, expansion of sewage treatment facility capacity, and any other construction of new or modified sewerage system components.

**Note:** When evaluating feasibility of alternatives, the department may consider factors such as technical achievability, the relationship between the control of storm water and the control of infiltration/inflow into the sewage collection system, costs and affordability of implementation, and risks to public health, the environment, and welfare of the community served by the sewage collection system.

(c) Identification of specific short and long term corrective actions. Schedules for implementation shall be established giving greatest priority to those actions that will protect public health and minimize environmental risk. The department may establish compliance schedules in WPDES permits to implement specific actions identified under this paragraph.

**Note:** Portions of a system evaluation and capacity analysis plan may include results from an infiltration/inflow analysis or a sewer system evaluation survey under s. NR 110.09 (5) or s. NR 110.09 (6), respectively.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, December, 1978, No. 276, eff. 1-1-79; r. and recr. (2), Register, February, 1983, No. 326, eff. 3-1-83; CR 12-027: r. and recr. (1) (h), cr. (4) Register July 2013 No. 691, eff. 8-1-13; CR 13-022: am. (1) (L) Register March 2014 No. 699, eff. 4-1-14.

**NR 110.11 Sewage lift stations. (1) FACILITIES PLANS FOR SEWAGE LIFT STATIONS.** The facilities plan shall include the following:

(a) *Contributory area.* A description of the extent of the existing and proposed contributory area with reference to a general system map as well as description of the areas of probable future expansion of the contributory area.

(b) *Location.* The location of the proposed lift station, force main and point of discharge to the existing sewer system. In addition the report shall discuss the capacity available in the existing downstream sewer to handle the additional flow.

(c) *Flooding.* A statement indicating whether the proposed lift station is in a floodway or floodplain as defined in ch. NR 116. All projects shall conform to the requirements of ch. NR 116.

(d) *Basis for design.* The design data for the proposed project including the following:

1. Design period;
2. Population densities per acre and total population served;
3. Area served in acres;
4. Per capita sewage contribution expressed as an average and as a maximum value;
5. Infiltration and inflow;
6. Industrial waste contributions;
7. Design flow rates expressed as average and maximum values; and
8. Design head conditions.

(e) *Essential features.* A description of the essential features of construction and operation of the proposed stations.

(f) *Costs.* Discussion of the estimated capital costs, estimated annual maintenance cost, and estimated annual cost to the average user of the system;

(g) *Environmental analysis.* The department may require the submittal of an environmental analysis meeting the requirements of s. NR 110.09 (3) for large or complex lift station projects, for those projects constructed in environmentally sensitive areas or for projects which could involve significant public controversy.

**Note:** Facilities plans for sewage lift stations may include results from a system evaluation and capacity assurance plan under s. NR 110.10 (4).

**(2) CONSTRUCTION PLANS AND SPECIFICATIONS FOR SEWAGE LIFT STATIONS.** In addition to the requirements of ch. NR 108 and ss. NR 110.06 and 110.07, the following requirements shall be adhered to for submission of construction plans and specifications for sewage lift stations:

(a) *Location plan.* A location plan shall be submitted showing the tributary area, the municipal boundaries within the tributary area and the location of the lift station and force main, and all pertinent elevations;

(b) *Detailed plans.* The detailed lift station plans shall show the following, where applicable:

1. The location and the topography (using a contour map) of the property to be used;
2. The station details and all appurtenant equipment including pumps, sump pumps, heaters, ventilation equipment, valving, access ladder, intermediate landings, and wet well;
3. The elevation of high water at the site, including the maximum elevation of sewage in the collection system in the event of power failure at the station.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, December, 1978, No. 276, eff. 1-1-79; CR 12-027: am. (1) (d) 5. Register July 2013 No. 691, eff. 8-1-13; CR 13-022: am. (1) (g) Register March 2014 No. 699, eff. 4-1-14.

**NR 110.12 Owner approval requirement.** If the engineer submitting plans to the department for approval is not an employee of or has not been retained by the owner of the sewerage system for which the plans are submitted, written acceptance of the final plans by the owner shall be required prior to submission of the plans to the department.

**Note:** For example, if an engineer is retained by a developer to design sewer extensions which will be connected to a municipal system and which will eventually be owned by the municipality, the plans must be accepted by the municipality before the department issues an approval.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74.

**NR 110.13 Sewer design criteria. (1) GENERAL DESIGN CONSIDERATIONS.** (a) *Separation of sewers.* New systems, or extensions to existing systems, which will serve presently unsewered areas shall be designed to exclude storm and other clear water sources from the sanitary sewer system.

(b) *Design basis.* 1. Sewage collection systems, exclusive of interceptors, shall be designed in accordance with s. NR 110.10 (1).

2. Interceptors shall be designed in accordance with s. NR 110.10 (2) (a).

3. Extensions to existing sewage collection systems may be designed assuming an average design flow rate of 378 liters (100 gallons) per capita per day.

(c) *Design capacity.* Sewers shall be designed to carry, when running full, the peak design flows expected from domestic, commercial, industrial and other sources, and infiltration and inflow. Peak design flow shall be established using existing sewage flow or water use records, and records of infiltration and inflow. Where peak flow records are not available, the peak design flow shall be determined by applying one of the following peak flow factors to the average design flow:

1. 250% of the average design flow for interceptors, main (trunk) sewers, and sewage outfall pipes; or,

2. 400% of average design flow for submain and branch sewers.

(d) *Protection of water supplies.* 1. Sanitary sewers shall be located with a minimum separation distance of 60 meters (200 feet) from any community water system well in accordance with ch. NR 811. A lesser separation distance may be allowed where the sanitary sewer main is constructed of water main materials and joints and pressure tested in accordance with ch. NR 811 requirements. When sanitary sewers are proposed to be laid within 60 meters (200 feet) of a community water system well the location of the well shall be shown on the design plans. The separation distance between a community water system well and a sanitary sewer main may not be less than 50 feet.

2. Sanitary sewers shall be located with a minimum separation distance of 15 meters (50 feet) from private water system wells or any other wells subject to ch. NR 812.

3. Horizontal and vertical separation of sewers from public water mains shall comply with the requirements of s. NR 811.67. Horizontal and vertical separation of sewer from private water laterals shall comply with the requirements of s. SPS 382.40 (8) (b).

4. Cross-connections with public and private water supply systems are prohibited.

**(2) DESIGN REQUIREMENTS.** (a) *Diameter.* 1. Conventional gravity sewer may not be less than 20 centimeters (8 inches) in diameter.

2. Gravity sewers with diameters less than 20 centimeters (8 inches) which are intended to transport septic tank effluent will be evaluated on a case-by-case basis.

(b) *Depth.* Sewers shall be designed deep enough to prevent freezing and, where economically feasible, to provide gravity basement drainage for sanitary wastes.

(c) *Slope.* 1. Conventional gravity sewers shall be laid with uniform slope between manholes. All sewers shall be designed and constructed to give average velocities of not less than 60 centimeters per second (2.0 feet per second) when flowing full. The minimum slopes shown in Table 1 shall be provided. Slopes less than 0.4% may be permitted for 20 centimeter (8 inch) sewers. In such cases, however, the slope may not be less than 0.3%. The department will approve these sewers only when the owner demonstrates that physical circumstances warrant the lesser slope. Furthermore, approval will not be granted until the department has received written assurance from the operating authority that the authority will provide the additional maintenance which may result from the sedimentation due to decreased velocities.

Table 1

| Sewer Size  | Minimum Slope (ft./100 ft.) |
|-------------|-----------------------------|
| (20 cm) 8"  | 0.40                        |
| (25 cm) 10" | 0.28                        |
| (30 cm) 12" | 0.22                        |
| (38 cm) 15" | 0.15                        |
| (46 cm) 18" | 0.12                        |
| (53 cm) 21" | 0.10                        |
| (61 cm) 24" | 0.08                        |

2. Gravity sewers with a diameter less than 20 centimeters (8 inches) shall be laid at uniform slopes between manholes and shall be designed to provide sufficient flow velocities to prevent sedimentation of septic tank solids.

(d) *Alignment.* 1. Sewers with diameters less than 91 centimeters (36 inches) shall be laid with straight alignment between manholes.

2. The department may approve curvilinear sewers with diameters of 91 centimeters (36 inches) or larger on a case-by-case basis.

(e) *Increasing size.* When a sewer joins a larger one, the invert of the smaller sewer shall be laid at the elevation necessary to maintain the same energy gradient.

(f) *Velocity.* Where velocities of greater than 4.6 meters per second (15 feet per second) are attained, special provision shall be made to protect against displacement or erosion.

(g) *Anchoring.* Sewers on slopes of 20% or greater shall be anchored securely with concrete anchors or the equivalent, spaced as follows:

1. Not over 11 meters (36 feet) center to center on grades 20% to 35%;

2. Not over 7.3 meters (24 feet) center to center on grades 35% to 50%; and

3. Not over 4.9 meters (16 feet) center to center on grades greater than 50%.

(h) *Trench width.* The width of the trench shall be sufficient to allow the pipe to be laid and jointed properly and to allow the backfill to be placed and compacted as needed. The trench sides shall be kept as nearly vertical as possible. When wider trenches are dug, appropriate bedding class and pipe strength shall be used. Ledge rock, boulders, and large stones shall be removed to provide a minimum clearance of 10 centimeters (4 inches) below and on each side of the pipe.

(i) *Bedding.* 1. Bedding classes A, B, or C, as described in ASTM C12-09 shall be used for all rigid pipe provided the proper strength pipe is used with the specified bedding to support the anticipated load.

2. Bedding classes I, II, or III, as described in ASTM D2321-09 shall be used for all flexible pipe provided the proper strength pipe is used with the specified bedding to support the anticipated load.

(j) *Backfill.* Debris, frozen material, large clods or stones, organic matter, or other unstable materials may not be used for backfill within 60 centimeters (2 feet) of the top of the pipe. Backfill shall be placed in such a manner as not to disturb the alignment of the pipe.

(k) *Construction quality testing.* 1. Groundwater infiltration into sanitary sewer systems shall be minimized. Tests for infiltration shall be specified in the construction specifications. This may include appropriate water or low pressure air testing. The leakage outward or inward (exfiltration or infiltration) may not exceed 0.19 cubic meters per centimeter pipe diameter per kilo-

meter per day (200 gallons per inch of pipe diameter per mile per day) for any section of the system. An exfiltration or infiltration test shall be performed with a minimum positive head of 60 centimeters (2 feet). The air test, if used, shall, at a minimum, conform to the test procedure described in ASTM C828-06 for clay pipe, ASTM C924-02 (2009) for concrete pipe, or ASTM F1417-92 (2005) for plastic pipe. The testing methods selected should take into consideration the range in groundwater elevations projected and the situation during the test.

2. Deflection tests shall be performed for all polyvinyl chloride pipe installations. The deflection test shall be performed using a rigid ball or mandrel, and shall be performed without mechanical pulling devices. If deflection testing occurs within 30 days of placement of the final backfill, deflection may not exceed 5%. Maximum deflection may not exceed 7.5% when testing occurs more than 30 days after placement of the final backfill.

(3) **MANHOLES.** (a) *Location.* Manholes shall be installed at the end of each line, at all changes in grade, size or alignment, and at all pipe intersections.

(b) *Manhole spacing.* 1. Manholes shall be located at intervals not greater than 120 meters (400 feet) for sewers with diameters of 38 centimeters (15 inches), or less, and not greater than 150 meters (500 feet) for sewers with diameters of 46 centimeters (18 inches) to 76 centimeters (30 inches). Distances up to 180 meters (600 feet) may be approved in cases where the sewer system owner has cleaning equipment which can reach this length.

2. Manhole spacing for sewers with a diameter greater than 76 centimeters (30 inches) shall be determined on a case-by-case basis.

(c) *Drop pipe.* An outside drop pipe shall be provided for a sewer entering a manhole where the invert elevation of the entering sewer is 60 centimeters (2 feet) or more above the spring line of the outgoing sewer. The entire drop connection shall be encased in the concrete. Inside drop connection may be approved on a case-by-case basis.

(d) *Diameter.* The minimum diameter of manholes shall be 1.1 meters (42 inches).

(e) *Construction.* Manholes shall be constructed of precast concrete, monolithic concrete, brick or block, or other approved materials. Fiberglass manholes may be approved on a case-by-case basis. Fiberglass manholes may be approved for use in high traffic areas provided the top section of the manhole is not made of fiberglass.

(f) *Flow channel.* The flow channel through manholes shall be made to conform to the shape and slope of the sewers.

(g) *Water tightness.* Solid watertight manhole covers shall be used wherever the manhole tops may be flooded by street runoff or high water. Where groundwater conditions are unfavorable, manholes of brick or block shall be waterproofed on the exterior with plastic coatings supplemented by a bituminous waterproof coating or other approved coatings. Inlet and outlet pipes shall be joined to the manhole with a gasketed flexible watertight connection or any watertight connection arrangement that allows differential settlement of the pipe and manhole wall to take place.

(h) *Cleanouts.* For sewers with diameters 20 centimeters (8 inches), or greater, cleanouts and lampholes may not be used as substitutes for manholes. The department may allow cleanout instead of manholes when the sewer diameter is less than 20 centimeters (8 inches).

(i) *Manholes for sewers with diameters less than 20 centimeters (8 inches).* Manholes shall be located at pipe intersections. The spacing of these manholes shall be determined on a case-by-case basis.



(4) **INVERTED SIPHONS.** Inverted siphons may not have less than 2 barrels with a minimum pipe size of 15 centimeters (6 inches) and shall be provided with the necessary appurtenances for convenient flushing and maintenance. The manholes shall have adequate clearance for rodding. Sufficient head shall be provided and pipe sizes selected to secure velocities of at least 91.5 centimeters per second (3.0 feet per second) at average flows. The inlet and outlet details shall be arranged so that the normal flow is diverted to one barrel, and so that either barrel may be removed from service for cleaning.

(5) **MATERIAL SPECIFICATIONS FOR SEWER CONSTRUCTION.**  
(a) *Materials.* Materials used in the construction of sanitary sewers shall be restricted to the following: concrete, vitrified clay, steel, ductile iron, polyvinyl chloride (PVC), acrylonitrile-butadiene-styrene (ABS) composite, fiberglass reinforced-PVC composite, and high density polyethylene (HDPE) pipe. Other pipe material will be considered on its merit and may be approved by the department. Where an approval is issued for a restricted or experimental use, the department may require a construction inspection report and annual reports including television inspection of the system as a condition of its approval.

(b) *Quality.* All material used for sanitary sewer construction shall be free from defects that impair service.

(c) *Labeling.* Each length of pipe and fitting used in a sanitary sewer shall be stamped or indelibly marked with the manufacturer's name or mark.

(d) *Material selection.* Pipe material selection shall recognize the design conditions of the sewer installation. Factors which shall be considered include depth of cover, soil types, loading on pipe, and corrosivity.

(e) *Nonpressure pipe.* All nonpressure sewer pipe shall have sufficient strength to withstand the loads which will exist. The following are minimum standards for nonpressure pipe:

1. Concrete pipe shall meet the requirements of ASTM C14-07, C76-10, or C655-09;
2. Vitrified clay pipe shall meet the requirements of ASTM C700-09;
3. Steel pipe shall meet the requirements of AWWA C200-05;
4. Ductile iron pipe and fittings shall meet the requirements of ASTM A746-09;
5. Polyvinyl chloride sewer pipe shall meet the requirements of D3034-08 or ASTM F679-08;
6. ABS composite sewer pipe shall meet the requirements of ASTM D2680-01(2009).

(f) *Joints for nonpressure pipe.* The method of making joints and the materials used shall be included in the specifications and meet the minimum standards in subs. 1. to 5. Sewer joints shall be designed to minimize infiltration and to prevent the entrance of roots. Joint material shall be of such a composition as not to be adversely affected by the sewage.

1. Rubber gasket joints for concrete sewer pipe shall meet ASTM C443-05ae1.
2. Resilient joints for vitrified clay sewer pipe shall meet ASTM C425-04 (2009).
3. Steel pipe joints shall meet the requirements of AWWA C200-05.
4. Polyvinyl chloride sewer pipe shall be joined by solvent weld joints or by elastomeric joints which have been approved by the department.
5. ABS composite sewer pipe shall be joined by solvent weld joints or by type OR mechanical-seal joints meeting the requirements of ASTM D2680-01 (2009).

(g) *Pressure sewer pipe and joints.* All pressure sewer pipe 10 centimeters (4 inches) or larger shall meet the following minimum requirements:

1. Ductile iron pipe and joints shall meet the requirements of AWWA C151.
2. Steel pipe and joints shall meet the requirements of AWWA C200-05.
3. Concrete pipe and joints shall meet the requirements of AWWA C300-04.
4. Polyvinyl chloride pipe and joints shall meet the requirements of AWWA C900-07 (minimum class 150) or ASTM D2241-09 (minimum class 250). Solvent weld joints may not be used.
5. Fiberglass reinforced-polyvinyl chloride composite pipe and joints shall meet the requirements of AWWA C950-07 (minimum class 250). Eight and 10-inch pipe shall have minimum category 3 stiffness as defined in ASTM D2996-01 (2007) e1. Four and 6-inch pipe shall have a minimum category 2 stiffness as defined in ASTM D2996-01 (2007).

(h) *Small diameter pressure sewer pipe and joints.* All pipe and joints 8 centimeters (3 inches) in diameter or smaller to be used in grinder pumps shall meet the following minimum requirements:

1. Polyethylene pipe and joints which meet the requirements of ASTM D2239-03 (minimum class 160) may be approved on a case-by-case basis depending on the expected system pressure relative to the pipe working strength. Solvent weld, butt fusion, or elastomeric joints will be acceptable.
2. For ABS pipe, solvent weld or elastomeric joints will be acceptable.
3. Polyvinyl chloride pipe and joints shall meet the requirements of ASTM D2241-09 (minimum class 160). Solvent weld or elastomeric joints will be acceptable.

(6) **SANITARY SEWER OVERFLOW STRUCTURES.** Sanitary sewer overflows structures may be provided as measures to manage and mitigate the effects of sanitary sewer overflow discharges that may occur under extreme conditions. Sanitary sewer overflow structures shall be designed in accordance with all the following requirements:

(a) The overflow may be activated either manually or automatically. If automatically activated, a monitoring system shall be provided to detect the initiation time of the overflow and to provide an alarm signal to the sewage collection system operator or other responsible authority.

(b) The overflow structure shall be designed to discharge only those wastewater flows greater than the peak flow conveyance capacity within the sewage collection system.

(c) Equipment shall be provided to measure the flow and, if practicable, sample the wastewater discharged from the structure.

**Note:** A department approval of a sanitary sewer overflow structure does not eliminate or alleviate the requirement that prohibits sewage treatment facility overflows in s. NR 210.21.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; am. (2), Register, December, 1978, No. 276, eff. 1-1-79; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83; corrections in (1) made under s. 13.93 (2m) (b) 7., Stats., Register, September, 1995, No. 477; corrections in (1) (d) 2. and 3. were made under s. 13.93 (2m) (b) 7., Stats., Register, May, 2001, No. 545; CR 09-123; am. (1) (d) 1., 2., (2) (i), (k) 1., (5) (a) and (e) to (h) Register July 2010 No. 655, eff. 8-1-10; correction in (1) (d) 3. made under s. 13.92 (4) (b) 7., Stats., Register January 2012 No. 673; CR 12-027; cr. (6) Register July 2013 No. 691, eff. 8-1-13.

**NR 110.14 Sewage lift stations design criteria. (1) GENERAL.** (a) *Applicability.* Lift stations may be approved when gravity sewers are not feasible or economical to transport the same design quantities of sewage.

(b) *Design report.* A design report shall be submitted with plans and specifications for all new sewage lift stations as well as the major rehabilitation of existing lift stations. Major lift station rehabilitation may include, but is not limited to, replacing pumps with larger units or changing the type of lift stations. The design report shall comply with the facilities planning requirements of s. NR 110.11, and shall contain the detailed design calculations for the lift station design capacity.

(2) DESIGN CONSIDERATIONS. (a) *Location.* 1. Lift stations may be constructed in floodplains provided the floodproofing requirements of ss. NR 116.16 and 116.17 are met.

2. Where practical, lift stations shall be located off the traffic way of streets and alleys.

3. Lift stations shall be located with a minimum separation distance of 60 meters (200 feet) from community water system well, and a minimum separation distance of 30 meters (100 feet) from a private water well or any other well subject to ch. NR 812. A lesser separation distance from a community water system well may be approved if hydrogeologic information is provided to the department to indicate the lesser separation distance would provide adequate protection of a well from contamination. When a lift station is proposed within 60 meters (200 feet) of a community water system well, or 30 meters (100 feet) of private water wells or any other well subject to ch. NR 812, the location of the well shall be shown on the engineering plans. Gravity or pressure sewers connecting to lift stations shall be separated from water supply wells in accordance with s. NR 110.13 (1) (d).

(b) *Design capacity.* 1. Pumping rates for lift stations integral to collection systems shall be determined in the same manner as the flows for the sewers contributory to the lift station and in accordance with the provisions of s. NR 110.11 (1) (d).

2. Pumping rates for lift stations which operate as part of sewage treatment facilities shall be determined in the same manner as the design flow for the treatment facility in accordance with s. NR 110.15 (4) (c).

3. Where possible, the pumping rate shall be designed to approximate the peak hour influent design flow rate to the lift station. For main lift stations or lift stations associated with treatment facilities, or in cases where large fluctuations of flow are known to occur, the use of variable speed pumps, or multiple constant speed pumps may be required by the department.

(3) GENERAL DESIGN REQUIREMENTS. (a) *Type.* Sewage lift stations in general use fall into 7 types: wet well/dry well, submersible, suction lift, screw pump, pneumatic ejector, grinder pump and septic tank effluent pump.

(b) *Structural features.* 1. Dry wells, including their superstructure, shall be completely separated from wet wells. Common walls shall be gas tight.

2. Provisions shall be made in all types of lift stations to facilitate removal of pumps, motors, and other mechanical and electrical equipment without entry into the wet well.

3. Permanent ladders or steps may not be provided in the wet wells with the possible exception of built-in place lift stations, in which stairways in the wet wells may be approved if there are special maintenance needs or physical conditions that prevent the provision of necessary access by any other reasonable means. A safe means of access shall be provided to dry wells containing equipment requiring inspection or maintenance. If a dry well is over 6 meters (20 feet) deep, an offset shall be made in the entrance ladder with an intermediate landing at approximately mid-depth. Where an intermediate landing is used, the diameter of the landing area shall be at least 1.5 meters (5 feet), or an equivalent landing area shall be provided. Landings shall be provided

with a suitable barrier to prevent an individual from falling past the intermediate landing to the lower level.

4. A caution sign shall be installed at top of entrances to wet wells. The caution sign shall provide a warning of the potential for hazardous gases in a confined space and indicate that there shall be no entry without proper equipment and supervision.

5. A sump pump shall be provided in a dry well to remove leakage or drainage. The sump pump discharge line shall be equipped with a check valve, and shall discharge above the maximum high water level of the wet well. A siphon break shall be provided when the sump pump discharge line enters at the high water level in the wet well. Pump seal water leakage shall be piped or channeled directly to the sump pit.

6. All floors and walkways shall be sloped to a point of drainage.

7. All wet wells shall be designed based on fill time and minimum pump cycle time. With any combination of influent flows and pumping rate, the minimum pump cycle time shall be greater than or equal to 5 minutes. The total fill time between pump on and off elevations in the wet well, at average design flow, may not exceed 30 minutes to prevent septicity.

8. The wet well floor shall have a minimum slope of one to one to the hopper bottom. The horizontal area of the hopper bottom may not be greater than necessary for proper installation and function of suction pipe intake or pump inlet.

9. There may not be a connection between any potable water system and sewage lift station which could potentially cause contamination of the potable water system.

10. Exteriors of steel factory built lift stations shall be provided with cathodic protection against corrosion.

11. Interior of steel wet wells shall be coated with a suitable water proof epoxy coating or water proof painting system or other appropriate methods to protect against corrosion.

(c) *Ventilation.* 1. All covered wet wells shall be vented to the atmosphere using an inverted "j" tube or other means. Adequate ventilation shall also be provided for all dry wells. Where the dry well is below the ground surface, permanent mechanical ventilation shall be provided.

2. A permanent mechanical ventilation system shall be provided in wet wells and submersible lift stations where routine entrance is required to inspect or maintain equipment. In all other cases, portable mechanical ventilation equipment shall be available for wet wells as required for entry to a confined space.

3. There shall be no interconnection between the wet well and dry well ventilation systems. Switches for operation of ventilation equipment shall be marked and conveniently located. All intermittently operated ventilating equipment shall be interconnected with the respective wet well or dry well lighting system. Consideration shall be given to automatic controls where intermittent operation is used. The manual lighting and ventilation switches shall override the automatic controls.

4. The fan wheel for ventilating hazardous areas shall be fabricated from nonsparking material.

5. Mechanical ventilation for wet wells shall provide at least 12 complete air changes per hour if ventilation is continuous and at least 30 complete air changes per hour if ventilation is intermittent. Air shall be forced into the wet well by mechanical means rather than exhausted from the wet well.

6. Mechanical ventilation for dry wells shall provide at least 6 complete air changes per hour if ventilation is continuous and at least 30 complete air changes per hour if ventilation is intermittent. For conserving heat in large lift stations, the department may approve the following 2 exceptions:

a. Intermittent ventilation with an initial ventilation rate of 30 complete air changes per hour for 10 minutes and automatic switch over to 6 complete air changes per hour.

b. A continuous ventilation system at a rate of 6 complete air changes per hour when the dry well is occupied and at a rate of 2 complete air changes per hour when not occupied.

(d) *Auxiliary equipment.* All of the following auxiliary equipment shall be installed in lift stations:

1. All dry wells shall be equipped with automatic heaters. The department may waive this requirement if it can be demonstrated that the heat output from the pump motors or controls is sufficient to keep equipment in the dry well from freezing.

2. The installation of dehumidifiers shall be considered for all underground dry wells.

3. Running time meters shall be installed for each pump in all lift stations. Where the department determines that flow measurement is necessary for the proper operation of the collection system or treatment system, suitable devices for measuring, totalizing, and recording flow shall be installed.

(e) *Electrical equipment.* Electrical systems and components including motors, lights, cables, conduits, switchboxes, and control circuits, which will be located in wet wells, or in enclosed or partially enclosed spaces where hazardous concentrations of flammable gases or vapors may be present, shall comply with the NEC requirements for Class 1, Group D, Division 1 locations. In addition, equipment located in the wet well shall be suitable for use under corrosive conditions. Each flexible cable shall be provided with a watertight seal and separate strain relief. A fused disconnect switch or equivalent circuit breaker located above ground shall be provided for the main power feed for all lift stations. When the equipment is exposed to weather, it shall meet the requirements of weatherproof equipment.

(f) *Duplicate units.* At least 2 pumps or pneumatic ejectors shall be provided in each lift station. Each pump or ejector shall be capable of pumping the design pumping rate as determined by sub. (2) (b). If 3 or more pumps are provided, they shall be designed to meet expected flow conditions and shall be capable of pumping the peak hour design pumping rate as determined by sub. (2) (b), with the largest unit out of service. Where the lift station will serve not more than 25 residential units, a single pump or ejector may be used, provided that the station is designed to permit the installation of a future duplicate pump or ejector with no structural changes.

(g) *Pumps.* 1. All pumps, except grinder and effluent pumps, shall be capable of passing spheres of at least 7.6 centimeters (3 inches) in diameter, and pump suction and discharge piping shall be at least 10 centimeters (4 inches) in diameter. The department may allow the use of pumps with a lesser solids handling ability provided the pump is protected by a comminutor, a mechanically cleaned bar screen, or other suitable equipment.

2. All pumps shall be nonclogging. Where a potential for clogging exists, protection in the form of manual bar screens, mechanically cleaned bar screens, comminutors or other suitable means shall be provided. Bar screens and comminutors shall be installed in accordance with s. NR 110.16.

3. Each pump shall be located so that under normal operating conditions it will operate under a positive suction head. Self-priming or vacuum primed pumps with adequate suction lift capability are exempted from this requirement.

(h) *Piping.* 1. Each pump, except submersible, screw, grinder and effluent pumps, shall be equipped with individual suction piping. Suction piping shall be as straight as possible.

2. When suction elbows are used, the bell shall be placed above the floor of the wet well at a distance which is not greater than 1/2 nor less than 1/3 the diameter of the bell.

3. A suitable shutoff valve shall be placed on each discharge line, except for screw pumps. A shutoff valve shall be placed on each suction line of the dry well pump. A check valve shall be placed on each discharge line between the shutoff valve and the pump, except for screw pumps. Check valves shall be placed in horizontal sections of the discharge pipe, except for ball check valves which may be placed in the vertical run.

4. Valves may not be located in wet wells.

(i) *Controls.* 1. Control systems shall be of the air bubbler type, the encapsulated float or displacement type, the ultrasonic type, the pressure transducer type or capacitance probe type.

2. The control system shall be located away from the turbulence of incoming flow and pump suction.

3. Provisions shall be made to automatically alternate the pumps in use where multiple equivalent capacity pumps are installed.

4. All lift stations shall be equipped with an alarm system. The alarm system shall include audible and visual signals. The alarm system shall be activated in cases of power failure, pump failure, and at a predesignated high water level. It is also recommended that alarm systems be activated in the event of unauthorized entry or other lift station malfunction. The department may require that alarm systems be telemetered to responsible authorities for large or main lift stations.

(j) *Force mains.* 1. At the design pumping rate, a cleansing velocity of at least 61 centimeters per second (2 feet per second) shall be maintained.

2. A combination automatic air relief and vacuum valve or an automatic or manual air relief valve shall be placed at each high point in the force main to prevent air locking.

3. When a force main enters the gravity sewer manhole, it shall discharge at a point not more than 60 centimeters (2 feet) above the spring line of the receiving sewer.

4. Friction losses through force mains shall be based on the Hazen and Williams formula or other acceptable method. The roughness coefficient "C" value used for design shall be noted on the design report.

**Note:** When the Hazen and Williams formula is used, the department recommends a "C" value between 100 and 125 be used for all pipe except plastic pipe. A "C" value between 120 and 140 is recommended for plastic pipe. When initially installed, force mains may have a significantly higher "C" value. The lower "C" value (higher coefficient of friction) should be considered when calculating the head at design conditions. The higher "C" value should be considered when calculating the minimum head in the pump operating range power requirements.

**(4) SUCTION LIFT PUMPS OR WET WELL MOUNTED LIFT STATIONS.** (a) *Priming.* Suction lift pumps shall be of the self-priming or vacuum primed type and shall comply with applicable requirements of sub. (3), except as modified in this subsection.

(b) *Lift.* The total dynamic suction lift may not exceed 6.0 meters (20 feet), unless the department approves pumps which exceed this specified limit based on certified pump performance curves and detailed calculations which justify the higher heads.

(c) *Compartment separation.* The pump equipment compartment shall be above grade, partially recessed in the ground or offset and shall be isolated from the wet well in a manner which will prevent the humid and corrosive sewer atmosphere from entering the equipment compartment. Wet well access may not be through the equipment compartment and shall be at least 61 centimeters (24 inches) in diameter or equivalent area.

(d) *Electrical.* Float type displacement switches and electronic transducers which are used in wet wells shall comply with the applicable requirements of sub. (5).



**(5) SUBMERSIBLE LIFT STATIONS.** (a) *Construction.* Submersible pumps and motors shall be designed specifically for raw sewage use, and for total submergence during operation, and shall comply with the applicable requirements of sub. (3), except as modified in this subsection.

(b) *Pump removal.* Submersible pumps shall be readily removable and replaceable without dewatering the wet well or disconnecting any piping in the wet well. Removal of one submersible pump from the lift station may not interrupt the operation of other pumps in the station.

(c) *Electrical equipment.* 1. Electrical supply, control, and alarm circuits shall be designed to provide strain relief appurtenances. All junction boxes containing terminals and connectors shall be protected from corrosion by being located outside the wet well or through the use of a watertight seal. Junction boxes for motor power cable connections and for intrinsically safe control circuits shall meet the requirements of weatherproof equipment.

2. The motor control center shall be located outside the wet well and be protected by conduit seals or other appropriate measures meeting the requirements of the NEC to prevent the atmosphere of the wet well from gaining access to the control center. The explosion proof seals shall be located so the pump motor and the level control float switches or transducers can be removed and electrically disconnected at the respective junction box without destroying the seal.

3. Pump motor power cables shall be designed for flexibility and serviceability. Ground fault interruption protection shall be provided in accordance with the NEC requirements. Power cord terminal fittings shall be corrosion-resistant and constructed in a manner to prevent the entry of moisture into the cable, shall be provided with strain relief appurtenances, and shall be designed to facilitate field connecting.

(d) *Explosion prevention.* In order to minimize the potential for ignition of explosive gases in submersible lift stations, one of the following requirements shall be met:

1. Low water level controls may be set such that the pump motor will remain totally submerged at all times. The lift station shall be equipped with a low water alarm in addition to meeting the requirements of sub. (3) (i) 4., and which is distinguishable from the high water alarm.

2. The pump motor may be rated for compliance with the NEC explosion proof requirements of Class 1, Group D, Division 1 locations.

**(6) WET WELL AND DRY WELL LIFT STATIONS.** (a) *Construction.* The wet well and dry well lift stations shall be designed specifically for raw sewage use and shall comply with applicable requirements of sub. (3), except as otherwise provided in this subsection.

(b) *Electrical.* When float type displacement switches or electronic transducers are used to control the liquid level in the wet well, they shall comply with the applicable requirements of sub. (5).

**(7) SCREW PUMP STATIONS.** (a) *Applicability.* Screw pumps may be approved by the department on a case-by-case basis upon submission of appropriate manufacturer's data and the detailed design calculations.

(b) *Construction.* Screw pumps and motors shall be designed specifically for raw sewage use, and shall comply with applicable requirements of sub. (3).

**(8) PNEUMATIC EJECTOR LIFT STATIONS.** (a) *Applicability.* Pneumatic ejectors may be approved by the department on a case-

by-case basis upon submission of appropriate manufacturer's data and the detailed design calculations.

(b) *Construction.* Pneumatic ejectors shall be designed specifically for raw sewage use, and shall comply with the applicable requirements of sub. (3).

(c) *Compressors.* Multiple compressors shall be provided. The compressors shall be sized to handle the maximum hour design flow with the largest compressor out of service.

**(9) GRINDER PUMP LIFT STATIONS — DUPLEX.** The department may approve duplex grinder pump lift stations. All duplex grinder pump lift stations shall be designed specifically for raw sewage use. Submersible duplex grinder pump lift stations shall meet the submersible pump requirements of sub. (5), and the non-submersible type duplex grinder pump lift stations shall comply with applicable requirements of sub. (6), except that each grinder pump shall have a minimum 1¼-inch pump opening and discharge piping. The total motor horsepower requirement for each pump shall be 5 horsepower or less.

**(10) GRINDER PUMP LIFT STATIONS — SIMPLEX.** (a) *Applicability.* The department may approve simplex or individual grinder pump lift stations if no more than 3 residential units are ultimately served by 1 pump.

(b) *Construction.* All simplex grinder pump lift stations shall be designed specifically for raw sewage use. The submersible grinder pump shall be designed for total submergence during operation. Both the submersible and the non-submersible type simplex grinder pump stations shall comply with all of the following requirements:

1. 'Location.' The location of the grinder pump stations and pressure service laterals shall be shown on the plans.

2. 'Flood proofing.' Grinder pump stations located in the floodplain shall be flood proofed by constructing 2 feet above the 100 year flood elevation or by providing watertight covers.

3. 'Pump removal.' Grinder pumps shall be readily removable without entry into the wet well or dewatering the wet well.

4. 'Valving.' a. All valving shall be accessible without entry into the wet well or dewatering the wet well.

b. If a grinder pump station discharges to a common pressurized sewer, a redundant check valve shall be provided. All valves including redundant check valves shall be installed at the grinder pump station. If a grinder pump station discharges to a gravity sewer, the redundant check valve is not required.

5. 'Ventilation.' All grinder pump stations shall be vented to the atmosphere either from the wet well or from the service lateral.

6. 'Accessibility.' No permanent ladders or steps shall be provided in the wet wells.

7. 'Pump size.' All grinder pumps shall have a minimum 1¼-inch pump inlet opening and discharge piping.

8. 'Velocity.' A minimum velocity of 61 centimeters (2 feet) per second shall be maintained in the discharge piping during pump operation.

9. 'Separation.' A minimum separation distance of 25 feet shall be maintained between the grinder pump lift station and a private well or any other well subject to ch. NR 812. A minimum separation of 200 feet shall be maintained between the grinder pump station and a community water system well in accordance with ch. NR 811.

10. 'Alarm.' Audible and visual high water alarm system shall be provided.

11. 'Electrical.' a. The motor control unit shall be located outside the wet well.

b. All junction boxes shall be located outside the wet well, and shall meet the requirements of weather proof equipment. Electrical supply, control and alarm circuits shall be designed to allow disconnection at the junction box, without destroying the conduit-seal.

c. Level controls using float type displacement switches shall be suspended in the wet well to facilitate maintenance.

12. 'Explosion prevention.' a. In order to reduce the potential for ignition of explosive gases in submersible grinder pump lift stations, one of the following conditions shall be met: the low water level alarm shall be set such that the pump motor will remain totally submerged at all times; the pump motor shall be rated explosion proof in accordance with NEC requirements of Class I, Group D, Division 1 locations; or the motor shall be listed as safe and appropriate for residential use by the Underwriters Laboratories, Inc.

b. In order to reduce the potential for ignition of explosive gases in non-submersible grinder pump lift stations with non-explosion proof pump motors, the pump motor shall be completely isolated from the wet well atmosphere in a separate gas tight housing.

(11) EFFLUENT PUMPS. The department may approve the duplex and simplex effluent pump lift stations for septic tank effluent in accordance with the applicable requirements of subs. (9) and (10), respectively.

(12) EMERGENCY OPERATION. (a) *General*. Provisions for emergency operation of lift stations shall be provided to prevent the discharge of raw or partially treated sewage to a surface water or to a ground surface and to prevent sewage backups into basements.

(b) *Lift station requirements*. One of the following provisions shall be made to insure continued operation of each lift station:

1. An on-site generator, with automatic switching and starting equipment may be installed. The generator shall have sufficient capacity to meet the total electrical demands of the pumps, controls, and auxiliary equipment.

2. An on-site gasoline or diesel engine driven pump, with automatic switching and starting equipment may be installed. The pump shall have a capacity equal to or greater than the lift station peak design pumping rate.

3. A portable generator may be available for use at the lift station. The generator shall have sufficient capacity to meet the total electrical demands of the pumps, controls, and auxiliary equipment. Electrical connections shall be accessible without maintenance personnel having to enter the lift station.

4. A portable pump with a pumping capacity equal to or greater than the lift station peak design pumping rate may be available for use at the lift station. Quick disconnect fittings shall be used to connect the portable pump to the suction and the discharge line, and shall be accessible without maintenance personnel having to enter the lift station.

5. The lift station electrical system may be connected to 2 independent electrical transmission routes which receive power from the same electrical grid network which supplies power to the lift station service area.

6. The lift station may be equipped with a holding facility which has a capacity to hold the average design flow for a minimum period of 24 hours.

(c) *Grinder pump and effluent pump lift stations*. 1. Emergency operation of duplex grinder pump and effluent pump lift stations which serve more than 3 residential units shall be provided by one of the methods described in par. (b).

2. Emergency operation provisions for duplex grinder pump and effluent pump lift stations may be waived for those stations

which serve homes with private water supply systems provided it is demonstrated to the department that the lift station wet well has the capacity to hold the residual water volume of the private water system.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83; correction in (2) (a) 3, made under s. 13.93 (2m) (b) 7., Stats., Register, September, 1995, No. 477; correction in (2) (a) 3, was made under s. 13.93 (2m) (b) 7., Stats., Register, May, 2001, No. 545; CR 09-123: am. (1) (b) and (2) (b) 3., r. and recr. (2) (a) 3. and (3) to (5), renum. (6) and (7) to be (9) and (12) and am., cr. (6) to (8), (10) and (11) Register July 2010 No. 655, eff. 8-1-10.

**NR 110.15 General requirements for sewage treatment facilities.** (1) *DESIGN REPORT*. A design report shall be submitted with plans and specifications for all sewage treatment facilities. This report shall summarize the design hydraulic loading, design biochemical oxygen demand (BOD), suspended solids and other appropriate pollutant loading, the sizing of treatment units, pump capacities, design calculations for major treatment units, and explain any deviations from the preliminary facilities planning design information which is required by s. NR 110.09 (1) (b).

(2) *EFFLUENT QUALITY*. (a) *Design*. Sewage treatment facilities shall be designed to achieve compliance with the monthly and weekly average effluent limitations for biochemical oxygen demand (BOD) and total suspended solids contained in ch. NR 210, or other WPDES permit requirements, as appropriate, or with any more stringent water quality related effluent limitations required to achieve appropriate water quality standards derived from chs. NR 102 to 106, or from any federally promulgated water quality standard for any waters of the state.

(b) *Treatment during construction*. During construction of new facilities, treatment shall be maintained at a level not less than that which existed prior to the start of construction.

(3) *PLANT LOCATION*. (a) *Flood protection*. 1. All sewage treatment facilities shall be located such that they are not subject to flooding.

2. Any sewage treatment facility located in a floodplain, or suspected to be in a floodplain, will not be approved until the flood analysis requirements of s. NR 110.09 (2) (n) are met.

3. A sewage treatment facility may be located in the flood fringe providing the requirements of par. (c) and ss. NR 116.14, 116.15, 116.16 and 116.17 are met.

4. All sewage treatment facilities which are located in a floodplain shall be floodproofed to an elevation of at least 2 feet above the regional flood elevation.

5. Location of a land disposal system in a floodplain will be evaluated on a case-by-case basis.

(b) *Floodway construction*. 1. No new sewage treatment facility shall be located in a floodway as defined in ss. NR 116.03 and 116.11. A change in the zoning classification of a treatment facility site from floodway to flood fringe in accordance with the procedures specified in s. NR 116.21 (6), will be acceptable only if the rezoning is complemented by the construction of a dry land access as defined in par. (c). When the facility site is rezoned from floodway to flood fringe, the dry land access requirement will not be waived.

2. Any existing sewage treatment facility which is located in a floodway shall meet the requirements of subd. 1. if the expansion or upgrading of the facility is greater than 50% of the value of the facility. Value is defined in par. (c).

3. Any existing sewage treatment facility which is located in a floodway will not have to comply with the requirements of subd. 1. if the expansion or upgrading of the facility is less than 50% of the value of the facility. In this instance, the dry land access requirements of par. (c) will not apply.

(c) *Accessibility.* 1. Sewage treatment facilities shall be accessible at all times. Sewage treatment facilities located in a flood fringe shall be accessible by dry land access. Dry land access is defined as a service road which has a minimum elevation of at least one foot above the regional flood elevation.

2. The dry land access requirement may be waived by the department if one of the following criteria is met:

a. The physical characteristics of the treatment site and the surrounding area pose practical difficulties for construction of dry land access, and the isolation of the sewage treatment facility during the regional flood is less than 24 hours for mechanical treatment facilities, or 5 days for lagoon systems. The duration of the regional flood shall be calculated using the methods described in s. NR 116.07; or

b. The physical characteristics of the treatment site and the surrounding area pose practical difficulties for construction of dry land access, and the treatment facility access is inundated by less than one foot of water during the regional flood. In these instances, the access roads shall be stabilized and delineated; or

c. The construction costs of the expansion or upgrading of an existing treatment facility are less than 50% of the value of the existing facility. The value of the existing facility shall be calculated by subtracting the 20-year total present worth of expanding or upgrading the existing facility from the 20-year total present worth of the most cost-effective treatment alternative located at another site which is not in a floodplain.

(d) *Isolation.* 1. In order to minimize any potential odor, noise, and nuisances caused by sewage treatment facilities, and to enhance plant security and reliability, sewage treatment facilities shall be isolated from commercial establishments and from buildings occupied or intended for residential use, and from land which is actively being developed for commercial or residential use. The following separation distances shall be maintained:

a. 150 meters (500 feet) for mechanical treatment facilities, effluent holding and polishing ponds;

b. 150 meters (500 feet) for seepage cells, ridge and furrow systems, and overland flow systems;

c. 230 meters (750 feet) for aerated lagoons;

d. 305 meters (1,000 feet) for off site sludge holding facilities and spray irrigation systems; and

e. 460 meters (1,500 feet) for stabilization lagoons.

2. The department may waive the requirements of subd. 1. if the requirements prevent implementation of the cost-effective treatment alternative at an existing sewage treatment facility site. When a waiver is requested, the owner shall demonstrate to the department that:

a. The owner has made reasonable attempts to obtain an agreement from any affected property owner which states that the property owner has been informed of the potential nuisances which may result from the operation of the sewage treatment facilities and that the property owner does not object to the construction and operation of the sewage treatment facilities; and

b. The treatment facility owner has enacted a zoning ordinance which prohibits future construction within the applicable separation distances, or has purchased sufficient land surrounding the sewage treatment facility to prevent future encroachment.

3. The department may waive the requirements of subd. 1. if the requirements prevent construction of the cost-effective treatment alternative at a new site. When a waiver is requested the owner shall demonstrate that:

a. The treatment facility has obtained from the affected property owners the agreements described in subd. 2. a. or that the proposed sewage treatment facility site is the only reasonably

available site or, the costs associated with using another site would place an unreasonable or excessive financial burden on the community; and

b. The treatment facility owner has enacted a zoning ordinance which prohibits future construction within the applicable separation distances, or has purchased sufficient land surrounding the proposed treatment facility site to prevent future encroachment.

(e) *Protection of water supply wells.* Wastewater lagoons or storage structures shall be located with a minimum separation distance of 1,000 feet from a community water system well, and 100 feet from a private water system well or any other well subject to ch. NR 812. Wastewater treatment plant effluent pipes shall be located with a minimum separation distance of 50 feet from a private water system well or any other well subject to ch. NR 812.

(4) *DESIGN OF SEWAGE TREATMENT FACILITIES.* (a) *Conformance with facilities plan.* The design capacity for municipally owned sewage treatment facilities shall be in accordance with s. NR 110.09 (2) (j). Privately owned domestic sewage treatment facilities shall provide design capacity for the estimated population 20 years from the time of start-up of the facility unless the cost-effective staging analysis in s. NR 110.09 (2) (j) 4. justifies a lesser design staging period.

(b) *Organic loading.* 1. The domestic design biochemical oxygen demand and suspended solids loading for upgrading or expanding existing sewage treatment facilities, or for the construction of new sewage treatment facilities to replace an existing facility shall be based on actual sewage and operating records from the existing facilities. The design shall include an appropriate growth increment.

2. When actual operating data is not available, the design loading shall be based on a contribution of 0.08 kilograms (0.17 pounds) of biochemical oxygen demand per capita per day and 0.09 kilograms (0.20 pounds) of suspended solids per capita per day. When garbage grinders are used in areas tributary to a sewage treatment facility, the design basis shall be increased to 0.10 kilograms (0.22 pounds) of biochemical oxygen demand per capita per day, and 0.22 kilograms (0.25 pounds) of suspended solids per capita per day.

3. Sewage treatment facilities which will receive industrial or commercial wastewater shall be designed to include these waste flows.

(c) *Hydraulic loading.* The design wastewater flow shall be estimated in accordance with s. NR 110.09 (2) (j). When flow or water use records do not exist, the maximum hour design flow shall be estimated by multiplying the average design flow by the appropriate peaking factor shown in Table 2.

Table 2

| Community Size (population) | Peaking Factor |
|-----------------------------|----------------|
| Under 1,000                 | 4 - 5          |
| 1,000 - 10,000              | 3.0 - 3.5      |
| 10,000 - 100,000            | 2.0 - 2.5      |
| Over 100,000                | 1.5 - 2.0      |

(d) *Sludge storage.* Adequate sludge storage shall be provided as indicated in ss. NR 110.09 (2) (h) 5 and 110.26 (10).

(5) *DESIGN FEATURES.* (a) *Design of conduits.* All piping and channels shall be designed to carry the peak design flow rate. The incoming sewer should be designed for unrestricted flow. Bottom corners of the channels must be filleted. Conduits shall be designed to avoid creation of pockets and corners where solids can accumulate. Suitable gates shall be placed in channels to seal off unused sections in which solids might accumulate. The use of



shear gates or stop planks may be used in place of gate valves or sluice gates.

(b) *Arrangement of units.* Component parts of the facility shall be arranged for greatest operating and maintenance convenience, flexibility, economy, continuity of effluent quality, and ease of installation of future units.

(c) *Flow measurement.* Equipment for flow measurement and recording shall be provided for the total waste flow. Equipment for measuring flow streams within the treatment facility should be provided to aid facility operation.

(d) *Emergency operation.* At least one of the following shall be provided to ensure continued operation of the sewage treatment facility in accordance with s. NR 210.08:

1. 'Emergency power generator.' An emergency power generator with sufficient generating capacity to meet the sewage treatment facility power demands to comply with s. NR 210.08.

2. 'Two independent electrical transmission sources.' An electrical system connected to two independent transmission routes that receive power from the same electrical grid network which supplies power to the sewage treatment facility service area.

3. 'Holding facilities.' Holding facilities that have a capacity to detain the maximum day design flow for a maximum period of 24 hours.

(e) *New processes, methods and equipment.* The department encourages the development of new process, methods, or equipment for the treatment of sewage. However, where new processes, methods, or equipment are proposed and where limited data is available which demonstrates the performance of the equipment, the department may require written certification that the use or design of the equipment is in accordance with the manufacturer's guidelines. Furthermore, the department may require the posting of a performance bond by the manufacturer.

(f) *Disinfection.* Disinfection shall be provided in accordance with WPDES permit requirements.

(g) *Controlled diversion structures and equipment.* Structures and equipment to enable controlled diversions shall be located and arranged to allow for proper maintenance of the sewage treatment facility. In all cases, it must be possible for each treatment unit to be independently removed from service.

**Note:** Section NR 205.07 (1) (u) 2. contains specific provisions associated with the use of controlled diversion structures and equipment and requires compliance with all permit effluent limitations during times of controlled diversion.

(h) *Sewage treatment facility overflow structures.* Sewage treatment facility overflow structures may be provided at an owner's discretion as a measure to protect sewage treatment facility integrity and treatment efficiency during severe operating conditions. Sewage treatment facility overflow structures may not be installed at the headworks of aerated or stabilization pond treatment systems. Sewage treatment facility overflow structures shall be designed in accordance with all the following requirements:

1. The overflow may be activated by either manual or automatic means. If automatically activated, a monitoring system shall be provided to detect the initiation time of the overflow and to provide an alarm signal to the sewage treatment facility operator or other responsible authority.

2. The structure shall be designed to discharge only those wastewater flows above the peak flow rate that the sewage treatment facility can safely process without threatening loss of life, causing severe property damage, or compromising treatment processes, including the washout of biological media in the biological treatment process.

3. Equipment shall be provided to measure the flow and sample the wastewater that is discharged from the structure.

**Note:** A department approval of a sewage treatment facility overflow structure does not eliminate or alleviate the requirement that prohibits sewage treatment facility overflows in s. NR 210.21.

(i) *Painting.* 1. The use of paints containing lead is prohibited. In order to facilitate the identification of piping, pipes shall be painted as follows:

- a. Sludge line—brown;
- b. Gas line—orange;
- c. Potable water line—blue;
- d. Chlorine line—yellow;
- e. Sewage line—gray;
- f. Compressed air line—green;
- g. Nonpotable water line—blue with 15 centimeter (6 inch) red bands spaced 76 centimeters (30 inches) apart.

2. In addition to the color code, each pipe shall be adequately labeled with a minimum of 2 labels in each room, crawl space or compartment.

3. Existing treatment facilities which do not comply with the provisions of this subsection shall bring the facility into compliance at the time of any major upgrading or expansion of the facility.

(j) *Valve identification.* All valves shall be identified in the plans and specifications and labeled during construction.

(k) *Operational considerations.* All necessary tools and accessories for the facility operator's use shall be provided. Storage space and a work area shall also be provided. All equipment shall be located as to provide sufficient clearance for proper and convenient maintenance. All tanks, wet wells, channels and pipe systems shall be equipped with drains, valves, or sumps to facilitate draining for maintenance and repair.

(L) *Laboratory space and equipment.* A treatment facility owner shall either include a laboratory for making the necessary analytical determinations and operating control tests, or contract with a neighboring facility or independent laboratory to have the analytical and operating control tests done.

(m) *Floor slope.* Floor surfaces shall be sloped adequately to a point of drainage.

(n) *Erosion control during construction.* Effective site erosion control shall be provided during construction. Project specifications shall detail erosion control methods. Manner of spoil material disposal shall also be detailed.

(o) *Construction materials.* Materials shall be selected that are compatible with the wastewater characteristics. Dissimilar metals should be avoided to minimize galvanic action.

(p) *Sanitary facilities.* Toilet, shower, lavatory, and locker facilities should be provided in sufficient numbers and convenient locations to serve the expected facility personnel. Toilet, shower, and lavatory facilities shall be provided in the following instances:

1. Any sewage treatment facility equipped with laboratory facilities;
2. Any sewage treatment facility equipped with a potable water supply; or
3. Any sewage treatment facility which has one or more full time operating personnel.

(q) *Safety.* 1. Sewage treatment facilities shall be enclosed with a fence to discourage entry of animals or unauthorized persons.

2. Hand rails shall be installed around all treatment tanks and in other areas of the facility where the potential of falling exists.

3. The department recommends the following safety measure be considered in the design of wastewater treatment facilities:

- a. Provision of first aid equipment;
- b. Posting of "No Smoking" signs in hazardous areas;
- c. Provision of protective clothing and equipment such as gas masks, goggles, gloves, hard hats, and safety harness;
- d. Portable blower and hose;
- e. Portable lighting equipment; and
- f. Nonpotable water supply bibs which are labeled.

4. The safety and health rules set forth in ch. SPS 332, and appropriate federal and local safety codes shall be adhered to in the operation of wastewater treatment plants.

5. Specific safety requirements for hazardous chemical handling are found in s. NR 110.22 (4).

6. Specific safety requirements for chlorination facilities are found in s. NR 110.23 (2) (g).

**(6) WATER SUPPLY.** (a) *Potable supply.* Any sewage treatment facility which has a laboratory shall be provided with a potable water supply.

(b) *Plumbing.* Sewage treatment facility plumbing systems shall be designed in accordance with ch. SPS 382.

(c) *Connection to public water systems.* Connection of a sewage treatment facility plumbing system to a public water system shall comply with the requirements of s. NR 811.25 and ch. SPS 382.

(d) *On-site wells.* Construction of wells for supplying water to a sewage treatment facility shall comply with the requirements of the approval obtained under s. NR 812.09 (4) (b).

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. (1) (c), Register, March, 1978, No. 267, eff. 4-1-78; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83, cr. (4) (d), Register, November, 1990, No. 419, eff. 12-1-90; corrections in (5) and (6) made under s. 13.93 (2m) (b) 7., Stats., Register, September, 1995, No. 477; corrections in (5) (q) 4., (6) (b), and (c) were made under s. 13.93 (2m) (b) 7., Stats., Register, May, 2001, No. 545; CR 09-123: am. (2) (a) and (4) (c), cr. (3) (e) Register July 2010 No. 655, eff. 8-1-10; corrections in (5) (q) 4., (6) (b) and (c) made under s. 13.92 (4) (b) 7., Stats., Register January 2012 No. 673; CR 12-027: r. (2) (c) to (e), r. and recr. (5) (d), am. (5) (g), r. and recr. (5) (h) Register July 2013 No. 691, eff. 8-1-13; CR 18-095: am. (6) (d) Register June 2020 No. 774, eff. 7-1-20; correction in (6) (b) made under s. 35.17, Stats., Register June 2020 No. 774.

**NR 110.16 Screening devices. (1) GENERAL DESIGN CONSIDERATIONS.** (a) *Applicability.* All wastewater treatment plants shall be provided with protection for pumps and other equipment by installing coarse screens, bar racks, mechanically cleaned bar racks or comminutors.

(b) *Location.* 1. Screening devices installed in a building where other equipment or offices are located shall be separated from the rest of the building and provided with separate outside entrances.

2. Screening devices shall be provided with convenient access.

3. Screening devices may not be located such that changes in backwater elevations will interfere with the accuracy of upstream flow measuring equipment.

(c) *Ventilation.* Screening areas shall be ventilated.

(d) *Channels.* 1. The channel preceding and following the screen shall be shaped to minimize settling of solids. Fillets shall be installed as necessary.

2. The screen channel invert must be at least 8 centimeters (3 inches) below the invert of the incoming sewer.

3. Where multiple screening units are installed the channels shall be equipped with the necessary gates to direct flow from any one screening unit. Methods for dewatering each channel shall be provided.

4. Entrance channels shall be designed to distribute flow uniformly to the screening units.

(e) *Handling screenings.* Adequate facilities must be provided for removal, handling and storage of screenings in a sanitary manner. Hand-cleaned screening facilities must include an accessible platform from which the operator may rake screenings. Suitable drainage facilities must be provided for both the platform and the storage areas.

**(2) DESIGN REQUIREMENTS FOR SCREENING DEVICES.** (a) *Bar screens.* 1. Clear spacing between bars may not be less than 2.5 centimeters (1 inch), nor more than 5 centimeters (2 inches).

2. Bar screens must be placed on a slope of 30 to 45 degrees with the horizontal with the exception of those installed for emergency use.

3. Approach velocities may be no less than 38 centimeters per second (1.25 feet per second) at design average flow conditions to prevent settling, and no greater than 91 centimeters per second (3 feet per second) at maximum design daily flow to prevent forcing material through the openings.

(b) *Mechanical screens.* 1. Maximum clean spacing between bars may not exceed 5 centimeters (2 inches).

2. Approach velocities may be no less than 38 centimeters per second (1.25 feet per second) at average design flow conditions to prevent settling, and no greater than 91 centimeters per second (3 feet per second) at maximum daily flow to prevent forcing material through the openings.

3. All mechanical units which are operated by timing devices shall be provided with auxiliary controls which will set the cleaning mechanism in operation at a preset high water elevation.

4. Automatic controls shall be supplemented by a manual override. Manual overrides shall be located in view of the equipment.

5. Electrical fixtures and controls in screening areas where hazardous gases may accumulate shall meet the requirements of the national electrical code for class 1, group D, division 1 locations.

(c) *Comminutors.* 1. Comminutors shall be designed to commute the maximum design flow rate.

2. Comminutor channels shall be designed to contain the upstream water depth associated with the head loss which occurs at maximum design flow without surcharging the incoming sewer or other treatment processes. The expected head loss shall take into account the effects of clogging during operation.

3. Comminutor channels shall be equipped with drains.

4. A screened bypass channel shall be provided so that the comminutor may be removed from service for maintenance.

5. Bypass channels will not be required where 2 comminutors are installed. Each comminutor shall be capable of comminuting the maximum design flow.

6. Control switches or a disconnecting device for the comminutor shall be located in view of the comminutor.

7. Electrical fixtures and controls in comminutors areas where hazardous gases may accumulate shall meet the requirements of the national electrical code for class 1, group D, division 1 locations.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83.

**NR 110.17 Grit removal facilities. (1) APPLICABILITY.** Grit removal facilities are recommended for all sewage plants and are required for plants receiving sewage from combined sewers or from sewer systems receiving substantial amounts of grit.

(2) DESIGN CONSIDERATIONS. (a) *Location*. Where practical, grit chambers should precede influent pumps. When installed, grit chambers shall precede all major treatment units.

(b) *Protection*. All grit chambers shall be preceded by a bar rack, coarse screen or comminutor.

(c) *Housed facilities*. 1. Enclosed grit removal areas shall be ventilated. Fresh air shall be introduced continuously at a rate of 12 air changes per hour, or intermittently at a rate of 30 air changes per hour.

2. All electrical work in enclosed grit removal areas where hazardous gases may accumulate shall meet the requirements of the national electrical code for class 1, group D, division 1 locations.

(d) *Number of units*. Grit removal facilities shall have at least 2 hand-cleaned units, or a mechanically cleaned unit with a bypass.

(e) *Grit handling*. 1. All facilities not provided with positive velocity control shall include means for grit washing to further separate organic and inorganic materials.

2. Grit removal facilities located in deep pits shall be provided with mechanical equipment for hoisting or transporting grit to ground level. Impervious, nonslip, working surfaces with adequate drainage shall be provided for grit handling areas. Storage areas for wasted grit shall be provided.

(f) *Basis of design*. Design of grit chambers shall be based on the size and specific gravity of the grit particle to be removed. If this information is not obtained from actual field measurements, then the design shall assume removal of all particles retained on a 65 mesh (0.21 mm) sieve and having a minimum specific gravity of 2.65. The design requirements of sub. (3) are based on these assumptions.

(3) DESIGN REQUIREMENTS. (a) *General requirements*. 1. Inlet turbulence into grit chambers shall be minimized.

2. Drains or other means for dewatering each grit unit must be provided.

3. An adequate supply of water under pressure shall be provided for cleaning grit equipment.

(b) *Velocity controlled grit chambers*. Positive hydraulic control shall be provided to maintain a channel velocity of 30 centimeters per second (one foot per second) through the expected flow range. Positive hydraulic control shall be provided by one of the following:

1. A flow channel with a parabolic cross-section;
2. A proportionate weir; or
3. A Parshall flume.

(c) *Aerated grit chambers*. 1. Air rates should be in the range of 4.6 to 12.4 liters per second per meter (3 to 8 cubic feet per minute per foot) of tank length.

2. The detention time at the maximum design flow rate should not exceed 3 minutes.

3. Inlets and outlets shall be designed to prevent short circuiting.

4. The design of the grit chamber shall be such to avoid producing dead spaces.

(d) *Short-term sedimentation tanks*. 1. Inlets shall be designed to distribute flow evenly across the tank.

2. Tanks shall be deep enough to prevent turbulent flow.

3. An additional depth of 15 to 25 centimeters (6 to 10 inches) shall be provided for raking mechanisms.

4. Surface area of the sedimentation tank shall be designed not to exceed a surface settling rate of 1,900 cubic meters per day per square meter (46,000 gallons per day per square foot).

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83.

**NR 110.18 Settling tanks.** (1) DESIGN CONSIDERATIONS. (a) *Multiple settling tanks*. 1. Multiple settling tanks shall be provided at all sewage treatment plants with an average design flow which exceeds 1,890 cubic meters per day (0.5 million gallons per day).

2. Control appurtenances such as valves, gates, splitter boxes, and flow measuring devices, shall be provided to divide inflow equally to each settling tank.

(b) *Servicing*. 1. All settling tanks shall be provided with easy access for maintenance.

2. Each settling tank shall be capable of being independently dewatered and isolated for servicing. Provisions shall be made to protect empty settling tanks from the hydrostatic uplift due to high groundwater.

(c) *Safety*. Operator safety shall be considered in the design of settling tanks. Safety features such as machinery covers, life lines, stairways, walkways, handrails and slip-resistant surfaces shall be provided if appropriate.

(d) *Electrical controls*. Electrical fixtures and controls in enclosed settling tanks shall meet the requirements of the national electrical code for class 1, group D, division 1 locations. The fixtures and controls shall be located to provide convenient and safe access for operation and maintenance.

(e) *Imhoff tanks*. Imhoff tanks will not be approved by the department.

(2) DESIGN REQUIREMENTS. (a) *Inlets*. 1. Settling tank inlets shall be designed to dissipate the inlet velocity, to distribute the flow equally and to prevent short-circuiting. Either channel shall be designed to maintain a velocity of at least 30 centimeters per second (one foot per second) at one-half of average design flow or some other means of preventing solid deposition shall be used. Corner pockets and dead ends shall be eliminated and corner fillets or channeling used where necessary. Elimination or removal of floating materials in inlet structures having submerged ports shall be required.

2. Inlet velocities in rectangular settling tanks may not exceed 15 centimeters per second (0.5 foot per second).

3. Inlet velocities in center feed circular settling tanks may not exceed 91 centimeters per second (3 feet per second).

(b) *Tank features*. 1. The side water depth of mechanically cleaned settling tanks shall be as shallow as practical but not less than 2.1 meters (7 feet). Final clarifiers for activated sludge may not be less than 3 meters (10 feet) in depth. Final clarifier for fixed film treatment systems may not be less than 2.1 meters (7 feet).

2. The tops of troughs, beams, and similar construction features which are submerged shall have a minimum slope of 1.4 vertical to one horizontal. A slope of one to one shall be provided on the underside of such features to prevent the accumulation of scum and solids.

3. Effluent weirs shall be adjustable.

(c) *Sludge and scum removal*. 1. Scum baffles shall be provided ahead of outlet weirs on all primary and final settling tanks.

2. Sludge hoppers shall have a minimum side wall slope of 1.7 vertical to one horizontal. Hopper wall surfaces should be made smooth with rounded corners to aid in sludge removal. The department will not approve increasing the depth of sludge hoppers for the purpose of sludge thickening in settling tanks.



3. Each sludge hopper shall have an individually valved sludge withdrawal line at least 15 centimeters (6 inches) in diameter. Head available for withdrawal of sludge shall be at least 76 centimeters (30 inches), or greater as necessary, to maintain a velocity of 91 centimeters per second (3 feet per second) in the withdrawal pipe.

4. A sludge well or other appropriate equipment shall be provided for viewing and sampling sludge.

5. Provisions for cleaning sludge piping shall be made.

6. The department may approve the use of glass lined pipe for sludge. Glass lined pipe may not be less than 10 centimeters (4 inches) in diameter.

7. Suitable mechanical sludge and scum collection equipment shall be provided in all settling tanks. Provisions for separate scum washing shall be made for treatment facilities which do not have primary settling facilities.

(d) *Design parameters.* 1. Operating design parameters for settling facilities may not exceed the values given in Table 3. The surface settling rate for primary settling tanks shall be calculated with all flows received at the settling tank. The surface settling rate for final settling tanks shall be based on influent flow. The maximum hour solids loading shall be computed based on the maximum day design flow plus the maximum design return sludge rate requirement and the design mixed liquor suspended solids (MLSS) under aeration.

**Table 3**  
SURFACE SETTLING RATE

| Location                      | Flow Basis     | Solids Loading  |   | Weir Overflow Rate<br>-m/m.d-<br>(gal/ft. day) |
|-------------------------------|----------------|---|---|--|
|                               |                | -m <sup>3</sup> /m <sup>2</sup> .d-<br>(gal/ft <sup>2</sup> .d) | -kg/m <sup>2</sup> .h-<br>(lb/ft <sup>2</sup> .h) |  |
| Primary                       | Average design | 40<br>(1000)  | -   | 125<br>(10,000)                                |
|                               | Maximum Hourly | 60<br>(1500)  | -   | -  |
| Intermediate                  | Maximum Hourly | 60<br>(1500)  | -   | -  |
| Final (following)             |                |   |   |  |
| -Trickling filter/RBC         | Average design | -   | 5.0<br>(1.0)                                      | 125<br>(10,000)                                |
|                               | Maximum Hourly | 49<br>(1200)  | 8.0<br>(1.6)                                      | -  |
| -Activated Sludge             | Average design | -   | 7.0<br>(1.4)                                      | 125<br>(10,000)                                |
|                               | Maximum Hourly | 49<br>(1200)  | 10.0<br>(2.0)                                     | -  |
| -Separate Stage Nitrification | Average design | -   | 7.0<br>(1.4)                                      | 125<br>(10,000)                                |
|                               | Maximum Hourly | 33<br>(800)   | 10.0<br>(2.0)                                     | -  |
| -Extended Aeration            | Average design | -   | 6.0<br>(1.2)                                      | 125<br>(10,000)                                |
|                               | Maximum Hourly | 40<br>(1000)  | 10.0<br>(2.0)                                     | -  |

2. For treatment plants with an average design flow greater than 3,785 cubic meters per day (one million gallons per day), the department may approve an overflow rate of 188 cubic meters per meter per day (15,000 gallons per foot per day).

3. The design parameters shown in Table 3 may be waived by the department if the settling tank design is based on settling tests of wastes currently received at the existing treatment facility, or if the effluent from the sewage treatment facility is to be disposed on land.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83; CR 09-123: am. (2) (d) Register July 2010 No. 655, eff. 8-1-10.

**NR 110.19 Trickling filters. (1) APPLICABILITY.** (a) *Surface water discharge.* New trickling filters shall be used in

conjunction with other treatment units which, in combination, will produce an acceptable level of treatment as defined in s. NR 110.15 (2) (a). Existing trickling filters may be used as a treatment unit in plant expansion if the effluent quality requirements of s. NR 110.15 (2) (a) are met.

(b) *Land disposal.* Trickling filter treatment systems are an acceptable means of treatment prior to land disposal of effluent.

(2) **DESIGN REPORT.** A design report must be submitted in accordance with s. NR 110.15 (1). The report shall show the empirical equations and the assumptions used for designing the trickling filter and the additional treatment units.

(3) **DESIGN REQUIREMENTS.** (a) *Recirculation.* Recirculation shall be provided for intermediate and high-rate filters to increase treatment efficiency and to provide wetting of the biological growth. The recirculation rate shall be variable. The recirculation rate to average influent flow ratio should not exceed 4:1.

(b) *Dosing cycle.* The interval between dosing cycles may not exceed one hour.

(c) *Flooding.* Filter structures should be designed to allow flooding of the filter.

(d) *Primary treatment.* Trickling filters shall be preceded by primary treatment facilities.

(4) **DESIGN LOADING.** Hydraulic and organic loadings to trickling filters may not exceed the values given in Table 4. Higher loadings may be approved if justified by pilot studies or if manufactured media is used. Higher loadings may also be used if the trickling filter is intended to act only as a roughing or polishing treatment unit.

**Table 4**

| Filter Type       | Recirculation Ratio | Hydraulic Loading<br>-m <sup>3</sup> /m <sup>2</sup> .d-<br>(mgal/acre-day) | Organic Loading<br>-kg/m <sup>3</sup> .d-<br>(lbs/10 ft <sup>3</sup> .d) |
|-------------------|---------------------|---|--|
| Low-rate          | -                   | 4<br>(4)  | 0.3<br>(20)  |
| Intermediate-rate | less than 2:1       | 10<br>(11)  | 0.5<br>(30)  |
| High-rate         | 2:1-to-4:1          | 40<br>(43)  | 1.0<br>(60)  |

(5) **HYDRAULIC FEATURES.** (a) *Dosing equipment.* 1. Sewage shall be distributed over the filter by rotary distributors or other suitable devices which will permit uniform distribution to the filter surface area.

2. Sewage shall be applied to the filters by siphons, pumps or by gravity discharge from preceding treatment units when suitable flow characteristics have been developed. The dosing rate shall be large enough to insure rotation of the distributor arms.

3. A minimum clearance of 15 centimeters (6 inches) between media and distributor arms shall be provided.

(b) *Underdrainage system.* 1. An underdrainage system which covers the entire floor of the filter shall be provided. Inlet openings into the underdrains shall have an unsubmerged gross combined area equal to at least 15% of the surface area of the filter.

2. The underdrains shall have a minimum slope of 1%. Effluent channels shall be designed to produce a minimum velocity of 60 centimeters per second (2 feet per second) at the average design flow of application to the filter including recirculated flows.

(6) **MEDIA.** (a) *Type.* The media shall be crushed rock, slag or material specially manufactured for use as media in trickling filters.

(b) *Quality.* 1. Rock and slag media shall be durable, resistant to spalling or flaking, and be relatively insoluble in sewage. Slag media may not contain iron.

2. Manufactured media shall be resistant to ultraviolet degradation, disintegration, erosion, aging, all common acids and alkalis, organic compounds, and fungus or biological attack. Either the media shall be structurally capable of supporting a person's weight or a suitable access walkway shall be provided to allow for distributor maintenance.

(c) *Size and grading of rock, slag and similar media.* 1. Rock, slag and similar media may not contain more than 5% by weight of pieces whose longest dimension is 3 times the least dimension. They shall be free from thin elongated and flat pieces, dust, clay, sand, or fine material and shall conform to the following size and gradings when mechanically graded over a vibrating screen with square openings:

- a. Passing 11.4 centimeter (4 1/2-inch) screen — 100% by weight;
- b. Retained on 7.6 centimeter (3-inch) screen — 95-100% by weight;
- c. Passing 5.1 centimeter (2-inch) screen — 0.2% by weight;
- d. Passing 2.5 centimeter (1-inch) screen — 0.1% by weight;

2. The department may approve other rock media gradations provided the gradations are consistent with accepted published engineering practices.

(7) **STRUCTURAL FEATURES.** (a) *Mercury seals.* Mercury seals may not be used on trickling filter distributors. Mercury seals shall be removed from existing filters during renovation. Mercury seals removed from existing filters during renovation or destruction of the filter shall be disposed in a location and manner approved by the department.

(b) *Depth of media.* Rock or slag filter media, or loose synthetic media, shall have a minimum depth of 1.5 meters (5 feet) above the underdrains. Synthetic corrugated filter media should have a minimum depth of 3 meters (10 feet) to provide adequate contact time with the wastewater. Rock or slag filter media depths may not exceed 3 meters (10 feet) and synthetic filter media depths may not exceed 9.1 meters (30 feet) except where special construction is justified through pilot studies.

(c) *Covers.* Covers shall be provided on all filters to prevent icing and freezing and to increase the treatment efficiency of the filter during winter conditions.

(d) *Ventilation.* 1. The underdrainage system, effluent channels, and effluent pipe shall be designed to permit free passage of air. The size of drains, channels and pipe shall be such that not more than 50% of their cross-sectional area will be submerged under the maximum daily design hydraulic loading. The design of the effluent channels should consider the probability of increased hydraulic loading.

2. Filter covers shall be designed to allow adequate ventilation to maintain the filter in an aerobic state at all times.

(e) *Maintenance.* All distribution devices, underdrains, channels and pipes shall be installed so that they may be properly maintained, flushed and drained.

(f) *Flow measurement.* Devices shall be provided to permit measurement of flow to the filter, including the amount of recirculated flow.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83; CR 09-123: am. (5) (b) 2. and (6) (b) 1. Register July 2010 No. 655, eff. 8-1-10.

**NR 110.20 Rotating biological contactors.** (1) **GENERAL.** (a) *Applicability.* Rotating biological contactors may be used when the wastewater is amenable to biological treatment. This treatment process may be used for carbonaceous or nitrogenous oxygen demand reduction, or both.

(b) *Manufacturer's warranty.* Manufacturers of rotating biological contactor equipment shall guarantee the rotating shafts, and media against failure during the initial 5 years of operation for all proposed rotating biological contactor treatment systems. The guarantee shall include equipment replacement and installation costs.

(2) **DESIGN CONSIDERATIONS.** (a) *Design report.* A design report for rotating biological contactors shall be submitted in accordance with s. NR 110.15 (1).

(b) *Design parameters.* The design of rotating biological contactors shall consider:

1. Design flow rate;
2. Influent carbonaceous and nitrogenous biochemical oxygen demand;
3. Rotational velocity;
4. Wastewater temperature; and
5. Percent influent biochemical oxygen demand which is soluble.

(3) **DESIGN FEATURES.** (a) *Primary treatment.* Rotating biological contactors shall be preceded by primary treatment.

(b) *Contact tanks.* 1. Contact tanks shall be sized to maintain a maximum hydraulic detention time of 100 minutes.

2. Tanks shall contain positive liquid level control so that the rotating biological contactors will remain approximately 40% submerged.

3. Contact tanks and rotating shafts shall be enclosed. The enclosure shall be ventilated.

4. Removable baffles shall be provided between contact stages.

(c) *Equalization.* Equalization facilities shall be provided ahead of rotating biological contactors if the ratio of maximum hourly design flow to average design flow exceeds 2.5:1.

(d) *High density media.* High density shafts may not be used in the first 2 stages of any rotating biological treatment unit or system.

(e) *Rotational speed.* 1. Contactors shall be equipped with drive units which will allow variable rotational speed.

2. Maximum rotational speed shall be limited to a peripheral velocity of 49 centimeters per second (1.6 feet per second).

(f) *Load monitoring.* Each rotating biological shaft shall be equipped with a load monitoring device.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83.

**NR 110.21 Activated sludge.** (1) **APPLICABILITY.** The activated sludge process, and its various modifications, may be used where sewage is amenable to biological treatment.

(2) **DESIGN REPORT.** A design report shall be submitted in accordance with s. NR 110.15 (1).

(3) **DESIGN CONSIDERATIONS.** (a) *Process selection.* The choice of activated sludge process will be influenced by the degree of treatment needed to achieve the required effluent limits, the proposed treatment facility size, and the characteristics of the waste to be treated.

(b) *Winter protection.* Activated sludge processes and aeration equipment which are subject to freezing or icing shall be designed to minimize the degree of freezing and icing.

(c) *Pretreatment.* Where primary settling tanks are not used, effective removal of grit, debris, excessive oil or grease, and comminution or screening of solids shall be provided prior to the activated sludge process.

(d) *Measuring devices.* Devices shall be installed for measuring and displaying flow rates of raw sewage or primary effluent,

return sludge, and air to the aeration facilities. It is recommended that these devices totalize and record, as well as indicate, flows if the average design flow for the treatment plant is greater than 5,680 cubic meters per day (1.5 million gallons per day).

(e) *Equalization.* Equalization chambers shall be provided when large daily fluctuations of influent flow or organic loading are expected to occur.

(4) **AERATION TANKS.** (a) *Process design.* The size of aeration units for any particular adaptation of the activated sludge process shall be determined by pilot plant studies, or calculations based mainly on food to microorganism (F/M) ratio and mixed liquor suspended solids (MLSS) levels. Other factors such as size of treatment plant, diurnal load variations and degree of treatment required shall also be considered. In addition, temperature, pH bicarbonate hardness, and reactor dissolved oxygen shall be considered when designing for nitrification. The calculations used to determine the aeration capacity shall be included in the design report required by s. NR 110.15 (1). Designs based on mixed liquor suspended solids levels greater than 5,000 milligrams per liter will not be approved unless adequate data is submitted showing the aeration and settling systems are capable of supporting such levels.

(b) *Permissible loadings.* In lieu of the design calculation requirements of par. (a), the parameters shown in Table 5 may be used to design aeration tank capacities. The volumetric loadings in Table 5 shall be based on the organic load influent to the aeration tank at the average design BOD<sub>5</sub> loading rate.

**Table 5**

| Process               | F/M Ratio mg BOD <sub>5</sub> /d/mg MLVSS                  | Volumetric Loading kg BOD /m <sup>3</sup> /d (lbs. BOD /1000 ft <sup>3</sup> /d) | MLSS mg/l   |
|-----------------------|--|--|---|
| Conventional          | 0.2 - 0.5  | 0.6 (40)   | 1,000 - 3,000   |
| Step Aeration         | 0.2 - 0.5  | 0.6 (40)   | 2,000 - 3,500   |
| Complete Mix          | 0.2 - 0.6  | 0.8 (50)   | 3,000 - 5,000   |
| Contact-Stabilization | 0.2 - 0.6  | 0.8 <sup>a</sup> (50)  | 1,000 - 3,000 <sup>b</sup><br>4,000 - 10,000 <sup>c</sup> |
| Extended Aeration     | 0.05 - 0.15  | 0.25 (15)  | 3,000 - 5,000   |
| footnotes:            | a) total aeration<br>b) contact tank<br>c) reaeration tank | capacity   |   |

(c) *Number of units.* Multiple aeration tanks shall be provided where the average design flow exceeds 1,890 cubic meters (500,000 gallons) per day.

(d) *Aeration tank design features.* 1. The dimensions of each aeration tank or return sludge reaeration tank shall be such as to maintain effective mixing and use of air.

2. Liquid depths in aeration tanks may not be less than 3 meters (10 feet). The department may allow liquid depths to exceed 5 meters (16 feet) on a case-by-case basis.

3. Baffling or the placement of aeration equipment shall provide positive control of hydraulic short-circuiting through aeration tanks.

4. Process piping, influent channels and inlet structure shall be arranged to provide operational flexibility.

5. Inlets and outlets for each aeration tank unit shall be equipped with valves, gates, stop plates, weirs or other devices to permit controlling the flow to each tank and to maintain a constant liquid level. The hydraulic properties of the system shall

permit the peak instantaneous design flow to be carried with any single aeration tank unit out of service.

6. Channels and pipes carrying liquids with suspended solids shall be designed to maintain self-cleansing velocities or shall be agitated to keep the solids in suspension at all rates of flow within the design limits.

7. All aeration tanks shall have a freeboard of not less than 46 centimeters (18 inches).

(5) **AERATION SYSTEMS.** (a) *General.* The aeration system shall be capable of meeting the oxygen requirements of the activated sludge system, or of maintaining adequate mixing of the mixed liquor suspended solids, whichever is greater.

(b) *Oxygen demand.* 1. Aeration equipment shall be capable of maintaining a minimum mixed liquor dissolved oxygen concentration of 2 milligrams per liter.

2. In the absence of experimentally determined values, the design oxygen requirements for all activated sludge processes shall be 1.1 kilograms oxygen per kilogram peak hour BOD<sub>5</sub> (1.1 pounds oxygen per pound peak hour BOD<sub>5</sub>) removed in the aeration tanks, with the exception of the extended aeration process, for which the value shall be 1.5 kilograms oxygen per kilogram peak hour BOD<sub>5</sub> (1.5 pounds oxygen per pound peak hour BOD<sub>5</sub>) to include endogenous respiration requirements.

3. To provide nitrification, the oxygen requirement for oxidizing ammonia shall be added to the requirement in subd. 2. for carbonaceous BOD<sub>5</sub> removal and endogenous respiration requirements. In the absence of experimentally determined values, the nitrogen oxygen demand (NOD) shall be 4.6 kilograms of oxygen per kilogram removed peak hour total Kjeldahl nitrogen (TKN) (4.6 pounds oxygen per pound removed peak hour TKN).

(c) *Air supply to meet oxygen demands.* 1. The design of the aerator system to provide the oxygen requirements calculated in accordance with par. (b) shall be done using standard design equations for diffused and mechanical aeration systems. Calculations shall incorporate such factors as tank depth, alpha factor of the waste, beta factor of the waste, certified aerator oxygen transfer efficiency, minimum aeration tank dissolved oxygen concentration, critical wastewater temperature and altitude of the wastewater treatment facility.

2. In the absence of specific design information, the air requirements for diffused aerators shall be calculated using an oxygen transfer efficiency of 7% in clean water under standard test conditions. The air requirements for mechanical aerators shall be based on a transfer rate of 1.2 kilograms oxygen per kilowatt-hour (2 pounds oxygen per horsepower-hour) in clean water under standard test conditions.

(d) *Mixing requirements.* The following minimum requirements shall be met to insure adequate mixing of mixed liquor suspended solids.

1. Diffused aeration systems shall be capable of delivering a minimum air flow rate of 20 cubic meters per minute per 1,000 cubic meters (20 cubic feet per minute per 1,000 cubic feet) of aeration volume.

2. Mechanical aerators shall deliver a minimum of 15 kilowatts per 1,000 cubic meters (0.6 horsepower per 1,000 cubic feet) of aeration volume.

(e) *Other air-use demands.* The aeration system shall also be capable of providing the air required for channel aeration, air-lift pumps, aerobic digesters, and any other air-use demand.

(6) **AERATION EQUIPMENT.** (a) *Diffused aeration systems.* 1. Multiple blowers shall be provided. The blowers shall be sized to meet the maximum air demand with the largest blower out of ser-



vice. The design shall also provide for varying the volume of air delivered in proportion to the air demand of the plant.

2. Diffusers and air piping shall be capable of supplying the peak hour air demand or 200% of the design average air demand, whichever is larger.

3. The arrangement of diffusers shall permit their removal for inspection, maintenance and replacement without dewatering aeration tanks or channels and without shutting off the air supply to other diffusers in the treatment system. The department may waive this requirement for systems with multiple aeration tanks provided the treatment efficiency of the system can be maintained with one aeration tank out of service.

(b) *Mechanical aerators.* 1. Multiple mechanical aeration units shall be designed and located so as to meet the peak hour oxygen demand or 200% of the design average oxygen demand, whichever is larger, with one unit out of service.

2. Due to high heat loss, the mechanical aerators shall be protected from freezing.

(c) *Pure oxygen.* Where pure oxygen is proposed, supporting data from pilot plant installations or full-scale installations similar to the one proposed shall be submitted to justify the aerator loading rate and the amount and type of aeration capacity and equipment proposed.

(7) **SLUDGE EQUIPMENT.** (a) *Return sludge rate.* The rate of sludge return expressed as a percentage of the average design flow of sewage shall lie within the limits shown in Table 6:

Table 6

|                       | Minimum | Maximum |
|-----------------------|---------|---------|
| Conventional          | 15      | 75      |
| Step aeration         | 20      | 75      |
| Contact stabilization | 50      | 150     |
| Extended aeration     | 50      | 200     |
| Complete mix          | 20      | 75      |

(b) *Return sludge pumps.* 1. If motor driven return sludge pumps are used, the maximum return sludge capacity shall be met with the largest pump out of service. A positive head shall be provided on pump suction. Pumps shall also have at least 7.6 centimeter (3-inch) suction and discharge openings.

2. If air lifts are used for returning sludge from each settling tank hopper, no standby unit will be required provided the design of the air lifts allows rapid and easy cleaning. Air lift pumps shall be designed to provide positive control of the return sludge rate.

(c) *Return sludge piping.* Suction piping and discharge piping for returning activated sludge shall be at least 10 centimeters (4 inches) in diameter and must be designed to maintain a velocity of not less than 60 centimeters per second (2 feet per second) at normal return sludge rates. Suitable devices for observing, sampling and controlling return activated sludge flow from each settling tank shall be provided.

(d) *Waste sludge piping.* Waste sludge piping shall comply with the requirements of s. NR 110.26 (4)

(e) *Waste sludge pumps.* Variable speed or multiple constant speed waste sludge pumps shall be provided. The maximum sludge pumping rate shall be at least 200% of the anticipated volumetric sludge production rate. Devices for measuring waste activated sludge flow rates shall be provided.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83; CR 09-123: am. (4) (b), (d) 5., (5) (b) 2., 3., (c) 2., (6) a) 2., (b) 1. and Table 5 (title) Register July 2010 No. 655, eff. 8-1-10.

**NR 110.22 Physical-chemical treatment.** (1) **APPLICABILITY.** Physical-chemical treatment processes may be used where appropriate to achieve the required effluent limits.

(2) **DESIGN REPORT.** A design report shall be submitted in accordance with s. NR 110.05 (1). The report shall detail any lab testing, pilot plant studies or operating experience used to design the physical-chemical process.

(3) **CHEMICAL TREATMENT.** (a) *Chemical selection.* Selection of chemicals used in chemical treatment shall be based on the characteristics of the wastewater and constituents to be removed.

(b) *Design basis.* 1. Design of chemical treatment processes shall be based on laboratory testing, pilot plant studies or practical operating experience.

2. Design of chemical treatment equipment, reactors, and appurtenances shall consider:

- a. The chemical requirements and feed rates;
- b. The location of chemical injection into the waste stream;
- c. The need for rapid mixing and flocculation basins;
- d. The settling characteristics of the chemical sludge;
- e. The need for pH adjustment to optimize chemical reactions; and
- f. The velocity of waste streams in flow conduits to minimize destruction of floc.

(c) *Phosphorus removal.* 1. Addition of lime or the salts of aluminum or iron may be used for the chemical precipitation of soluble phosphorus.

2. The addition of polyelectrolytes to aid in the settling of phosphate precipitates should be considered.

3. Chemicals shall be mixed rapidly and thoroughly with the wastewater.

(4) **HAZARDOUS CHEMICAL HANDLING.** (a) *Safety.* 1. Eye-wash fountains and safety showers using potable water shall be provided in the laboratory and on each floor level or work location where hazardous chemicals are stored, mixed or slaked, pumped, metered or unloaded. These fountains and showers shall be less than 7.6 meters (25 feet) from points of exposure to hazardous chemicals and shall be fully usable during all weather conditions.

2. Eye-wash fountains shall be supplied with water with a temperature not exceeding 38°C (100°F). This supply shall be separate from the hot water supply and be able to provide 15 to 30 minutes of continuous irrigation of the eyes.

3. Safety showers shall be capable of discharging 1.9 to 3.2 liters per second (30 to 50 gallons per minute) of water with a temperature not exceeding 38°C (100°F) temperature, and at pressures of 1.41 to 3.52 kilograms force per square centimeter (20 to 50 pounds per square inch).

4. The following protective clothing and equipment shall be available for use with all operations or procedures in which their use will minimize the risk of injury to personnel:

- a. Chemical worker's goggles or other suitable goggles (safety glasses are insufficient);
  - b. Face masks or shields for use over goggles;
  - c. Rubber gloves;
  - d. Rubber aprons with leg straps;
  - e. Rubber boots; and
  - f. Safety harness and line.
5. Warning signs shall be installed where appropriate.

(b) *Storage requirements.* 1. The materials used for storing of hazardous chemicals shall be selected based on the physical and chemical characteristics of each chemical used.

2. Chemical storage areas shall be enclosed by dikes or curbs which will contain the stored volume in case of a spill until it can be either safely transferred to another storage area or released to

the wastewater at a controlled rate which will not damage the treatment facilities, inhibit the treatment processes, or contribute to stream pollution. Liquid polymer shall be similarly contained.

3. Chemical storage and mixing areas shall be separate from other treatment plant functions.

4. Nonslip floor surfaces are desirable in polymer handling areas.

(c) *Pumping and piping requirements.* 1. The materials used for piping, valves, pumping, metering, splash guards and any other equipment used to convey hazardous chemicals shall be selected based on the physical and chemical characteristics of each chemical used.

2. All piping containing or transporting hazardous chemicals shall be identified with labels every 3 meters (10 feet) and with at least 2 labels in each room, closet or pipe chase. Color coding may also be used but is not an adequate substitute for labeling.

3. All pumps or feeders for hazardous or corrosive chemicals shall have splash guards which will effectively prevent spray of chemicals into space occupied by workers. The splash guards are in addition to guards to prevent injury from moving or rotating machinery parts. All connections except those adjacent to storage or feeder areas shall have guards which will direct any leakage away from space occupied by workers.

4. Exposed pipes containing hazardous chemicals may not be located above shoulder level except where continuous drip collection trays and coupling guards will eliminate the spraying or dripping of these chemicals onto workers.

(5) **PHYSICAL TREATMENT.** (a) *Design.* Physical treatment shall be evaluated on a case-by-case basis. The design shall be based on pilot plant studies or operating experience.

(b) *Filtration.* 1. Selection of type, size, and depth of filter media shall depend on the filtration rate, the type of treatment provided prior to filtration, filter configuration, available hydraulic head, and the desired effluent quality.

2. Multiple filters shall be provided.

3. For high rate effluent filtration, the filtration rate at maximum hour design flow may not exceed 3.4 liters per second per square meter (5 gallons per minute per square foot). The filtration rate shall be calculated on the total available filter area with one filter unit out of service.

4. Provisions shall be made for backwashing each filter. The backwash system shall be capable of providing a variable backwash rate with a maximum rate sufficient to fluidize the filtering material. A minimum backwash period of 10 minutes shall be provided.

5. Air scour for aiding backwashing is recommended.

6. Provision shall be made for chlorinating each filter.

7. Backwash reservoirs shall be provided. Total backwash water storage provided shall equal or exceed the volume required for 2 complete backwash cycles.

8. Spent backwash water shall be individually treated or returned to the head of the treatment facility. The return rate of backwash to the head of the treatment facility may not exceed 15% of the average design flow rate.

(c) *Microstraining.* 1. Multiple screening units shall be provided.

2. The screening rate at maximum daily design flow may not exceed 3.4 liters per second per square meter (5 gallons per minute per square foot) based on submerged area with one screening unit out of service.

3. Provisions shall be made for backwashing each unit. The backwash system shall be capable of delivering at least 1.7 liters

per second per meter (8 gallons per minute per foot) of filter length. Backwash shall be delivered at 4.2 kilograms force per square centimeter (60 pounds per square inch).

4. Spent backwash shall be individually treated or returned to the head of the treatment facility. The return rate of backwash to the head of the treatment facility may not exceed 15% of the average design flow rate.

(6) **RECIRCULATING SAND FILTERS.** (a) *Applicability.* Recirculating sand filters may be approved on a case-by-case basis.

(b) *Primary treatment.* Recirculating sand filters shall be preceded by a minimum of primary treatment.

(c) *Recirculation tanks.* Recirculation tanks shall be equipped with a highwater and pump failure alarm.

(d) *Maintenance.* Recirculation tanks and sand filters shall be readily accessible for inspection and maintenance.

(7) **INTERMITTENT SAND FILTERS.** (a) *Applicability.* Intermittent sand filters may be approved on a case-by-case basis.

(b) *Primary treatment.* Intermittent sand filters shall be preceded by a minimum of primary treatment.

(c) *Loading.* 1. The loading rate for installations which operate with significant rest periods may not exceed 41 liters per square meter (one gallon per square foot) per day, at the average design flow.

2. The loading rate for filters which operate on a continuous basis may not exceed 20 liters per square meter (0.5 gallons per square foot) per day, at the average design flow, for total bed area.

(d) *General requirements.* 1. Duplicate filters shall be provided.

2. Intermittent sand filters shall be sealed in compliance with the provisions of s. NR 110.24 (4).

3. Intermittent sand filters shall be underdrained. Underdrains may be constructed of open jointed or perforated clay, concrete or plastic pipe. Underdrain spacing may not exceed 3 meters (10 feet) on center.

(e) *Media.* 1. Clean graded gravel shall be placed around the underdrains. Depth of the gravel shall be at least 15 centimeters (6 inches) over of the top of the underdrains.

2. At least 60 centimeters (2 feet) of clean sand shall be placed over the gravel. The effective size of the sand shall be 0.3 to 0.6 millimeter (0.01 to 0.02 inches) with a uniformity coefficient of 3.5.

(f) *Buried sand filters.* 1. Distribution piping shall have a minimum diameter of 10 centimeters (4 inches). Spacing of distribution pipes may not exceed 3 meters (10 feet). Distribution pipes may not be placed directly above underdrains.

2. The distribution piping shall be vented to the atmosphere.

3. Buried sand filters shall be dosed by pumps or siphons. The dosing volume shall be 90% of the volume of the distribution piping. The dosing system and distribution piping shall be sized to handle the average daily design flow.

4. Buried sand filters shall be covered by a minimum of 91 centimeters (36 inches) of soil.

(g) *Exposed sand filters.* 1. Exposed filters may be used when the filter will be designed to operate during the summer months.

2. Distribution troughs or piping shall be spaced not more than 6 meters (20 feet) on center.

3. Splash pads shall be provided at each point of discharge.

4. Exposed sand filters shall be dosed by pumps or siphons. The dosing volume shall be sufficient to cover the filter with 3 to 6 centimeters (2 to 4 inches) of effluent. The dosing system shall be sized to handle the average daily design flow.

5. Exposed sand filters shall have a freeboard of at least 60 centimeters (2 feet).

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83; CR 09-123: am. (5) (b) 3., (c) 4., (6) (c) and (7) (c) Register July 2010 No. 655, eff. 8-1-10; CR 12-027: am. (5) (b) 2., (c) 1. Register July 2013 No. 691, eff. 8-1-13.

**NR 110.23 Disinfection. (1) GENERAL.** (a) *Applicability.* Disinfection shall be provided in accordance with WPDES permit requirements.

(b) *Effluent limitations.* 1. When chlorine is used as the disinfectant, the residual concentration in the wastewater treatment facility effluent shall comply with WPDES permit requirements.

2. Effluent bacterial concentrations shall conform with WPDES permit requirements.

(c) *Process selection.* 1. The selection of the method of disinfection shall be based on a cost effective comparison of disinfection methods.

2. Where a disinfection process other than chlorine is proposed, supporting data from pilot plant installations or similar full-scale installations shall be submitted as a basis for the design of the system in accordance with s. NR 110.15 (1).

**(2) DISINFECTION WITH CHLORINE.** (a) *Storage requirements.* 1. If a gas chlorinator and chlorine cylinders are installed in a building used for other purposes, the chlorinator and chlorine cylinders shall be separated from all other portions of the building by being kept in a gas-tight room. Doors to this room shall open only to the outside of the building, and shall be equipped with panic or emergency hardware. Those rooms shall be at or slightly above grade and must permit easy access to all equipment. If one ton or larger cylinders of chlorine are used, the chlorination equipment shall be kept in a room separate from the chlorine cylinders.

2. A clear glass, gas-tight window shall be installed in an exterior door or interior wall of the chlorinator room to permit the chlorinator to be viewed without entering the room.

3. Chlorinator and chlorine cylinder storage rooms shall be provided with a means of heating so that a temperature of at least 16°C (60°F) can be maintained. The rooms shall be protected against temperatures exceeding 65°C (140°F).

4. Forced, mechanical ventilation of the chlorinator room and the chlorine storage room shall be installed. Ventilation equipment shall be capable of providing one complete air change per minute. The entrance to the air exhaust duct from the room shall be near the floor and the point of discharge shall be so located as not to contaminate the air inlet to any buildings or inhabited areas. Air inlets shall be so located as to provide cross ventilation and at such a temperature that will not adversely affect the chlorination equipment. The vent hose from the chlorinator shall discharge to the outside atmosphere above grade.

5. The controls for the ventilator and lights shall be such that the ventilator and lights will automatically operate when the door is opened but must be manually switched off even if the door closes. Switches shall also be provided to allow manual operation of the lights and ventilator from outside of the room without opening the door.

6. One ton cylinders shall be used at treatment facilities where the chlorine use rate exceeds 68 kilograms (150 pounds) per day.

(b) *Feed equipment.* 1. Solution-feed vacuum type chlorinators or positive displacement type hypochlorite feeders shall be used.

2. An ample supply of water shall be supplied for operating the chlorinator.

3. Chlorinators and feed equipment shall be sized to handle the maximum design chlorine demand.

4. Chlorinators shall be equipped to provide control of chlorine application through the full range of design chlorine demand. If necessary, more than one rotometer shall be supplied to ensure control of chlorine application through the design range.

5. Scales or other means of determining chlorine usage shall be provided. Scales shall be of corrosion-resistant material.

6. Evaporators for converting liquid chlorine to a gas may be used if necessary.

(c) *Piping and connections.* 1. Only piping systems specifically manufactured for chlorine service shall be used.

2. Due to the corrosiveness of wet chlorine, all lines designed to handle dry chlorine shall be protected from the entry of water or air containing water.

(d) *Chlorine control systems.* In all systems with an average design flow of greater than 945 cubic meters per day (0.25 million gallons per day), the chlorine feed mechanism shall be provided with either an automatic flow proportional control or an automatic residual control. Chlorine residual analyzers shall be located near the chlorine contact tank. The total response time for automatic residual control systems may not exceed 3 minutes.

(e) *Application.* 1. The chlorine shall be mixed as rapidly as possible. This may be accomplished by either the design of a turbulent flow regime or the use of a mechanical flash mixer.

2. A chlorine contact tank shall be provided and shall be sized to provide a detention time of 60 minutes at average design flow or 30 minutes at maximum hour design flow.

3. Chlorine contact tanks shall be baffled to provide a flow channel with a minimum length to width ratio of 40:1.

4. The department may approve contact tanks which do not comply with the requirements of this paragraph. Such facilities, however, shall be field tested to demonstrate that short circuiting of hydraulic flow through the contact chamber does not occur.

(f) *Dechlorination.* 1. Dechlorination shall be provided in accordance with WPDES permit requirements.

2. Dechlorination chemicals shall be rapidly mixed with the effluent.

3. Sulfur dioxide dechlorination systems shall be designed in the same manner as chlorination systems.

4. Effluent reaeration shall be provided after dechlorination if necessary to insure adequate dissolved oxygen concentration in the receiving stream.

5. Dechlorinated effluent shall be monitored for chlorine residual and dissolved oxygen in accordance with WPDES permit requirements.

(g) *Safety equipment.* 1. Respiratory air-pack protection equipment, meeting the requirements of the national institute for occupational safety and health (NIOSH) shall be available where chlorine gas is handled, and shall be stored at a convenient location. The equipment may not be stored inside any room in which chlorine is used and stored. The equipment shall use compressed air or oxygen, have at least a 30-minute capacity, and be compatible with the units used by the fire department having jurisdiction over the plant.

2. A plastic bottle of ammonium hydroxide shall be provided for the detection of chlorine leaks.

3. Leak repair kits shall be provided when one ton chlorine cylinders are used.

**(3) ULTRAVIOLET DISINFECTION.** Provisions shall be made to clean ultraviolet units without loss of disinfection. This shall be accomplished by installing multiple ultraviolet units, by provid-



ing ultrasonic cleaners, or by providing an effluent holding tank with a capacity of one hour detention at average design flow.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, March, 1978, No. 267, eff. 4-1-78; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83; CR 09-123: am. (2) (d) and (e) 2. Register July 2010 No. 655, eff. 8-1-10.

**NR 110.24 Lagoons. (1) DESIGN REPORT.** A design report shall be submitted in accordance with s. NR 110.15 (1).

**(2) BASIS OF DESIGN. (a) Number of cells.** A minimum of 2 treatment cells shall be provided for aerated lagoons and stabilization ponds. Where a controlled discharge is required, additional effluent storage cells shall be provided.

1. For aerated lagoons designed to treat domestic wastewater only, the hydraulic detention time of each cell shall be based on the following formula:

$$T = \frac{E}{K(100 - E)}$$

Where:

T = detention time, days

E = BOD removal efficiency, percent

K = Reaction coefficient (log base e), days<sup>-1</sup>

a. For domestic wastewater K = 0.5 at 20°C.

b. The reaction coefficient (K) must be adjusted for temperature according to the formula:

$$K_T = K_{20}^{\Theta^{T-20}}$$

Where:

K<sub>T</sub> = Corrected reaction coefficient

K<sub>20</sub> = 0.5

Θ = 1.07

T = Low design temperature, °C

2. The appropriate summertime and wintertime reaction coefficients for aerated lagoons designed to treat combined domestic and industrial wastewater shall be determined from laboratory or pilot studies, or from operating data of existing full scale aerated lagoons which are treating similar wastewater. The reaction coefficients developed shall be used to calculate the required detention time.

3. In addition to the treatment volume calculated in subd. 1. or 2., quiescent settling zone or cell shall be provided for aerated lagoon systems. Minimum settling time shall be 6 days for surface water discharge, and 3 days for land disposal discharge.

4. Aerated lagoons designed to treat combined domestic and industrial wastewater shall be provided with the means to recirculate final lagoon effluent to the first treatment cell.

**(b) Stabilization ponds.** 1. Stabilization ponds may be used to treat domestic wastewater. Combined domestic and industrial wastewater may be treated in stabilization ponds only if the treatability of the industrial wastewater is demonstrated through pilot testing.

2. The BOD<sub>5</sub> loading to any one stabilization pond may not exceed 23 kilograms per hectare (20 pounds per acre) per day.

3. A minimum hydraulic detention time of 150 days at the average design flow shall be provided in the entire stabilization pond system. In accordance with s. NR 210.06 (3) (h), a stabilization pond system which discharges to surface water, and has a hydraulic detention time of 180 days or longer at average design flow, does not require disinfection except in extenuating circumstances.

**(3) DESIGN REQUIREMENTS. (a) Location.** Lagoon systems shall be located in compliance with s. NR 110.15 (3) (b) and (c).

**(b) Separation from groundwater.** 1. For all lagoons not sealed with a synthetic liner, a minimum separation distance of

1.25 meters (4 feet) shall be maintained between the bottom of lagoons and the highest recorded or indicated seasonal groundwater table elevation.

2. For all lagoons sealed with a synthetic liner, a minimum separation distance of 60 centimeters (2 feet) shall be maintained between the bottom of the lagoon and the highest recorded or indicated seasonal groundwater table elevation.

**(c) Separation from bedrock.** A minimum separation of 3 meters (10 feet) shall be maintained between the bottom of lagoons and bedrock. The department may waive this requirement on a case-by-case basis if it can be demonstrated that a lesser separation distance will not cause groundwater quality problems. Criteria which will be evaluated to waive this requirement include the depth to bedrock, the type of bedrock, the fracture condition of the bedrock, the direction of groundwater movement, the existing groundwater quality, and the downgradient uses of the groundwater.

**(d) Test pits and soil borings.** 1. Backhoe test pits and soil borings shall be conducted at each proposed lagoon site. Logs of the test pits and soil borings shall be submitted with the facilities plan as required in s. NR 110.09 (8) (a). Soil boring and test pit analyses shall be conducted by an independent soil testing laboratory, a qualified engineering firm or an individual or firm which has demonstrated the capability to perform and evaluate such tests.

2. Soil borings and test pits shall be used to determine subsurface soil characteristics and variability, seasonal high groundwater level and elevations, and type, nature and depth to bedrock. Soils shall be classified according to the unified soil classification system. Cross-sections using the soil boring and test pit logs shall be prepared and submitted with the facilities plan.

3. Soil sampling shall be performed in accordance with ASTM D1586-08a or ASTM D1587-08.

4. Soil profile descriptions shall be written for all soil test pits. The thickness in inches and the difference between each soil horizon shall be indicated for each test pit. Horizons shall be differentiated on the basis of color, texture, soil mottles or bedrock. Depth shall be measured from the ground surface and the slope at the test pit shall be indicated.

5. A minimum of one soil boring per acre shall be conducted at each lagoon site. The number of test pits and borings shall be sufficient to adequately characterize the soil type and variability and delineate unsuitable soil areas in the field. The department may require additional soil borings and test pits to properly describe the site soils, bedrock or groundwater conditions.

6. Each boring shall have a minimum depth of 7.6 meters (25 feet) or to bedrock.

7. All soil borings in which wells are not installed shall be properly abandoned according to s. NR 141.25.

8. All test pits shall be refilled with the excavated materials.

**(e) Lagoon shape.** The shape of lagoons shall be such that there are no narrow or elongated portions. Islands, peninsulas or coves will not be approved. Dikes shall be rounded at corners to minimize accumulations of floating materials. Commonwall dike construction is encouraged. Round, square or rectangular lagoons with a length not exceeding 3 times the width are recommended.

**(f) Dikes.** 1. The minimum top width of dikes shall be 3.6 meters (12 feet).

2. Maximum dike slopes shall be 3:1 (horizontal to vertical).

3. The minimum allowable interior slope shall be 4:1.

4. A minimum one meter (3 feet) freeboard from operating water surface to the top of dikes shall be provided.

(g) *Operating water depth.* 1. A minimum liquid depth of 0.6 meters (2 feet) for stabilization ponds and 1.8 meters (6 feet) for aerated lagoons shall be provided.

2. Maximum water depth may not exceed 1.8 meters (6 feet) for stabilization ponds and 4.3 meters (15 feet) for aerated lagoons.

(4) **SEALING REQUIREMENTS.** (a) *General.* All lagoons shall be sealed to prevent excessive exfiltration.

(b) *Exfiltration rate.* 1. Loss of water from wastewater treatment or storage lagoons may not exceed 10 cubic meters per water surface hectare (1,000 gallons per acre) per day and loss of water from sludge storage or treatment lagoons or other sludge handling facilities may not exceed 5 cubic meters per sludge surface hectare (500 gallons per acre) per day.

2. In circumstances where soil or groundwater characteristics, groundwater quality, or waste characteristics warrant, the department may require exfiltration rates less than 10 cubic meters per water surface hectare (1,000 gallons per acre) per day for wastewater treatment or storage lagoons.

(c) *Materials.* 1. Soil materials or synthetic liners approved by the department may be used to seal lagoons.

2. Soil materials or synthetic liners used to seal lagoons shall be compatible with the wastewater characteristics.

(d) *Sampling and testing standards.* 1. Core samples taken to determine soil texture, grain size distribution or permeability shall be taken in accordance with ASTM D1586-08a, ASTM D1587-08, or ASTM 3550-01 (2007).

2. Permeability shall be determined using a falling head permeability test. The test shall be performed at the same approximate density as the in-place field condition. Tests on remolded or undisturbed samples are acceptable.

3. Sieve analyses performed to determine grain size distribution shall be performed in accordance with ASTM D422-63 (2007).

4. Plasticity index shall be determined in accordance with ASTM D4318-10.

5. Standard proctor densities shall be determined in accordance with ASTM D698-07 e1.

(e) *Uniform construction.* All lagoon seals shall be uniformly constructed across the lagoon bottom and interior dike walls. Seals shall extend up the dike wall to the berm.

(f) *Synthetic liners.* 1. Synthetic liners shall have a minimum thickness of 0.8 millimeters (30 mils).

2. All synthetic liners shall be installed under the supervision of a qualified manufacturer's representative.

3. Synthetic liners shall be protected by an inorganic soil layer. The soil layer shall have a minimum thickness of 30 centimeters (one foot). The soil shall be uniformly graded and free from large rocks, angular stones, soil clumps, sticks or other material which may puncture the liner. When a granular, noncohesive soil is used for the cover, a soil fabric shall be placed between the liner and the soil cover. The soil fabric shall be anchored at the dike berm.

4. Synthetic liners shall be securely anchored to the dike berm.

5. Synthetic liners shall be vented.

6. Riprap or other means of erosion control shall be provided to prevent exposure of the synthetic liner due to erosion of the protective soil layer.

7. Prior to constructing the synthetic liner, the underlying soils shall be treated with a herbicide in accordance with manufacturers recommendations.

(g) *Soil or soil-bentonite liners.* 1. The permeability of soil or bentonite liners may not be greater than  $1 \times 10^{-7}$  cm/sec. ( $2.83 \times 10^{-4}$  ft/day).

2. The liner thickness shall be determined according to Darcy's equation, and shall include an appropriate safety factor for construction variability. In no case shall the liner thickness be less than the minimum values shown in Table 7.

3. When the soil or soil-bentonite liner is to be constructed over the existing soil at the lagoon site, 15% of the soil particles of the existing soil must pass a no. 200 sieve. If this requirement cannot be met, a soil filter fabric material shall be placed between the liner and the existing soil.

4. Liners shall be compacted at or above optimum moisture content.

5. A means shall be provided to prevent the liner from desiccating after the completion of construction and prior to placing the system in operation.

6. Liners shall be protected by an inorganic soil layer. The soil layer shall have a minimum thickness of 10 centimeters (4 inches). The cover shall be uniformly graded and free from large rocks, soil clumps, and sticks.

**Table 7**  
**Minimum Liner Thickness**

| Coefficient of Permeability<br>cm/sec (ft/day) | Centimeters<br>(Inches) |           | Water Depth<br>Meters (feet) |             |
|--|-------------------------|-----------|------------------------------|-------------|
|  | 1.8<br>(6)              | 3<br>(10) | 3.8<br>(12)                  | 4.6<br>(15) |
| $1 \times 10^{-7}$                             | 22 cm                   | 33 cm     | 40 cm                        | 48 cm       |
| $(2.83 \times 10^{-4})$                        | (9 in)                  | (13 in)   | (16 in)                      | (19 in)     |
| $5 \times 10^{-8}$                             | 14                      | 19        | 23                           | 27          |
| $(1.42 \times 10^{-4})$                        | (6)                     | (8)       | (8)                          | (11)        |
| $1 \times 10^{-8}$                             | 10                      | 10        | 10                           | 10          |
| $(2.83 \times 10^{-5})$                        | (4)                     | (4)       | (4)                          | (4)         |
| $5 \times 10^{-9}$                             | 10                      | 10        | 10                           | 10          |
| $(1.42 \times 10^{-5})$                        | (4)                     | (4)       | (4)                          | (4)         |
| $1 \times 10^{-9}$                             | 10                      | 10        | 10                           | 10          |
| $(2.83 \times 10^{-6})$                        | (4)                     | (4)       | (4)                          | (4)         |

(h) *Soil liner material specifications.* 1. Soil liners shall consist of soils of which more than 50% of the soil particles pass a no. 200 sieve. The soil liner shall have a plasticity index of at least 15.

2. Soil liners shall be compacted to at least 95% of the maximum standard proctor density.

3. Soil liners shall be constructed and compacted in lifts. Each lift may not exceed a compacted thickness of 15 centimeters (6 inches).

4. Frost susceptible soils may not be used to construct the liner. Any soil which is primarily silt, silty sand, or lean clay which has a plasticity index less than 12 shall be considered as frost susceptible.

5. Soil liners constructed of natural in-place soils shall be scarified prior to compaction.

(i) *Bentonite liner material specifications.* 1. Bentonite shall be mixed with a soil in which at least 30% of the soil particles pass a no. 200 sieve. The soil shall have a plasticity index of at least 15.

2. Bentonite shall be applied at a rate recommended by the manufacturer or independent soil expert. The constructed liner shall have a minimum bentonite content of 5% by dry weight.

3. Ninety percent of the bentonite by weight shall pass a no. 80 sieve.

4. Bentonite shall be thoroughly mixed with the soil material.

5. The bentonite liner shall be compacted to at least 85% of the maximum standard proctor density.

(j) *Construction quality testing.* 1. All liners shall be tested before placing the lagoons into operation to insure compliance with par. (b). Test results shall be submitted to the department.

2. The method of testing shall be presented to the department with the project plans and specifications.

3. Testing shall be performed in accordance with one of the testing methods of par. (k).

4. All tests shall be performed under the supervision of the design engineer.

(k) *Testing methods.* 1. All liners may be tested using an in-field full lagoon water balance. The test shall occur over a minimum 14-day period. The manner of determining precipitation and evaporation rates shall be shown in the project plans and specifications.

2. The integrity of the field constructed seams for synthetic liners shall be tested with compressed air prior to placing the protective soil cover. All faulty seams shall be repaired and retested.

3. Core samples of soil or soil-bentonite liners may be taken and the liner thickness and permeability measured in a laboratory. Core samples shall be taken in accordance with ASTM D1587 (1974). A minimum of 12 samples per wetted hectare (5 samples per wetted acre) must be analyzed. The samples shall be proportionately taken from the lagoon bottoms and dikes. The lagoon liner shall be considered to meet the performance standard of par. (b) if:

a. The average seal thickness of the samples are equal or to greater than the specified design thickness. No sample shall have a thickness more than 1-inch less than the design thickness; and

b. The coefficient of permeability of 90% of the samples must be equal to or less than the design coefficient of permeability.

**(5) CONSTRUCTION DETAILS.** (a) *Material.* 1. Embankments and dikes shall be constructed of relatively impervious materials and compacted at near optimum moisture content to 95% of the standard proctor density.

2. Vegetation and other unsuitable materials shall be removed from the area where the embankment is to be placed.

(b) *Erosion control.* 1. Riprap or other means of preventing erosion shall be used at locations on lagoon bottoms and interior dike walls where erosion or activity of burrowing animals is likely to occur.

2. Riprap or other erosion control methods shall be used on the exterior dike walls for lagoons which are constructed in a flood fringe.

3. Exterior dike walls, berms and interior dike walls above the normal operating water depth, shall be riprapped or seeded with perennial, low growing, spreading grasses.

(c) *Fencing.* Lagoons shall be enclosed within a fence. A vehicle access gate shall be provided.

(d) *Warning signs.* Appropriate signs shall be provided along the fence surrounding lagoons to designate the nature of the facility and prohibit trespassing.

**(6) AERATION EQUIPMENT.** (a) *Air requirements.* Air shall be provided to the aerated lagoons at a rate of not less than 1.5 kilograms oxygen per kilogram (1.5 pounds of oxygen per pound) of peak hour BOD removed.

(b) *Surface aeration equipment.* 1. The department may approve the use of surface aeration equipment only in those cases in

which the equipment can be properly maintained and operated during the winter.

2. Surface aeration equipment shall be so designed and placed to provide optimum mixing of pond lagoon contents and dispersion of oxygen to the waste. Unless sufficient justification is presented to the contrary, surface aerators shall be designed using an oxygen transfer rate of 1.2 kilograms of oxygen per kilowatt-hour (2.0 pounds of oxygen per horsepower-hour) in clean water under standard conditions.

(c) *Subsurface aeration equipment.* 1. Flexible tubing containing air release slits shall be provided across the lagoon bottom in accordance with the manufacturer's recommendations. Air tubing shall be securely anchored to prevent floating. To prevent clogging of the air lines, provision shall be made to accommodate cleaning.

2. Air tubing and anchors shall be constructed of materials which resist corrosion.

3. Air shall be supplied to the lagoon system at a rate sufficient to meet the oxygen requirements of par. (a) assuming an oxygen transfer efficiency of 7% in clean water under standard conditions.

4. Tubular aeration units shall be provided in sufficient number to supply adequate air to the pond system based on a maximum transfer rate of 0.6 kilograms (1.25 pounds) of oxygen per unit per hour in clean water under standard conditions.

5. Where data is presented to the department to justify oxygen transfer rates varying from the requirements of this paragraph the department may approve such design transfer rates.

(d) *Aeration systems.* 1. Multiple blowers shall be provided. Capacity of the blowers shall be sufficient to meet total air demands with one blower out of service.

2. Diffusers and air piping shall be capable of supplying 200% of the average daily air demand.

**(7) HYDRAULIC STRUCTURES.** (a) *Materials.* Influent lines, interconnecting piping, and overflow structures shall be constructed of materials suitable for underground gravity sewer construction.

(b) *Capacity.* 1. Influent lines to all lagoon systems shall be sized in accordance with s. NR 110.13 (4).

2. Overflow structures and interconnecting piping for continuous flow lagoon systems shall be sized in accordance with s. NR 110.13 (4).

3. Overflow structures and interconnecting piping for controlled discharge lagoon systems shall be sized to handle the anticipated interlagoon flow rates during periods of discharge.

(c) *Influent piping.* 1. A manhole shall be installed at the end of the influent line or force main and shall be located as close to the dike as topography permits. Its invert shall be at least 15 centimeters (6 inches) above the maximum operating water level of the lagoon to provide sufficient hydraulic head without surcharging the manhole.

2. Influent lines shall be located such that the top of the pipe is at least 15 centimeters (6 inches) below the lower surface of the soil, bentonite, or synthetic liner.

3. For circular lagoons, the inlet shall terminate at the center of the lagoon. Influent lines to rectangular or square lagoons shall terminate in the first one third of the lagoon length. Influent and effluent piping shall be located to minimize short-circuiting within the lagoon.

4. The inlet line shall discharge either horizontally onto a concrete pad or by means of an upturned elbow terminating at least 30 centimeters (one foot) above the pond bottom.



(d) *Overflow structures.* An overflow structure shall be provided and shall consist of either a manhole or box equipped with multiple-valved pond drawoff lines or an adjustable overflow device. The overflow structure shall allow the liquid level of the lagoon to be adjusted to permit operation at depths ranging from 60 centimeters (2 feet) to the maximum design operating depth in stabilization ponds and from 1.2 meters (6 feet) to the maximum design operating depth in aerated lagoons. The department recommends that stop planks not be used in overflow structures to control operating depth.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83, am. (3) (c) and (4) (b), r. and rec. (3) (d), Register, November, 1990, No. 419, eff. 12-1-90; CR 09-123; am (2) (b) 2., 3., (3) (d) 3., (4) (d) 1., 3. to 5., (6) (a), (b) 2., (c) 3. and 4. Register July 2010 No. 655, eff. 8-1-10.

**NR 110.25 General conditions required for all land disposal systems.** The provisions of this section apply to all municipal and privately owned domestic sewage treatment works.

(1) **APPLICABILITY.** Land disposal systems shall be reviewed and approved on a case-by-case basis.

(2) **DESIGN REPORT.** A design report shall be submitted in accordance with s. NR 110.15 (1).

(3) **TREATMENT PRIOR TO DISPOSAL.** All discharges to land disposal systems shall receive biological, chemical, physical or a combination of treatments necessary to meet effluent standards in ch. NR 206 and groundwater quality standards in ch. NR 140 as approved by the department. Industrial waste discharges tributary to the municipal system shall be in compliance with applicable pretreatment standards under s. NR 211.30.

(3m) **MANAGEMENT PLAN.** (a) A management plan shall be submitted with plans and specifications for all land disposal facilities.

(b) The management plan shall contain specific information on pretreatment processes, scheduled maintenance, vegetative cover control and removal, load and rest schedules, application rates, operational strategies for periods of adverse weather, monitoring procedures and other pertinent information.

(4) **DESIGN REQUIREMENTS.** (a) *Application rates.* 1. The application rate of wastewater may not exceed the long term infiltrative capacity of the soil.

2. The application rate of wastewater containing heavy metals may not exceed the soil capacity for preventing the movement of the heavy metals through the soil.

3. Multiple wastewater application areas shall be provided to allow load and rest cycles. The discharge shall be alternately distributed to individual cells of the disposal system in a manner to allow sufficient resting periods to maintain the absorptive capacity of the soil, and to allow soil conditions to become unsaturated and aerobic between loadings.

(c) *Separation from water supplies.* 1. Land disposal systems shall be separated from private water supply wells by a minimum horizontal distance of 76 meters (250 feet).

2. The minimum horizontal separation distance between a land disposal system and public water supply wells shall be determined during facilities planning in accordance with s. NR 110.09 (2) (p). In all cases the department recommends a minimum horizontal separation of 305 meters (1,000 feet) be maintained.

(e) *Storage lagoons.* Storage lagoons shall be provided for all land disposal systems which are adversely affected by winter conditions or wet weather. Storage lagoons shall be constructed in accordance with s. NR 110.24 (3) and (4).

(f) *Load and rest cycles.* Load and rest cycles for each system shall be determined based on hydrogeologic and other relevant site conditions such as soil permeability, texture, cation-exchange

capacity, topography, depth to groundwater and bedrock and the wastewater characteristics.

(g) *Construction precautions.* 1. All precautions shall be taken during construction of a land disposal system to minimize compaction of absorption areas and to prevent reduction in soil infiltration rate. Project specifications shall detail the specific precautions to take, which may include no heavy equipment use and erosion control on berms.

2. Erosion control measures shall be practiced during the construction of the land disposal system to avoid erosion of soil into a surface water and into or from the land disposal system.

(5) **GROUNDWATER MONITORING.** (a) *Applicability.* Groundwater monitoring systems shall be installed in accordance with approved plans and specifications as required in ch. NR 108, for the purpose of determining groundwater conditions for the engineering report in s. NR 110.09 (1) (b) and for WPDES permit requirements. Plans and specifications shall be prepared by a hydrogeologist or other qualified person.

**Note:** The skills and knowledge required of a hydrogeologist making submittals under this chapter include: the ability to apply hydrogeologic principles and practices to the siting, design and operation of land disposal systems; knowledge of contaminants associated with land disposal of wastewater, their transport mechanisms and fate in the environment; familiarity with environmental monitoring practices, sampling techniques and groundwater standards; and proficiency in the design of groundwater monitoring systems for defining the physical and chemical characteristics of groundwater flow. A soil scientist or other environmental scientist who can demonstrate the above skills and knowledge, as reflected in submittals made under this chapter, shall be deemed a "qualified person".

(b) *Well locations.* 1. A minimum of 3 monitoring wells per land disposal system shall be installed to monitor groundwater quality in accordance with s. NR 206.10 (2), to determine flow directions and for a flow rate determination. At a minimum, one well will be upgradient and one well downgradient of the land disposal system.

2. One or more upgradient monitoring wells shall be installed at locations and depths sufficient to yield groundwater samples that are representative of background water quality near the facility. Selection of well locations should take into account past and present land uses which might affect groundwater quality. The upgradient well should be located so it will not be affected by the land disposal system. Any upgradient wells should be located at the most distant point of upgradient of the application area, and not closer than 75 feet from the application area. If the well is located beyond the property boundary, an easement for access shall be obtained prior to installation of the well.

3. Downgradient wells shall be located so as to intercept any groundwater impacted by the land disposal system, considering the vertical and horizontal gradients of flow. The wells shall be no closer than 30 feet for rapid infiltration systems, and at a minimum, one well shall be located between the application area and the design management zone (DMZ) or property boundary. It is recommended that one well be located at or beyond the DMZ or property boundary. If the well is located beyond the property boundary, an easement for access shall be obtained prior to installation of a well.

(c) *Monitoring well construction.* All groundwater monitoring wells shall be constructed in accordance with ch. NR 141 and this section.

1. a. For piezometers, inlet screens shall have a length of at least 2.5 feet but not more than 5 feet unless otherwise approved by the department.

b. For water table observation wells, inlet screens shall not exceed 10 feet in length.

c. For a multilevel groundwater system, the deeper the well shall be screened at a depth to be determined from the site investigation.

2. All groundwater monitoring wells shall be developed in accordance with s. NR 141.21.

3. All groundwater monitoring systems shall be sampled in accordance with department published groundwater sampling procedures as referenced in s. NR 140.16.

4. All unsuccessful wells, boreholes or other vertical holes and wells whose use is no longer required must be properly abandoned in accordance with s. NR 141.25.

5. Documentation of well construction, well development and abandonment shall be submitted to the department in accordance with ss. NR 141.23 and 141.25. A location map shall also be provided in accordance with s. NR 141.065.

(d) *Alternative methods and materials.* The department may approve alternative construction methods or materials for installation of groundwater monitoring wells on a case-by-case basis.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83; cr. (intro.), (3m), (4) (a) 3. and (4) (g), am. (2), r. and recr. (3), (4) (f) and (5), Register, November, 1990, No. 419, eff. 12-1-90; CR 09-123: am. (5) (b) 1., 3., (c) 3. and 5. Register July 2010 No. 655, eff. 8-1-10.

**NR 110.255 Conditions required for specific types of land disposal systems. (1) ABSORPTION POND SYSTEMS.**

(a) *Design and construction criteria for absorption pond systems.* 1. New absorption pond systems shall consist of a minimum of 3 individual absorption ponds of approximately equal size. Absorption pond systems consisting of 1 or 2 individual ponds may be approved by the department on a case-by-case basis if it is demonstrated that the system has effluent storage capabilities or other provisions to ensure the operation of the system in accordance with the load and rest cycles determined under s. NR 110.25 (4) (f).

2. The design hydraulic application rate for an absorption pond system shall be based on field and laboratory test results for infiltration and hydraulic conductivity. The design hydraulic application rate shall be conservatively established to allow for pond resting cycles and for a long term reduction in infiltration rate due to wastewater solids clogging the soil.

3. Multiple pond systems shall be designed and constructed to allow individual ponds to be taken out of service for resting without interrupting the discharge to the remaining ponds.

4. Wastewater effluent shall be discharged to absorption ponds such that it is evenly distributed over the entire absorption pond bottom. Effluent storage may be required to provide effluent dosing control by fill and draw operation.

5. The absorption pond bottom shall be as level as possible at all locations.

6. The shape of each absorption pond and the placement of ponds at the site must take into account the information in the hydrogeologic study required by s. NR 110.09 (8) such as the groundwater flow direction, the presence of discharge or recharge zones and the variability of soils. Infiltration areas should be oriented in relation to the direction of groundwater flow in such a manner as to minimize groundwater impacts. When possible, absorption ponds shall be constructed in areas which are not groundwater recharge areas.

7. The minimum top width of an embankment or dike shall be 12 feet if the dike is intended to provide access for maintenance vehicles on a routine basis. The minimum top width shall be 8 feet if the embankment or dike is not designed for vehicle access. Outside embankment and dike slopes may not be steeper than 3 horizontal to one vertical and shall be properly seeded with a mixture of grasses to prevent erosion. Inside embankments and dikes may not be steeper than 2 horizontal to one vertical and shall be properly graveled or ripped to prevent erosion. Interior ramps for maintenance vehicle access are acceptable.

8. Absorption ponds may not be constructed on backfilled material. Earthwork activities within 1 foot of the final pond surface shall be limited to times when soil conditions are dry.

9. The bottom of the absorption pond may not be closer than 5 feet to the highest anticipated groundwater elevation.

10. An absorption pond system shall be constructed on soils which meet with the following minimum requirements:

a. Soil texture may not be coarser than loamy sand (USDA soils classification) or have less than 5% passing a number 200 sieve.

b. Soil texture may not be finer than clay loam (USDA soil classification) or have liquid limits greater than 50% (unified soil classification).

c. Soil pH may not be less than 6.5.

11. A minimum separation distance of 10 feet shall be maintained between the bottom of the absorption pond and bedrock.

(b) *Discharge limitations for absorption pond systems.* Effluent limitations are as specified in s. NR 206.08 (1) (b).

(2) **SPRAY IRRIGATION SYSTEMS.** (a) *Design and construction criteria for spray irrigation systems.* 1. All spray irrigation systems shall be designed with a wastewater distribution system capable of loading and resting various portions of the site to optimize wastewater treatment within the soil and crop growth.

2. Spray irrigation onto frozen ground is prohibited. The department may restrict loadings during times of the year when the cover crop is not actively growing.

3. Application of wastewater to the spray irrigation system shall incorporate a rest/load cycle and application intensity such that the soil moisture holding capacity in the top foot of the soil column is not exceeded and ponding or runoff do not occur. Following wastewater application to a portion of the field, that portion shall be rested. Table 8 provides values for the maximum volume of wastewater that may be applied per load cycle and the maximum intensity of wastewater application for specific soil textures. The values in Table 8 are the maximum amount approvable unless greater values can be justified through soil testing and are approved by the department. The volume applied and the intensity sprayed may be restricted by the department to values less than those listed in Table 8 if site conditions warrant.

**Table 8**

| Soil Texture (USDA - SCS) | Maximum Volume Applied Per Load Cycle | Maximum Intensity of Application |
|---------------------------|---------------------------------------|----------------------------------|
| Sands                     | 0.65 inches                           | 1.00 in/hr                       |
| Sandy Loams               | 0.90 inches                           | 0.90 in/hr                       |
| Loams                     | 1.30 inches                           | 0.45 in/hr                       |
| Silt Loams                | 1.40 inches                           | 0.45 in/hr                       |
| Clay Loams                | 1.10 inches                           | 0.40 in/hr                       |
| Clays                     | 0.70 inches                           | 0.40 in/hr                       |

4. The spray nozzle openings shall be sized to prevent plugging and shall be located as near to the ground surface as practical to minimize wind drift of the wastewater.

5. The spray nozzles shall be arranged so that the wastewater will be evenly distributed over the entire area under irrigation.

6. The spray irrigation system shall be arranged so that individual sections within the system can be taken out of service for resting without interrupting discharge to the remaining sections.

7. The spray irrigation system shall be seeded with perennial grasses such as reed canary grass, tall fescue and orchard grass. The cover crop shall be a crop which is not used for direct human consumption. New seedings shall also contain a nurse crop. The cover crop shall be maintained by cutting and removing the grass a minimum of twice per growing season. The department may

approve the use of other types of cover crops such as corn but will restrict the use of such sites to times when the cover crop is actively growing. The department may also require reduced hydraulic application rates, grass buffer strips or both around the perimeter of the site to prevent wastewater runoff during rainfall events.

8. The ground surface of the spray irrigation system shall have a minimum separation distance to bedrock of at least 5 feet.

9. A minimum separation distance of 5 feet shall be maintained between the land surface elevation of the spray irrigation field and the highest anticipated groundwater elevation. The department may on a case-by-case basis allow this distance to be reduced if the permittee can show, based on hydrogeologic and other relevant site factors, that the groundwater will be adequately protected.

10. The department may require disinfection of effluent to spray irrigation systems if there is a potential risk to public health.

(b) *Discharge limitations for spray irrigation systems.* Effluent limitations are as specified in s. NR 206.08 (2) (b).

**(3) RIDGE AND FURROW SYSTEMS.** (a) *Design and construction criteria for ridge and furrow systems.* 1. The shape of each cell within the ridge and furrow system shall be such that a minimum of soil disturbance is necessary to form the system.

2. A ridge and furrow system may not be constructed on a site at which less than 50% of the soil particles pass a no. 200 sieve. Coarser textured soils may be approved on a case-by-case basis depending on system design and wastewater strength. Suitable soils shall extend at least 3 feet below the base grade of the furrow bottoms.

3. The furrows of the ridge and furrow system shall be one foot deep and one foot wide at the furrow bottom.

4. Furrow side slopes may not be steeper than one horizontal to 2 vertical.

5. The system shall be sized and constructed in order to allow sufficient resting to allow soil conditions to become unsaturated and aerobic prior to any wastewater being reapplied.

6. The system shall be constructed in a manner which provides equal liquid distribution during loading of each cell. The header ditch shall be sealed in order to allow complete drainage and to prevent wastewater seepage. The drainage of the header ditch and the grading of the furrows for equal liquid distribution shall be tested with water before seeding the ridges with grasses.

7. The system shall consist of at least 2 cells which can be alternately loaded and rested.

8. The wastewater distribution system shall be arranged so that individual cells within the ridge and furrow system can be taken out of service for resting without interrupting discharge to the remaining cells.

9. The bottom of the ridge and furrow system may not be closer than 5 feet to the highest anticipated groundwater elevation. The department may allow this distance to be reduced on a case-by-case basis if the permittee can show, based on hydrogeologic and other relevant site factors, that the groundwater will be adequately protected.

10. The bottom of the system shall be at least 5 feet from bedrock.

11. Outside embankments and dikes may not be steeper than 3 horizontal to one vertical. Inside embankments and dikes may not be steeper than 2 horizontal to one vertical. All embankments and dikes shall be properly seeded with perennial grasses to prevent erosion. A nurse crop of annual grasses shall be used to establish a ground cover.

12. All ridge tops shall be a minimum of 6 feet wide to allow mechanical removal of grasses. The grasses shall be cut, and if possible removed, at least once during the growing season and shall be burned or cut and removed each spring.

13. All areas within a ridge and furrow system shall be accessible for maintenance equipment.

14. The system shall be constructed to prevent surface runoff from entering the system.

15. The ridges shall be seeded with perennial grasses which are suited to wet soil conditions. A nurse crop shall be used to seed new or modified systems. In addition, the grass cover shall be established to at least a 2-inch length before the system is used for wastewater treatment.

16. Discharge to ridge and furrow systems which have frozen soils is prohibited.

(b) *Discharge limitations for ridge and furrow systems.* Effluent limitations are as specified in s. NR 206.08 (3) (b).

**(4) OVERLAND FLOW SYSTEMS.** (a) *Design and construction criteria for overland flow systems.* 1. Overland flow systems shall be underlain by at least one foot of heavy textured soils such as clays or clay loams to retard leakage through the base.

2. The downslope flow distance shall be 100 feet or greater.

3. The downslope gradient for the overland flow fields shall be between 2% and 8%.

4. The system shall consist of at least 2 cells of approximately equal area which can be alternately loaded and rested. Where self-propelled equipment which operates on a continuous basis is installed and division into identifiable cells is impossible, its movement shall be regulated to provide alternate loading and resting of the soil.

**Note:** It is recommended that an overland flow cell be rested for at least one third of the total time in the rest/load cycle.

5. The system shall be arranged so that individual cells within the system can be taken out of service for resting without interrupting discharge to the remaining cells.

6. The distribution equipment shall be arranged so that the wastewater will be evenly distributed over the entire area of an overland flow cell and the wastewater travels down the slope in a non-channelized flow.

7. The wastewater distribution equipment shall be located at or near the ground level.

**Note:** A pressurized distribution system is recommended for ease of operation.

8. The wastewater distribution system shall be designed to allow for cleaning of the distribution orifices. Flexibility to adjust the flow from individual orifices shall be provided to allow the system to be hydraulically balanced to minimize flow channeling.

9. An impermeable channel shall be provided for collecting runoff from the overland flow fields. The collection system shall be capable of removing the effluent and the rain from a 10-year frequency, 24-hour duration rainfall event with only temporary backing up of water onto the fields.

10. The overland flow fields shall be vegetated with a water tolerant mixed perennial grass cover crop such as reed canary grass, tall fescue and orchard grass. A nurse crop shall be used to establish the perennial grass cover. The grass cover shall be maintained by frequent resting. The crop shall be cut and the cuttings removed at least twice per year to stimulate grass growth and enhance nutrient removal from the system.

11. Winter operation may be allowed as long as the soil surface remains unfrozen. The department may require storage or additional treatment of the runoff during cold weather.



12. An overland flow field may be used when the land surface elevation is at least 5 feet above bedrock.

13. The land surface elevation of an overland flow field shall be no closer than 5 feet to the seasonally high groundwater level. The department may on a case-by-case basis allow this distance to be reduced if the permittee can show, based on hydrogeologic and other relevant site factors, that the groundwater will be adequately protected. If such a variance is approved or if the risk of groundwater contamination is otherwise high, the department may require additional pretreatment of the wastewaters.

14. The hydraulic application rate, expressed as a flow rate per unit width of slope, shall be between .16 gpm/ft and .60 gpm/ft.

(b) *Discharge limitations for overland flow systems.* Effluent limitations are as specified in s. NR 206.08 (4) (b) and applicable surface water limitations.

**History:** Cr. Register, November, 1990, No. 419, eff. 12-1-90.

### NR 110.26 Sludge handling, storage and disposal.

(1) **DESIGN REPORT.** A design report shall be submitted in accordance with s. NR 110.15 (1). The report shall show calculations used to design the sludge facilities. Design of sludge handling facilities shall consider such factors as the volume of sludge generated, its percent solids and character, the degree of volatile solids reduction, sludge temperature, the degree or extent of mixing to be obtained, the sludge percent solids and characteristics after processing and the size of the installation with appropriate allowances for sludge and supernatant storage and energy requirements whenever such factors are appropriate for the design of the sludge processing facilities.

(2) **GENERAL DESIGN CONSIDERATIONS.** (a) *Grit removal.* When grit removal facilities are not provided, the volume of thickeners, digesters and storage facilities shall be increased to accommodate the additional solids loading.

(b) *Sludge thickening.* 1. Whenever practical, sludge thickening shall be provided.

2. Thickened sludge should have a minimum solids concentration of 5% prior to transmission to the digesters.

(c) *Multiple units.* Multiple units shall be provided. A single unit may be allowed, provided an alternate method of sludge processing, emergency storage or ultimate disposal operation exists to insure continuity of service.

(d) *Maintenance.* 1. Provisions shall be made for draining, cleaning, inspection, and maintenance of all units.

2. Tank bottoms shall be sloped to drain to a sump pump or withdrawal pipe.

3. Access manholes shall be provided. Covered tanks shall have one side wall entrance large enough to permit the use of mechanical equipment to remove grit and sand.

(e) *Storage facilities.* 1. The construction of sludge storage facilities shall be required to improve sludge handling capabilities, provide flexibility in operations, and to avoid environmental or public health hazards due to improper disposal techniques.

2. Construction of these facilities will depend upon treatment plant capabilities, land availability, surface and groundwater protection, health factors, municipal sludge management capabilities and other environmental factors.

(3) **GENERAL DESIGN REQUIREMENTS.** (a) *Flow measurement.* Devices for measuring flow to and from sludge digestion facilities shall be provided.

(b) *Ventilation.* All enclosures which are connected with sludge digesters, or which contain sludge or gas piping or equipment shall be provided with forced ventilation in accordance with s. NR 110.14 (3) (b). The piping gallery for digesters may not be

connected to other passages unless a tightly fitting self-closing door is provided at connecting passageways.

(c) *Safety.* Nonsparking tools, rubber soled shoes, safety harness, gas detectors for inflammable and toxic gases and at least 2 self-contained respiratory units should be provided for emergency use whenever inflammable and toxic gases may be present.

(d) *Supernatant withdrawal.* Sludge thickeners and aerobic digestors shall be designed for effective separation and withdrawal of supernatant and for effective collection and removal of scum and grease.

(e) *Sampling.* 1. Provisions shall be made for sampling at each supernatant draw-off level and for collecting sludge samples for analysis. Sampling pipes shall be at least 4 centimeters (1 1/2 inches) in diameter and shall terminate in a suitably-sized sink or basin.

2. Unless sampling facilities are otherwise provided, quick-closing sampling valves shall be installed at the sludge pumps. The size of valve and piping shall be at least 4 centimeters (1 1/2 inches).

(f) *Chemical handling.* Chemical handling facilities shall meet the provisions of s. NR 110.22.

(4) **SLUDGE PUMPS AND PIPING.** (a) *Sludge pumps.* 1. Sludge pumping systems shall be designed to handle the expected range of sludge flows.

2. Multiple pumps shall be provided.

3. Pumps with demonstrated solids handling capability shall be provided for handling raw and processed sludge.

4. A minimum positive head of 60 centimeters (2 feet) shall be provided at the suction side of centrifugal-type pumps and is desirable for all types of sludge pumps. Maximum suction lifts may not exceed 3.0 meters (10 feet) for plunger pumps.

(b) *Sludge piping.* 1. Sludge withdrawal piping shall have a minimum diameter of 20 centimeters (8 inches) for gravity withdrawal and 15 centimeters (6 inches) for pump suction and discharge lines. The department may approve the use of glass lined pipe which is at least 10 centimeters (4 inches) in diameter. Where withdrawal is by gravity, the available head on the discharge pipe shall be sufficient to maintain a minimum velocity of 90 centimeters (3 feet) per second.

2. Gravity piping shall be laid on uniform grade and alignment. The slope on gravity discharge piping may not be less than 3%.

3. Provisions shall be made for draining, flushing and cleaning sludge piping.

4. Air relief valves shall be provided at high points in pressure sludge lines.

5. Special consideration shall be given to the corrosion resistance and continuing stability of pipes and supports located inside the digestion tank.

(5) **ANAEROBIC DIGESTION.** (a) *General.* 1. Anaerobic digesters which will also serve as supernatant separation tanks shall have a minimum side water depth of 6 meters (20 feet).

2. Multiple sludge inlets and draw-offs shall be provided. Multiple recirculation suction and discharge points to facilitate flexible operations and effective mixing of the digester contents shall be provided unless adequate mixing facilities are provided within the digester. One sludge inlet shall discharge above the liquid level and be located at approximately the center of the tank to assist in scum breakup. Raw sludge inlet discharge points shall be so located as to minimize short circuiting to the supernatant draw-off. Sludge withdrawal for disposal shall be from the bottom of the tank. The pipe shall be interconnected with the recirculation piping to increase flexibility in mixing tank contents.

(b) *Tank capacity.* 1. The total digestion tank capacity shall be calculated based upon the factors indicated in sub. (1). If such calculations are not done, the following minimum requirements shall be met:

a. A minimum detention time of 15 days at design flows shall be provided;

b. Completely mixed digestion systems shall provide for intimate and effective mixing to prevent stratification and to assure homogeneity of digester content. The maximum system loading shall be 1.28 kilograms per cubic meter per day (80 pounds of volatile solids per 1,000 cubic feet of volume per day) in the digester;

c. The maximum system loading for moderately mixed digestion systems in which mixing is accomplished only by circulating sludge through an external heat exchanger shall be 0.64 kilograms per cubic meter per day (40 pounds of volatile solids per 1,000 cubic feet of volume per day) in the digester; and

d. The loading rates indicated in subd. 1. a., b., and c. assume that the raw sludge is derived from ordinary domestic wastewater. The loading may be modified upward or downward depending upon the degree of mixing provided. Where mixing is accomplished by other methods, loading rates may be approved on the basis of information submitted justifying the design.

(c) *Temperature.* Heating equipment shall have the ability to maintain digestion temperature in the range of 33° to 38°C (90° to 100°F).

(d) *Gas collection, piping and appurtenances.* 1. All portions of the gas system, including the space above the tank liquor, storage facilities and piping shall be so designed that under normal operating conditions, including sludge withdrawal, the gas will be maintained under positive pressure. All enclosed areas where any gas leakage might occur shall be ventilated.

2. Safety facilities shall be included where gas is produced. Pressure and vacuum relief valves and flame traps, together with automatic safety shut off valves, shall be provided. Waterseal equipment may not be installed. Gas compressors with gas safety equipment should be housed in a separate room with an exterior entrance.

3. The diameter of gas piping shall be based on the volume of gas which will be generated. Gas piping shall slope to condensate traps. The use of float-controlled condensate traps is prohibited.

4. Gas burning boilers, engines and other units using gas as a fuel shall be located in ventilated rooms, preferably at ground level and shall be isolated in accordance with the provisions of chs. SPS 361 to 365. Gas lines to these units shall be provided with suitable flame traps.

5. Electrical fixtures and controls in enclosed places where gas may accumulate shall comply with the national electrical code requirements for class 1, group D, division 2 locations.

6. Waste gas burners shall be readily accessible and shall be located at least 7.6 meters (25 feet) away from any plant structure if placed at ground level. Waste gas burners may be located on the roof of the control building if sufficiently removed from the tank. All waste gas burners shall be equipped with automatic ignition, such as a pilot light or a device using a photoelectric cell sensor. Consideration should be given to the use of natural or propane gas to insure reliability of the pilot light. If the waste gas burner is in a remote location, the department may approve the discharge of gas to the atmosphere through a return-bend screened vent terminating at least 3 meters (10 feet) above the walking surface, provided the assembly incorporates a flame trap.

7. A gas meter with by-pass shall be provided to meter total gas production. Additional gas meters may be required to measure gas usage.

(e) *Digestion tank heating.* 1. Whenever possible, digestion tanks shall be constructed above groundwater level. Digestion tanks shall be insulated to minimize heat loss.

2. Piping shall be designed to provide for the heating of feed sludge before introduction to the digesters. Heat exchanger sludge piping shall be sized for heat transfer requirements.

3. Sufficient heating capacity shall be provided to consistently maintain the design sludge temperature. Where digestion tank gas is used for other purposes, an auxiliary fuel supply shall be provided.

4. Consideration should be given to equipping hot water internal heating controls with an automatic mixing valve to temper the boiler water with return water so that the inlet water to the heat jacket can be maintained between 49° to 55°C (120° to 130°F) to prevent excessive caking or encrustation of sludge on the heat jacket. Manual controls shall also be provided.

5. The boiler shall be provided with automatic controls to maintain the boiler temperature at approximately 82°C (180°F). Automatic controls shall also be provided to shut off the main gas supply in the event of pilot burner or electrical failure, low boiler water level or excessive temperature.

6. Thermometers shall be provided to show temperatures of the sludge, hot water feed, hot water return and boiler water.

(f) *Supernatant withdrawal.* 1. Supernatant piping shall be 15 centimeters (6 inches) in diameter, or larger.

2. Piping shall be arranged so that withdrawal can be made from 3 or more levels in the tank. A positive unvalved vented overflow shall be provided.

3. If a supernatant selector is provided, provisions shall be made for at least one other draw-off level located in the supernatant zone of the tank in addition to a positive unvalved vented overflow pipe. High pressure backwash facilities shall be provided.

**(6) AEROBIC SLUDGE DIGESTION.** (a) *General.* 1. Aerobic digesters shall be designed to provide effective air mixing, reduction of organic matter, supernatant separation and sludge concentration.

2. The digester detention time may be calculated based upon the factors indicated in subd. 1. If such calculations are not done, the following minimum requirements shall be met:

a. A minimum of 15 days detention time shall be provided for waste activated sludge and 20 days for primary sludge or a combination of primary and waste activated sludge. Where sludge temperature is lower than 10°C (50°F), additional time shall be provided; and

b. The volatile suspended solids loading may not exceed 1.60 kilograms per cubic meter per day (100 pounds per 1,000 cubic feet per day) in the digestion unit. Lower loading rates may be necessary depending on temperature, type of sludge and other factors.

3. The aeration system for aerobic digesters shall be capable of meeting the oxygen requirements of par. (b), or the mixing requirements of par. (c), whichever is greater.

(b) *Oxygen demand.* 1. Aeration systems shall be capable of maintaining a minimum digester dissolved oxygen concentration of one milligram per liter.

2. In the absence of experimentally determined values, the design oxygen requirements for aerobic digesters shall be 2.0 kilograms oxygen per kilogram (2.0 pounds oxygen per pound) anticipated volatile suspended solids reduction. An additional

1.8 kilograms oxygen per kilogram (1.8 pounds oxygen per pound) of BOD<sub>5</sub> applied to the digester by primary sludge shall be supplied.

3. The design of the aeration system to meet the digester oxygen requirements shall comply with the provisions of s. NR 110.21 (5) (c).

(c) *Mixing requirements.* 1. Digestion tanks shall be designed for effective mixing.

2. Diffused aeration systems shall be capable of delivering a minimum air flow rate of 30 cubic meters per minute per 1,000 cubic meters (30 cubic feet per minute per 1,000 cubic feet) of digester volume.

3. Mechanical aerators shall deliver 26.3 kilowatts per 1000 cubic meters (1.0 horsepower per 1,000 cubic feet) of digester volume.

(d) *Aeration equipment.* Aeration equipment needed to meet the requirements of pars. (b) and (c) shall comply with the provisions of s. NR 110.21 (6).

(e) *Supernatant withdrawal.* Aerobic digesters shall be equipped for supernatant draw-off. It is recommended that multi-level draw-off be provided.

**(7) OTHER SLUDGE STABILIZATION PROCESSES.** (a) *Lime stabilization.* Sufficient lime shall be added to produce a pH of 12 after 2 hours of contact.

(b) *Composting.* Static aerated pile, within vessel, or windrow composting methods shall be maintained at a minimum operating temperature of 40°C (104°F) for at least 5 days. For 4 hours during this period the temperature must exceed 55°C (131°F). Composting design, siting and operation shall be done in accordance with chs. NR 204 and 500 to 538.

(c) *Other methods.* Other methods or operating conditions may be acceptable for sludge stabilization if pathogens and volatile solids are reduced to an extent equivalent to anaerobic digestion.

**(8) SLUDGE DEWATERING.** (a) *Sludge drying beds.* 1. The drying bed area shall be calculated based upon such factors as climatic conditions, character and volume of sludge to be dewatered, the method and character of sludge removal and other methods of sludge disposal. At least 0.19 square meters (2 square feet) of drying bed area per capita population equivalent shall be provided when the drying bed is the primary method of dewatering, and 0.09 square meters (1 square foot) per capita population equivalent if it is to be used as a back-up dewatering unit. The bed area shall be increased by 25% if the beds are paved.

2. At least 2 drying beds shall be provided.

3. Percolation type drying beds shall meet the following conditions:

a. The lower course of gravel around the underdrains shall be properly graded and shall be at least 30 centimeters (one foot) in depth, extending at least 15 centimeters (6 inches) above the top of the underdrains. It is desirable to place this in 2 or more layers. At least 8 centimeters (3 inches) of the top layer shall consist of gravel 3 to 6 millimeters ( $\frac{1}{8}$  to  $\frac{1}{4}$  inches) in size.

b. The top course shall consist of at least 15 to 23 centimeters (6 to 9 inches) of clean, medium to coarse, sand with a grain size of 1 to 3 millimeters in diameter. The finished sand surface shall be level.

c. Underdrains shall be clay pipe or concrete drain tile at least 10 centimeters (4 inches) in diameter laid with open joints. Underdrains shall be spaced not more than 6 meters (20 feet) apart.

d. An impervious layer shall be provided immediately beneath the lower course to prevent the downward movement of filtrate into the groundwater.

4. Paved surface drying beds may be used if adequate center or side drains are provided. If partially paved drying beds are used, they shall be designed with consideration for space requirement to operate mechanical equipment for removing the dried sludge.

5. Sludge influent piping to the beds shall terminate at least 30 centimeters (one foot) above the surface and be so arranged that the beds will drain. Concrete splash plates for percolation type beds shall be provided at sludge discharge points.

6. Walls shall be watertight and extend 38 to 46 centimeters (15 to 18 inches) above and at least 15 centimeters (6 inches) below the surface of the beds. Outer walls shall be curbed to prevent soil from washing onto the beds.

7. Drying beds shall be arranged to facilitate sludge removal. Concrete truck tracks shall be provided for all sludge beds. Pairs of tracks shall be on 6 meter (20-foot) centers.

(b) *Sludge drying lagoons.* 1. The bottom of the lagoons must be at least 1.25 meters (4 feet) above the maximum seasonal high groundwater level and at least 3 meters (10 feet) above bedrock. The bottom of the lagoons shall be constructed in accordance with s. NR 110.24.

2. Lagoons may not be more than one meter (39 inches) in depth.

3. The area required will depend on design conditions. At least 2 lagoons shall be provided.

4. Lagoons shall be adequately isolated to avoid creating nuisances.

(c) *Mechanical dewatering facilities.* 1. 'General.' Provision shall be made to maintain sufficient continuity of service so that sludge may be dewatered without accumulation beyond storage capacity. Design calculations or pilot plant data shall be submitted to justify the basis of design and equipment.

2. The capacity of vacuum filters, centrifuges, filter presses, belt filters or other mechanical dewatering facilities shall be sufficient to dewater the sludge produced with the largest unit out of service.

3. There shall be provided at least one back-up vacuum pump and one back-up filtrate pump for each vacuum filter installation. The vacuum filter shall be designed to allow for the easy removal and replacement of the vacuum pump and filtrate pump.

4. The dewatering area shall be ventilated.

(d) *Drainage and filtrate disposal.* The drainage from drying beds or shallow lagoons and the filtrate from dewatering units shall be returned to the sewage treatment process at appropriate points.

(e) *Other dewatering facilities.* If other methods of reducing the quantity of sludge are proposed, a detailed description of the process and design data shall accompany the plans.

**(9) SLUDGE REDUCTION.** (a) *Incineration.* Adequate provisions for residue disposal and air pollution control shall be provided. The appropriate requirements of chs. NR 415, 204 and 500 to 538 shall be met.

(b) *Other reduction facilities.* If other methods of reducing the quantity of sludge are proposed, a detailed description of the process and design data shall accompany the plans.

**(10) SLUDGE STORAGE FACILITIES.** (a) *General.* Sludge storage shall be provided by facilities in accordance with ch. NR 204. A detailed description of the wastewater treatment process and design data shall accompany the plans for the proposed storage facility. Sludge storage facilities shall be designated and operated



to maintain compliance with the groundwater quality standards in ch. NR 140. In the event a sludge storage facility is temporary, it shall be abandoned in such a manner so as to prevent safety, environmental and aesthetic problems from occurring. The department shall be notified in writing if the storage facility is to be abandoned and how abandonment will be accomplished.

(b) *Separation distances.* 1. Sludge storage facilities may not be located within the following distances of a water supply well.

- a. 1000 feet from a community public water supply well;
- b. 250 feet from a private water supply well;

2. Separation distances from residential and commercial buildings shall be maintained as required in s. NR 110.15 (3) (d).

3. A minimum separation distance of 1.25 meters (4 feet) shall be maintained between the bottom of storage lagoons and the highest recorded or indicated seasonal groundwater table elevation.

4. A minimum separation distance of 3 meters (10 feet) shall be maintained between the bottom of storage lagoons and bedrock.

(c) *Lagoon sludge storage facilities.* 1. Lagoon sludge storage facilities shall be designed to facilitate easy addition and removal of sludge without causing damage to the facility.

2. Lagoon sludge storage facilities shall be designed and constructed in accordance with s. NR 110.24.

3. The maximum lagoon depth shall be 6 meters (20 feet). The depth may be increased by the department on a case-by-case basis.

(d) *Liquid sludge storage facilities.* 1. Storage tanks shall be designed, installed and maintained to prevent leaks due to corrosion or structural failure.

2. Liquid sludge storage facilities shall be designed with mixing capabilities to completely mix the tank contents.

3. Supernatant withdrawal capability is recommended if the solids content of the sludge is less than 5%.

4. Sampling ports shall be provided on both the influent and effluent lines to the tank. It is recommended that sample ports be equipped with drainage capabilities or other provisions to allow for a means to sample the tank effluent without actual discharge.

(e) *Cake storage facilities.* 1. Permanent and temporary cake storage facilities shall be designed to minimize odors and to protect surface waters, groundwaters and soil.

2. Surface runoff shall be diverted away from the storage location.

3. Cake storage facilities designed as pits shall provide a method of draining-off and collecting precipitation.

4. All sludge stored at an intermediate term cake storage facility shall be covered with a temporary cover. At a minimum, the cover shall be a 4 mil polyethylene cover or equivalent. The cover shall be anchored or otherwise secured.

5. All sludge stored at a long term cake storage facility shall be covered with a permanent cover.

6. Leachate collection systems shall be provided.

(f) *Monitoring wells.* Monitoring wells may be required on a case-by-case basis. Construction of monitoring wells shall comply with the requirements of s. NR 110.25 (5) and ch. NR 141.

(g) *Amount of storage.* Appropriate sludge storage and length of storage shall be provided in accordance with ch. NR 204 and this chapter. The required storage capacity may not include storage in clarifiers, oxidation ditches or aeration basins. However, digestive capacity that is greater than the digestive requirements can be considered as storage and can be used in calculating the amount of available storage.

(11) TRANSPORTATION OF SLUDGES. (a) *Liquid.* 1. Liquid sludge shall be transported in an enclosed watertight unit from treatment plant to disposal site.

2. All sludge field spreading equipment shall be provided with a control so that the discharge valve can be opened and closed by the driver while the vehicle is in motion.

(b) *Semi-solid cake.* Sludge cake shall be transported in a covered watertight unit to prevent leakage of sludge moisture released in transit. Provision shall be taken to prevent the spilling of sludge from the vehicle while in transit and to prevent an odor nuisance while in transit.

(12) ULTIMATE DISPOSAL. The final use or disposal of sludge shall be regulated through the WPDES permit, this chapter and ch. NR 204 which comply with all applicable provisions of the clean water act (CWA) section 405 and 40 CFR 503.

**History:** Cr. Register, November, 1974, No. 227, eff. 12-1-74; r. and recr. Register, February, 1983, No. 326, eff. 3-1-83, am. (9), (12) (a) 3. and (c), r. and recr. (10) and cr. (12) (d), Register, November, 1990, No. 419, eff. 12-1-90; correction in (7) (b) made under s. 13.93 (2m) (b) 7., Stats., Register, September, 1995, No. 477; am. (2) (e) 1., (7) (b), (9) (a), (10) (a), (c) 1., 2. and (11) (a) 2., r. (10) (c) 4., 5., (f) 2. and 3., renum. (10) (d) to (f) 1. to be (10) (e) to (g) and am. (e) 6., (g) (intro.) and 1., cr. (10) (d), r. and recr. (12), Register, December, 1995, No. 480, eff. 1-1-96; correction in (5) (d) 4. was made under s. 13.93 (2m) (b) 7., Stats., Register, May, 2001, No. 545; corrections in (5) (d) 4., (7) (b) and (9) (a) made under s. 13.92 (4) (b) 7., Stats., Register February 2010 No. 650; corrections in (5) (b) 1. d. and (8) (b) 1. made under s. 13.92 (4) (b) 1. and 7., Stats., Register July 2010 No. 655; correction in (5) (d) 4. made under s. 13.92 (4) (b) 7., Stats., Register January 2012 No. 673.

### NR 110.27 Requirements for certified or registered laboratory.

Bacteriological analyses of groundwater samples, and all radiological analyses, shall be performed by the state laboratory of hygiene or a laboratory certified or approved by the department of health services. Other laboratory test results for those pollutants which are required by the WPDES permit to be monitored and which are submitted to the department in support of facility plans or plans and specifications under this chapter shall be performed by a laboratory certified or registered under ch. NR 149. The department may require, on a case-by-case basis, that certain other laboratory test results submitted to the department be performed by a certified or registered laboratory. The following tests are excluded from this requirement:

- (1) Temperature,
- (2) Turbidity,
- (3) Bacteria tests in wastewater effluent,
- (4) pH,
- (5) Chlorine residual,
- (6) Specific conductance,
- (7) Physical properties of soils and sludges,
- (8) Nutrient tests of soils and sludges,
- (9) Flow measurements.

**Note:** The requirement in this section to submit data from a certified or registered laboratory is effective on August 28, 1986.

**History:** Cr. Register, April, 1986, No. 364, eff. 8-28-86; correction in (intro.) made under s. 13.92 (4) (b) 6., Stats., Register April 2023 No. 808.