UDC Appendix

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Wisconsin Division of Safety and Buildings			WISCO	ONSIN ERMIT	UNIFORM	BUILD	ING		Applic	ation N	0.		T(
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Building Address		Su	bdivision	Name				Lot No		Blo	ock No.		
Zoning District(s)	Zoning Pe	rmit No	<u> </u>		Setbacks:	Front	ft.	Rear	ft.	eft	ft.	Right	ft.
1. PROJECT	3. OCCUP	ANCY	6. ELEC	TRICAL	9. HVACEQ	UIPMENT	AZ EN	ERGY SO	URCE		Flee	I Salid	L Sala
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Living	5. STORIE	s	∕on].	i i i	D Municipal		Envelop	pe				BTU	/HR
AreaSq Ft	1-Story		8-0-50		Septic Perm	it No.:	Infiltrat	ion				ודרפ	
Garage Sq Ft	□ 2-Story □ Other:	$\langle \mathcal{Y} \rangle$	□ Perma	ment	11. WATER	and a second	14. EST	r. BUILD	ING COST	Г	:		
Deck Sa Ft	D Plus Base	ement	□ Other:	:	Municipal U Private On-	Itility Site Well	s			-			
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FEES:		PE	RMIT(S)	ISSUED	WIS PERMIT	SEAL #	PERMI	r issued	BY:				
Plan Review \$			Construct	tion			Name						
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Other \$		— D	Plumbing	g			Date		Tel				-(
Total \$			101011			l	Cert No	·			·		
2D_5823(R 05/98) WH	ITE - Issuin	9 Jurisdiction	YE	LLOW - St	tate w/in 30 dav	s if new dw	elling	GREEN	I - Inspect	tor	PINK	- Owner	/Agent

INSTRUCTIONS

The owner, builder or agents shall complete the application form down through the Signature of Applicant block and submit it and building plans and specifications to the enforcing municipality. Permit application data is used for statewide statistical gathering on new one- and two-family dwellings, as well as for local code administration.

PERMIT REQUESTED

- Check off type of Permit Requested, such as structural, HVAC, Electrical or Plumbing.
- Fill in owner's current Mailing Address and Telephone Number.
- PROJECT LOCATION

Fill in Building Address (number and street or sufficient information so that the building inspector can locate the construction site.

• Fill in Contractor Information. Note, per s. 101.63 (7) Wis. Stats., that the master plumber name and number must be entered before issuing a plumbing permit.

• Local zoning, land use and flood plain requirements must be satisfied before a building permit can be issued. County approval may be necessary.

Fill in Zoning District, lot area and required building setbacks.

PROJECT DATA - Fill in all numbered project data blocks (1-14) with the required information. All data blocks must be filled in, including the following:

2. Area (involved in project):

- Basements include unfinished area only
- Living area include any finished area including finished areas in basements
- Two-family dwellings include total combined areas
- 3. Occupancy Check only "Single-Family" or "Two-Family" if that is what is being worked on. In other words, do not check either of these two blocks if only a new detached garage is being built, even if it serves a one or two family dwelling. Instead, check "Garage" and number of stalls. If the project is a community based residential facility serving 3 to 8 residents, it is considered a single-family dwelling.
- 9. HVAC Equipment Check only the major source of heat, plus central air conditioning if present. Only check "Radiant Baseboard or Panel" if there is no central source of heat.
- 10. Plumbing A building permit cannot be issued until a county sanitary permit has been issued for any new of affected existing on-site sewage system.
- 14. Estimated Cost Include the total cost of construction, including materials and market rate labor, but not the cost of land or landscaping

SIGNATURE - Sign and date this application form.

CONDITIONS OF APPROVAL - The authority having jurisdiction uses this section to state any conditions that must be complied with pursuant to issuing the building permit.

ISSUING JURISDICTION: This must be completed by the authority having jurisdiction.

- Check off Municipality Status, such as town, village or city.
- Fill in Municipality Name and Municipality Number of inspection authority.
- Fill in Municipality Number of Dwelling Location if different from municipality where inspection authority is located (applies to county or state enforcement)
- Check off type of Permit Issued, such as construction, HVAC, electrical or plumbing.
- Fill in Wisconsin Uniform Permit Seal Number, if project is a new one- or two-family dwelling.

Fill in Name and Inspector Certification Number of person reviewing building plans and date building permit issued.

PLEASE RETURN YELLOW COPY WITHIN 30 DAYS AFTER ISSUANCE TO (You may fold along the dashed lines and insert this form into a window envelope.):

Safety & Buildings Division P O Box 2509 Madison, WI 53701-2509

CAUTIONARY STATEMENT TO OWNERS OBTAINING BUILDING PERMITS

101.65 (1r) of the Wisconsin Statutes requires municipalities that enforce the Uniform Dwelling Code to provide an owner who applies for a building permit with a statement advising the owner that:

If the owner hires a contractor to perform work under the building permit and the contractor is not bonded or insured as required under s. 101.654 (2) (a), the following consequences might occur:

(a) The owner may be held liable for any bodily inquiry to or death of others or for any damage to the property of others that arises out of the work performed under the building permit or that is caused by any negligence of the contractor that occurs in connection with the work performed under the building permit.

(b) The owner may not be able to collect from the contractor damages for any loss sustained by the owner because of a violation by the contractor of the one- and 2- family dwelling code or an ordinance enacted under sub. (1) (a), because of any bodily injury to or death of others or damage to the property of others that arises out of the work performed under the building permit or because of any bodily injury to or death of others in connection with the work performed under the building permit.

SBD-5823 (R.05/98)

				Work shall not	proceed until the insr	pector has approved the	e various stages of	construction or the 48
	SITE IN	FO	1	business hr. per	riod since notification	has elapsed. This per	mit will expire	24 months after the da
SUBDIVISION	ti. San na sana sana sana sana sana sana san			of issuance if the	he building's exterior	has not been complete	d. Keep this card	posted until final
LOT NO	BLOCK	NO		Inspection has	Deen made. (WI Stat	(s. 101.63)	r	
ZONING DISTRICT				WISC	UNSIN U	NIFORM		
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	PLBG.		#		NOTICE OF NONCO	OMPLIANCE: This issuit	ng jurisdiction shall n	otify the applicant in writing
· · · · · ·			#	<u></u>	ot any violations to be notification, unless ext	corrected. All cited violati ension-time is granted.	ions shall be corrected	I within 30 days of

SBD-5824 (R. 05/96)

Comm 20 APPENDIX

74

State of Wisconsin

Safety and Buildings Division

Submit to non-enforcing municipalities for new 1and 2- family dwellings

WISCONSIN ADMINISTRATIVE BUILDING PERMIT APPLICATION (Wis. Stats. 101.63 (7) & 101.65 (3))

(Wis. Stats. 101.63 (7) & 101.65 (3)) EE INSTRUCTIONS ON BACK OF YELLOW COPY.

Personal information you provide may be used for secondary purposes. [Privacy Law 15.04(1)(m)]

PERMIT APPLICANT							
Last Name	First	Name			Middle Initi	al	
					2. 2019 - 1	· · · · · · · · · · · · · · · · · · ·	
Street Address			1949 - A.				2 2
City		State	Zip Coc	le	Telephone No. (I	nclude area o	code)
				and the second			
PROTECTION		A CARLES AND A CARLES	100 100				
Building Address	A CONTRACTOR OF A CONTRACTOR O		Subdivision	Name		Lot #	Block #
					14		
Legal Description		1 ,			Pareel No.	······································	
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1. PROJECT TYPE	2. HVACEQU	HENDEN					
$\square 1 Family \qquad \square For \square 2 Family \qquad \square Bo$	rced Air Furnace	X]]	adjant Base	board or F	ane!	☐ Heat	Pump r:
		t Cas	I P	Oil	Elect	Solid	Solar
Space Heating							
Water Heating							
4. CONSTRUCTION TYPE		5. FO	UNDATIC	N			
Site Constructed			ncrete		Masonry	🗋 Treat	ed Wood
O. A.K.BA					<u>MAGAGON PRA</u>		ISSUE AND
Living area =	Square Feet	\$					
I youch that all the above infor	mation is correct a	nd underst	and that the is	suance of	this permit is for a	iministrative p	ourposes only. I
understand that onsite construct	tion inspections wi	ll not be p	erformed by t	he municip	ality, but that the U	Iniform Dwell	ling Code,
Chapters Comm/ILHR 20-25, s issuance of this permit does not	still applies to all ne t relieve me of com	ew 1- and 2 pliance wi	2-family dwel ith other appli	cable code	nust be complied w s and ordinances.	vith. I underst	and that the
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Applicant's Signature				Date S	igned		and the second sec
Applicant's Signature		FORWAR	DINC BINK BL	V TO THE S	TATE DRUSION OF	SAFETY AND	PULLDINCS
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of Dwelling Location	K: #	······		<u> </u>		LES:	
PERMIT ISSUED RV-						ATE	
					Ī	SSUED:	
SBD-8254 (R 01/98) White - Is	suing Jurisdiction	Pink -	State Within	n 30 Dave	Yellow -	Applicant	

INSTRUCTIONS

The owner, builder or agent shall complete and provide all required information on the application form down through the Signature of Applicant block. This data is used for statewide statistical gathering on new one- and two-family dwellings, as well as for local administration. When completed, submit to local municipality having jurisdiction. Plan review or building inspections will not be performed by the municipality.

PERMIT REQUESTED:

• Fill in building address.

• Fill in legal description of lot, subdivision name, lot number and block number.

PROJECT DATA:

- Fill in **all numbered** project data blocks (1–7) with the required information. All data blocks must be filled in, including the following:
 - 1. **Type** Check only "1–Family" or "2–Family" if that is what is being built. In other words, do NOT use this form if only a new detached garage is being built, even if it serves a one or two family dwelling.
 - 2. **HVAC Equipment** Check only the major source of heat, not any supplemental sources. Mark central air conditioning if present. Only check "Radiant Baseboard or Panel" if there is no central source of heat.

6. Living Area – Include any finished area including finished areas in basements. For two– family dwellings, include total combined areas.

7. Estimated Cost – Include the total cost of construction, but not cost of land or land-scaping.

SIGNATURE:

• Sign and date application form.

ISSUING JURISDICTION – This must be completed by the AUTHORITY HAVING JURISDICTION.

or

Check off MUNICIPALITY STATUS of issuing jurisdiction, such as town, village, city county.

Fill in MUNICIPALITY NUMBER OF DWELLING LOCATION. If issued by a county, indicate the specific municipality number where the dwelling will be built.

Fill in name of person issuing permit and date building permit issued.

PLEASE RETURN PINK COPY WITHIN 30 DAYS AFTER ISSUANCE TO (You may fold along the dashed lines and insert this form into a window envelope.):

Safety & Buildings Division P O Box 2509 Madison, WI 53701-2509

		INSPECT NOTICE OF	ION REPORT AN NONCOMPLIAN	D ICE	
Report Date:		Inspection Date	Permit No.:	Parcei No:	
Project Addr	ess		Subdivision	Lot No.:	Block No.:
Inspection Type(s)	Footing Heat/Vent/AC	Erosion Control	Foundation Electrical	Bsmt Drain Tile Insulation/Energy	Construction
Owner:			Contractor:		
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<u></u>					
AN INSPEC	CTION OF THE ABOVE P	REMISES HAS DISCLC	SED THE FOLLOWIN	CNONCOMPLIANCES:	
ORDER NO.	CODE SECTION		FINDINGS AN	DREQUIREMENTS	
		BAN			
				n An airtean a' Ann Ann	
	IMPORT	TANT: Please report w	hen violation are corre	ected. AVOID DELAY	
NOTICE O All cited viol	F NONCOMPLIANCE ations shall be corrected with er notice shall constitute a se	hin <u>30</u> days after write coarate offense and is subject	tten notification unless an ect to remedies and penal	extension of time is granted. Ea	ch day that the violation diction.
Enforceme	ent Town non: Village	County City State OF		Authority By Municipal Ordi	nance Section::
Inspector's N	ame:	Viola	tions Explained To:	Compliance Date:	

Office Hours:

Telephone No:

Inspector's Address:

DO NOT REMOVE OFFICIAL MUNICIPAL NOTICE OF VIOLATION

LOCATION:

Register, February, 1999, No 518



Safety and Buildings Division 201 W. Washington Avenue P O Box 7162 Madison, WI 53707–7162 Telephone: (608) 266-3151

PETITION FOR VARIANCE INFORMATION AND INSTRUCTIONS – ILHR 3

In instances where exact compliance with a particular code requirement cannot be met or alternative designs are desired, the Division has a petition for variance program where it reviews and considers acceptance of alternatives which are not in strict conformance with the letter of the code, but which meet the intent of the code. A variance is not a waiver from a code requirement. The petitioner must provide an equivalency which meets the intent of the code section petitioned to obtain a variance. Documentation of the rationale for the equivalency is requested below. Failure to provide adequate information may delay your petition. Pictures, sketches, and plans may be submitted to support equivalency. If the proposed equivalency does not adequately safeguard the health, safety, and welfare of building occupants. frequenters, firefighters, etc., the variance request will be denied. NOTE: A SEP-ARATE PETITION IS REQUIRED FOR EACH BUILDING AND EACH CODE ISSUE PETITIONED (i.e., 57.13 window issue cannot be processed on the same petition as 51.16 stair issue). It should be noted that a petition for variance does not take the place of any required plan review submittal.

The Division is unable to process petitions for variance that are not properly completed. Before submitting the application, the following items should be checked for completeness in order to avoid delays:

- Petitioner's name (typed or printed)
- Petitioner's signature •
- The Petition For Variance Application must be signed by the owner of the building or project unless a Power of Attorney is submitted.
- Notary Public signature with affixed seal
- Analysis to establish equivalency, including any pictures, illustrations or sketches of the existing and proposed conditions to clearly convey your proposal to the reviewer.
- Proper fee •

Any required position statements by fire chief or municipal official

A position statement from the chief of the local fire department is required for fire safety issues. No position statement is required for non-fire safety topics such as sanitary and energy conservation. Position statements for both the fire department and municipality are required for ILHR 69 barrier-free petitions. For rules relating to one- and two-family dwellings, only a position statement from the local enforcing municipality is required. Position statements must be completed and signed by the appropriate fire chief or municipal enforcement official. See the back of SBD-9890, Petition For Variance Application form for these position statement forms. Signatures or seals on all documents must be originals. Photocopies are not acceptable.

SBD-9890 (R.01/98)

Contact numbers and fees for the Division's review of the petition for variance are as follows:

Chapters ILHR 20–25, Uniform Dwelling Code	•••
Chapters ILHR 67–68, Rental Unit Energy Efficiency Code (608) 266–1930 \$125.00	•••
Chapters ILHR 50–64, Commercial Building Code (608) 266–1835 \$490.00	•••
Chapter ILHR 66, Uniform Multi–Family Dwellings (608) 266–0669 \$490.00	•••
 The cities of Milwaukee and Madison may process requests for variances from Chapters ILHR 50 through 64 requirements on projects in their jurisdiction.) 	; «
Chapter ILHR 66, Multifamily Dwelling (608) 266–1930	•••
Chapter ILHR 69, Barrier–Free Requirements	•••
Chapter ILHR 70, Historic Building Code (715) 524–3626 \$300.00	• • • •
All Other Chapters	
Boilers and Pressure Vessels (414) 548–8617 Electrical (608) 266–7529	
Elevators	

Priority Review: Does not apply to Uniform Dwelling Code or Historic Building Code issues which already are treated as a priority Double Above Amounts Except for special cases, the Division will review and make a determination on a petition for variance within 30 business days of receipt of all calculations, documents, and fees required for the review. Uniform Dwelling Code petitions will be processed within 5 business days. Priority petitions will be processed within 10 business days. **Petitions for variance should be submitted to:**

Safety and Buildings Division 201 West Washington Avenue P O Box 7162 Madison, Wisconsin 53707 (608) 266–3151

Elevator or barrier-free petitions may be submitted directly to the Waukesha office.

General Plumbing or Private Sewage petitions may be submitted to any of the six full-service offices.

GREEN BAY S&BD	HAYWARD S&BD	LACROSSE S&BD	MADISON S&BD	SHAWANO S&BD	WAUKESHA S&BD
2331 San Luis Place Green Bay, WI 54304 920–492–5601 FAX: 920–492–5604	15837 USH 63 Hayward, WI 54843 715-634-4870 FAX: 715-634-5150	2226 Rose Street La Crosse, WI 54603 608–785–9334 FAX: 608–785–9330	201 W. Washington Ave. P.O. Box 7162 Madison, WI 53707-7162 608-261-8490 FAV: 609 267 0566	1340 Green Bay St Shawano, WI 54166 715–524–3626 FAX: 715–524–3633	401 Pilot Court Waukesha, WI 53183 414–548–8600 FAX: 414–548–8614
and the second			FAX: 008-20/-9500		

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Plan No.	1		۶.	
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PETITION FOR VARIANCE APPLICATION

Safety and Buildings Division 201 W. Washington Ave. P.O. Box 7162 Madison. WI 53707 Page 1 of

Name	OR	2. Project Information	A Designed States	3. Designer Information		
Name		Building Occupancy Chapter(s) a	Ind Use	Designer	Registration No.	
Company Name		Tenant Name (if any)		Design Firm		
Number and Street		Building Location (number and s	ireet)	Number and Street		
City, State, Zip Code	· · · · · · · · · · · · · · · · · · ·	City Village Township of		City, State, Zip Code		
Contact Person		County of		Contact Person		
Telephone Number	FAX Number	Property ID # (tax parcel # - cont	act county)	Telephone Number	FAX Number	
Review by State Municipa Plan Number 5. State the code se		minary design oved, requesting revision nitted with petition AND the specific condition or is:	Duilt acc brought Plan wil Other sue you are n	t into compliance with of the submitted after pe	ut must be current code tition determination	
variance.		ter de la composition de la filme de la composition de la composition de la composition de la composition de l Composition de la composition de la comp				
6. Reason why com	pliance with the code	cannot be attained without the v	ariance			
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 State your proposisection petitioned 	ed means and rationa	ale of providing equivalent degre	e of health, s	afety, or welfare as ad	dressed by the code	
7. State your propos section petitioned	ed means and rationa	ale of providing equivalent degre	e of health, s	afety, or welfare as ad	dressed by the code	
 7. State your proposes section petitioned 3. List attachments texpert opinion, principal principal	ed means and rationa l o be considered as pa eviously approved va	ale of providing equivalent degre art of the petitioner's statements riances, pictures, plans, sketche	e of health, s (i.e., model c s, etc.)	afety, or welfare as ad ode sections, test repo	dressed by the code	
 7. State your proposes section petitioned 3. List attachments to expert opinion, pressure opinion, pre	ed means and rationa I to be considered as pa reviously approved val	ale of providing equivalent degre art of the petitioner's statements riances, pictures, plans, sketche	e of health, s (i.e., model c s, etc.).	afety, or welfare as ad	dressed by the code	
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SBD-9890 (R.01/98)

DEPARTMENT OF COMMERCE

Owner's Name	Project Location		Plan Number
Page 2 of			
To be completed for variance	Fire Department Posi ces requested from ILHR 5	tion Statemen D-64, ILHR 69, ILI	t HR 10, and other fire related
I have read the application Approval Condition Explanation for recommendation	requiremen for variance and recomm onal Approval De ation including any conflicts	ts. iend: (check app nial No with local rules a	ropriate box) Comment nd regulations and
suggested conditions:			
	/		
			······································
Fire Department Name and Address	· · · · · · · · · · · · · · · · · · ·		
Name of Fire Chief or Designee (type or p	print)	Tele	ephone Number
Signature of Fire Chief or Designee	<u></u>	Date	e Signed
MUNICIPAI To be completed for varia review is by municipality of I have read the application Approval Condition Explanation for recommendation include	L BUILDING INSPECT inces requested from ILHR r orders are written on the l cases. for variance and recommonal Approval De ding any conflicts with local rules and	ION RECOMM 20-23. Also to b building under con end: (check appl nial No d d regulations and sugge	IENDATION e used if ILHR 50-64 plan nstruction; optional in other ropriate box) Comment ested conditions:
			an a
		Marina da Angelanda Angelanda	
Municipality Exercising Jurisdiction			
Name and Address of Municipal Official (t	ype or print)	Telephone Nu	
Signature of Municipal Enforcement Offici	al	Date Signed	

MINIMUM FASTENER SCHEDULE TABLE

Other interior and exterior panel products and finishes installed per manufacturer requirements. For engineered connectors, use manufacturer's specified fasteners.

Description of Building Materials/Connection	Number and Type of Fas- tener ^{1 2 3}
Floor Framing	
Joist to sill or girder, toe nail	2–16d, 3–8d
Band or rim joist to joist, end nail	3–16d
Band or rim joist to sill or top plate	2–16d at 16" o.c.
Bridging to joist, toe nail each end	28d
Built-up girder and beams, top loaded	10d at 32" o.c. at top and bot- tom and staggered and two at ends and at each splice
Built-up girder and beams, side-loaded	16d at 16" o.c. at top and bot- tom and staggered and two at ends and at each splice
Ledger strip to beam, face nail	3–16d each joist
Joist on ledger to beam, toe nail	3–8d
Wall Framing	
Sole plate to joist or blocking, face nail	16d at 16" o.c.
Top or sole plate to stud, end nail	2–16d
Stud to sole plate, toe nail	48d or 316d
Doubled studs, face nail	16d at 24" o.c.
Doubled top plates, face nail	16d at 16" o.c.
Top plates, laps and intersections, face nail	2–16d
Continuous header, two pieces	16d at 16" o.c. along each edge
Continuous header to stud, toe nail	4-8d -
1" corner brace to each stud and plate, face nail	2-8d or 2 staples, 1 3/4"
Built-up corner studs	16d at 30" o.c., 16d at 24" o.c.
Roof/Ceiling Framing	
Ceiling joists to plate, toe nail	2–16d, 3–8d
Ceiling joist, laps over partitions, face nail	3–16d
Ceiling joist to parallel rafters, face nail	3–16d
Rafter to plate, toe nail (maximum 6' rafter span, engineered connector for longer)	2–16d, 3–8d
Roof rafters to ridge, valley or hip rafters, toe nail	4–16d
Roof rafters to ridge, valley or hip rafters, face nail	3–16d
Collar ties to rafters, face nail	3–8d
Boards and planks	
1" x 6" subfloor or less to each joist, face nail	2-8d or 2 staples, 1 3/4"
Wider than 1" x 6" subfloor toe to each joist, face nail	3-8d or 4 staples 1 3/4"
2" subfloor to joist or girder, blind and face nail	2–16d
1" x 6" roof sheathing to each bearing, face nail	2-8d or 2 staples, 1 3/4"
1" x 8" roof sheathing to each bearing, face nail	2-8d or 3 staples, 1 3/4"
Wider than 1" x 8" roof sheathing to each bearing, face nail	3-8d or 4 staples, 1 3/4"
2-inch planks	2–16d at each bearing

Panel Sheathing							
		Spacing of Fastener					
Material	Fastener	Edges	Intermediate S	upports			
Engineered wood panel for subfloor and roof sheathing and wall corner wind bracing to framing							
5/16-inch to 1/2-inch	6d common or deformed nail or staple, 1 1/2"	6"	12" ⁴				
5/8-inch to 3/4-inch	8d smooth or common, 6d deformed nail, or staple, 14 ga. 1 $\frac{3}{4}$ "	6"	12" ⁴				
7/8-inch to 1-inch	8d common or deformed nail	6"	12"				
1 1/8-inch to 1 1/4-inch	10d smooth or common, or 8d deformed nail	6"	12"				
Combination subfloor/ underlayment to framing							
3/4—inch or less	6d deformed or 8d smooth or common nail	6"	12"				
7/8-inch to 1-inch	8d smooth, common or deformed nail	6"	12"				
1 1/8-inch to 1 1/4-inch	10d smooth or common or 8d deformed nail	6"	12"				
Wood panel siding to fram- ing							
1/2-inch or less	6d corrosion-resistant siding and casing nails	6"	12"				
5/8—inch	8d corrosion-resistant siding and casing nails	6"	12"				

²All nails are smooth–common, box or deformed shank except where otherwise stated ²Nail is a general description and may be T–head, modified round head or round head. ³Staples are 16–gauge wire, unless otherwise noted, and have a minimum 7/16–inch o.d. crown width. ⁴Staples shall be spaced at not more than 10 inches o.c. at intermediate supports for floors.

UDC Floor & Ceiling Joist and Roof Rafter Span Tables And Design Value Tables

Use the following Span Tables to determine the maximum spans for floor and ceiling joists and roof rafters. These spans are based on:

- simple, single spans (although the tables may be safely used for continuous two-span floor joists)
- uniformly distributed loads
- fully supported members with one edge properly sheathed and nailed
- for floor joists and roof rafters, the top edge shall be properly sheathed and nailed

The criteria for each Span Table is given in the upper left hand corner and is also summarized in the table of Span Tables below. Choose the appropriate Span Table based on the member type and required loading. Select your desired member depth, member spacing and span to determine the minimum Fb value. Note that these tables include recommended deflection criteria. However, for strict code compliance, only the Fb strength requirements must be satisfied. The modulus of elasticity (E) values, would be met for serviceability purposes only.

Note that straight-line interpolation is permitted for intermediate spans and design values. Span is measured from face to face of supports. For sloping rafters, the span is measured along the horizontal projection.

Section Comm 21.27 allows reduction of the snow live load for roof slopes greater than 30 degrees (7/12 slope) based on the formula Cs = 1 - (a-30)/40, where "a" is the slope of the roof expressed in degrees. Following is a table of tabulated values for certain roof slopes.

Slope	Angle in Degrees	Zone 1 Live Load (psf)	Zone 2 Live Load (psf)
7/12	30	40	30
10/12	40	30	22.5
12/12	45	25	18.8
14/12	50	20	15

Use the Design Value tables following the Span Tables to determine the acceptable species and grades to satisfy minimum Fb values obtained from the Span Tables. The Design Value tables assume at least three members spaced no more than 24" on center. Use the Normal Duration column Fb values for joists and the Snow Loading column Fb values for rafters.

See the following examples for further guidance.

Tables are reprinted courtesy of American Forest & Paper Association.

Register, February, 1999, No 518

Table	Member	Live	Dead	Condition	(Deflection)*
No.	Туре	Load	Load		
		(psf)	(psf)		
F-2	Floor Joists	40	10	_	L/360
C-1	Ceiling Joists	10	5	Drywall ceiling, no attic storage	L/240
C-2	Ceiling Joists	20	10	Attic storage	L/240
R-2	Roof Rafters	30 (Zone	10	Maximum 2 layers of asphalt	L/240
		2)		shingles or wood shakes/shingles	м. Алт
R-3	Roof Rafters	40 (Zone	10	Maximum 2 layers of asphalt	L/240
· .		1)		shingles or wood shakes/shingles	
R-10	Roof Rafters	30 (Zone	20	Heavy roof covering (clay tile)	L/240
		2)	· · ·		
R-11	Roof Rafters	40 (Zone	20	Heavy roof covering (clay tile)	L/240
		1)			
R-14	Roof Rafters	30 (Zone	10	Maximum 2 layers of asphalt	L/180
· · ·		2)		shingles or wood shakes/shingles	
R-15	Roof Rafters	40 (Zone	10	Maximum 2 layers of asphalt	L/180
		1)		shingles or wood shakes/shingles	
R-22	Roof Rafters	30 (Zone	20	Heavy roof covering (clay tile)	L/180
		2)			
R-23	Roof Rafters	40 (Zone	20	Heavy roof covering (clay tile)	L/180
		1)			

*Deflection criteria are optional. For roof rafters with drywall on the underside, use the stricter L/240 tables to limit deflection.

Example 1. Floor Joists. Assume a required single span of 12'-9'', live load of 10 psf and joists spaced 16 inches on center. Table F-2 (see following highlighted tables) shows that one solution is a grade of 2x8 having an Fb value of 1255 would allow a span of 12'-10 which satisfies the condition. (Note that the recommended E value to limit deflection would be 1,600,000.) Going to the Design Value Tables, we find that as an example, 2x8 Hem Fir grade No.1 has an Fb value of 1310 for normal duration. (It also has an E value of 1,500,000 which satisfies the recommended deflection criteria.)

Example 2. Rafters. Assume a horizontal projected span of 13'-0", a live load of 40 psf, dead load of 10 psf, a roof slope of 4/12 and rafters spaced 16 inches on center. Since the slope is shallower than 7/12, there is no allowable reduction of the snow live load. Table R-3 shows that a 2x8 having an Fb value of 1300 would allow a span of 13'-1" which satisfies the condition. (Note that the recommended E value to limit deflection would be 1,120,000.) Going to the Design Value Tables, we find that as an example, 2x8 Douglas Fir-Larch grade No.2 has an Fb value of 1390 for snow loading. (It also has an E value of 1,600,000 which satisfies the recommended deflection criteria.)

Example 1 TABLE F- 2 FLOOR JOISTS WITH L/360 DEFLECTION LIMITS

DESIGN CRITERIA:

Deflection - For 40 psf live load. Limited to span in inches divided by 360. Strength - Live load of 40 psf plus dead load of 10 psf determines the required bending design value.

Joist Size	Spacing							Modulus of Elasticity, E, in 1,000,000 psi												
(in)	(in)	0.8	0.9	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4		
	12.0	8-6	8-10	9-2	9-6	9-9	10-0	10-3	10-6	10-9	10-11	11-2	11-4	11-7	11-9	11-11	12-1	12-3		
2. (16.0	7-9	8-0	8-4	8-7	8-10	9-1	9-4	9-6	9-9	9-11	10-2	10-4	10-6	10-8	10-10	11-0	11-2		
2X 0	19.2	1-3	7-7 7-0	7-10	8- I 7- 6	8-4	8-/	8-9	9-0 0-1	9-2	9-4 0 0	9-6	9-8	9-10	10-0	10-2	10-4	10-6		
	24.0	0-7	1-0	17.3	7-0	1- 7	7-11	0-2	0-4	0-0	0-0	8-1U	9-0	9-2	9-4	9-0	9-1	9-9		
	12.0	11-3	[]-8	12-1	12-6	12-10	13-2	13-6	13-10	14:2	14-5	14-8	15-0	15-3	15-6	15-9	15-11	16-2		
• •	16.0	10-2	10-7	11-0	11-4	11-8	12-0	12-3	12-7	(12-10)	13-1	13-4	13-7	13-10	14-1	14-3	14-6	14-8		
2x 8	19.2	9-7	10-0	10-4	10-8	11-0	11-3	11-7	11-10	12-1	12-4	12-7	12-10	13-0	13-3	13-5	13-8	13-10		
	24.0	0-11	9- 3	9.1	A-11	10-2	10- 0	10-9	11-0	11- 3	11-3	11-8	11-11	12-1	12-3	12-6	12-8	12-10		
	12.0	14-4	14-11	15-5	15-11	16-5	16-10	17-3	17-8	18-0	18-5	18-9	19-1	19-5	19-9	20- 1	20-4	20-8		
	16.0	13-0	13-6	14-0	14-6	14-11	15-3	15-8	16-0	16-5	16-9	17-0	17-4	17-8	17-11	18-3	18-6	18-9		
2x10	19.2	12-3	12-9	13-2	13-7	14-0	14-5	14-9	15-1	15-5	15-9	16-0	16-4	16-7	16-11	17-2	17-5	17-8		
	24.0	11-4	11-10	12-3	12-8	13-0	13-4	13-8	14-0	14-4	14-7	14-11	15-2	15-5	15-8	15-11	16-2	16-5		
	12.0	17-5	18-1	18-9	19-4	19-11	20-6	21-0	21-6	21-11	22-5	22-10	23-3	23-7	24-0	24-5	24-9	25-1		
	16.0	15-10	16-5	17-0	17-7	18-1	18-7	19-1	19-6	19-11	20-4	20-9	21-1	21-6	21-10	22-2	22-6	22.10		
2x12	19.2	14-11	15-6	16-0	16-7	17-0	17-6	17-11	18-4	18-9	19-2	19-6	19-10	20-2	20-6	20-10	21-2	21-6		
	24.0	13-10	14-4	14-11	15-4	15-10	16-3	16-8	17- ()	17-5	17-9	18-1	18-5	18-9	19-1	19-4	19-8	19-11		
F.	12.0	718	777	833	888	941	993	1043	1092	1140	1187	1233	1278	1323	1367	1410	1452	1494		
F,	16.0	790	855	917	977	1036	1093	1148	1202	(1255)	1306	1357	1407	1456	1504	1551	1598	1644		
F.	19.2	840	909	975	1039	1101	1161	1220	1277	1333	1388	1442	1495	1547	1598	1649	1698	1747		
F.	24.0	905	979	1050	1119	1186	1251	1314	1376	1436	1496	1554	1611	1667	1722	1776	1829	1882		
n																	•			

25- 1 22- 10 21- 6

Note: The required bending design value, F_a, in pounds per square inch is shown at the bottom of each table and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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20 APPENDIX

DEPARTMENT OF COMMERCE

Example	1	

Species and Grade	Size	Design	Value in	Modulus	Grading
		Bendin	g, "FD"	OI Floaticita	Kules
		Normal	Snow	Elasticity	Agency
·		Duration	Loading	E	1
Eastern White Pine				T	- Ì
Select Structural		2155	2480	1,200,000	
No.1	·	1335	1535	1,100,000	
No.2	_	990	1140	1,100,000	
No.3	2x4	605	695	900,000	
Stud		570	655	900,000	
Construction		775	895	1,000,000	
Standard		430	495	900,000	
Utility		200	230	800,000	
Select Structural		1870	2150	1,200,000	
No.1		1160	1330	1,100,000	
No.2	2x6	860	990	1,100,000	
No.3		525	600	900,000	
Stud		520	595	900,000	NELMA
Select Structural		1725	1985	1,200,000	NSLB
No.1	2x8	1070	1230	1,100,000	
No.2		795	915	1,100,000	-
No.3		485	555	900,000	
Select Structural		1580	1820	1,200,000	
No.1	2x10	980	1125	1,100,000	
No.2		725	835	1,100,000	
No.3		445	510	900,000	
Select Structural		1440	1655	1,200,000	
No.1	2x12	890	1025	1,100,000	
No.2		660	760	1,100,000	
No.3		405	465	900,000	
Hem Fir					ļ
Select Structural		2415	2775	1,600,000	
No.1 & Btr	1 1	1810	2085	1,500,000	
No.1	1 1	1640	1885	1.500.000	
No.2	1 1	1465	1685	1,300,000	
No.3	2x4	865	990	1,200,000	
Stud		855	980	1.200.000	
Construction	1 1	1120	1290	1.300.000	1
Standard	1 1	635	725	1,200,000	
Utility	1 1	290	330	1,100,000	
Select Structural		2095	2405	1,600,000	
No.1 & Btr		1570	1805	1.500.000	
No.1	2x6	1420	1635	1.500.000	
No.2	1 [1270	1460	1,300,000	
No.3	1 1	750	860	1.200.000	
Stud	1 [775	895	1,200,000	
Select Structural		1930	2220	1,600,000	WCLIB
No.1 & Btr	1	1450	1665	1.500.000	WWPA
No 1	2+8	1310	1510	1 500 000	31 1 1 1 1
No.2	. 240	1176	1350	1,300,000	
No.2	┥╴┝	1175	1350	1,300,000	2 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -
NO.3	<u> </u>	1770	/95	1,200,000	
Ne 1 8 Day	4 -	1770	2035	1,000,000	9 a
	1 2.10	1330	1545	1,500,000	as statistica 🖡
No.1		1200	1380	1,500,000	
NO.2	4 1	1075	1235	1,300,000	
No.3	<u> </u>	635	725	1,200,000	
Select Structural	1 L	1610	1850	1,600,000	
No.1 & Btr		1210	1390	1,500,000	
No.1	2x12	1095	1255	1,500,000	· · · · · ·
NO.2	4	980	1125	1,300,000	
No.3		575	660	1,200,000	

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Example 2 TABLE R-3 RAFTERS WITH L/240 DEFLECTION LIMITATION

DESIGN CRITERIA: Strength - Live Load of 40 psf plus

Dead Load of 10 psf determines the required bending design value. Deflection - For 40 psf live load. Limited to span in inches divided by 240.

AL												P Design	1 4140, 1	Pi (hari				e de					
Size Spa	acing											n de la composición d Composición de la composición de la comp											
		300	400	500	600	700	800	900	1000	1100	1200	(1300)	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400
	n Alexand Alexandri Alexandri										4	T ^{ant}		1.2									4
12.0	.0	5-6	6-4	7-1	7-9	8-5	9-0	9-6	10-0	10-6	11-0	11-5	11-11	12-4	12-8	13-1	13-6	13-10	14-2				
2x 6 19 1	.0	4-9	5-6	6-2 5-7	6-9	7-3	7-9	8-3	8-8	9-1	9-6	9-11	10-3	10-8	11-0	11-4	11-8	12.0	12-4	12-7	12-11	10.0	
24.0	.0	3-11	4-6	5-0	5-6	5-11	6-4	6-9	7-1	7-5	7-9	8-1	8-5	9-9 8-8	9-0	10- 4 9- 3	9-6	9-9	10-0	10-3	11-9	12- 0 10- 9	12-4 11-0
12.(.0	7-3	8-4	9-4	~10-3	11-1	11-10	12-7) 13-3	13-11	14-6	15-1	15-8	16-3	16-9	17.3	17-9	18- 1	18.0				
16.0	.0	6-3	7-3	8- i	8-11	9-7	10-3	10-10	11-6	12-0	12-7		13-7	14-0	14-6	14-11	15-5	15-10	16-3	16-7	17-0		
2x 8 19.2	.2	5-9	6-7	7-5	8-1	8-9	9-4	9-11	10-6	11-0	11-6	11-11	12-5	12-10	13-3	13-8	14-0	14-5	14-10	15-2	15-6	15-10	16-3
24.0	.0	3- 2	3-11	0-7	1-3	7-10	8-4	8-11	9-4	9-10	10-3	10-8	41-1	11-6	11-10	12-2	12-7	12-11	13-3	13-7	13-11	14-2	14-6
12.(.0	9-3	10-8	11-11	13- i	14-2	15-1	16-0	16-11	17-9	18-6	19-3	20.0	20- 8	21-4	22-0	22-8	23-3	23-11				
16.0	.0: 	8-0	9-3	10-4	11-4	12-3	13-1	13-10	14-8	15-4	16-0	16-8	17-4	17-11	18-6	19-1	19-7	20-2	20-8	21-2	21-8	•••	.
24.0	.0	6-6	8-3 7-7	9- 3 8- 5	10-4 9-3	10-0	10-8	12-8	13-4	14-0	14-8	13-3	13-10	10-4 14-8	10-11 15-1	17-5	17-11 16- 0	18-5 16-6	18-11	19-4 17-4	19-10	20- 3 18- 1	20-8 18-6
															-								÷.,
107	A		12 A			12.2		10 4			~ ~ ~							1					
12.0	.0	9.9	13-0	14-0	13-11	17-2	18-4	19-0	20-6	21-7	22-0	23-5	24-4	25-2	26.0	22.5	23.10	24.6	25.2	25.0			
2x12 19.3	.2	8-11	10-3	11-6	12-7	13-7	14-6	15-5	16-3	17-0	17.9	18-6	19-3	19-11	20-6	21-2	21-9	22-5	23-0	23-6	24-1	24-8	25-2
24.(.0	7-11	9-2	10-3	11-3	12-2	13-0	13-9	14-6	15-3	15-11	16-7	17-2	17-9	18-4	18-11	19-6	20.0	20-6	21-1	21-7	22-0	22-6
E 12.0	2.0	0.14	0.22	0.31	0.41	0.51	0.63	0.75	-0.88	1.01	1.15		1.45	L61	1.77	1.94	2.12	2.30	2.48				
E 16.0	0.0	0.12	~0.19	0.27	0.35	0.44	0.54	0.65	0.76	0.88	1.00	1.12	1.26	1.39	1.54	1.68	1.83	1.99	2.15	2.31	2.48	2.42	2 60
E 24.(.0	0.10	0.16	0.24	0.32	0.41	0.50	0.59	0.69	0.80	0.91	0.92	1.15 1.03	1.14	1.40	1.34	1.07	1.81	1.90	2.11 1.89	2.20 2.02	2.42 2.16	2.58 2.30

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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Example 2

Species and Grade	Size	Design Bendin	Value in g, "Fb"	Modulus of	Grading Rules
		Normal	Snow	Elasticity	Agency
		Duration	Loading	"E"	
Cottonwood					-l
Select Structural		1510	1735	1 200 000	٦
No 1		1080	1240	1 200,000	4
No 2		1080	1240	1,200,000	4
No.3	2×4	605	605	1,100,000	4
Stud	- 224	600	600	1,000,000	4
Construction		000	090	1,000,000	4
Standard		460	520	1,000,000	4
Utility		200	330	900,000	-
Select Structural		1310	1505	900,000	4
No.1		025	1005	1,200,000	4 .
No.2	- 226	935	1075	1,200,000	4
No.3	- 2.0	935	600	1,100,000	4
Stud		545	620	1,000,000	
Sidu Select Structurel		1210	1200	1,000,000	NOLD
No 1		1210	1390	1,200,000	NSLB
No.2	- 200	805	990	1,200,000	
No.2		805	990	1,100,000	
Folget Structurel		485	1075	1,000,000	
No.1	- 2	1105	12/5	1,200,000	·
No.1	-2×10	790	910	1,200,000	
No.2	{ } }	/90	910	1,100,000	
INO.3		445	510	1,000,000	
Select Structural		1005	1155	1,200,000	
No.1	-2x12	720	825	1,200,000	
No.2		/20	825	1,100,000	
IN0.3		405	465	1,000,000	
Douglas Fir-Larch		· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·		
Select Structural		2500	2875	1,900,000	
No.1 & Btr		1985	2280	1,800,000	
No.1		1725	.1985	1,700,000	
No.2		1510	1735	1,600,000	
No.3	2x4	865	990	1,400,000	
Stud	[855	980	1,400,000	
Construction		1150	1325	1,500,000	. 1
Standard		635	725	1,400,000	1
Utility		315	365	1,300,000	[
Select Structural		2170	2495	1,900,000	1
No.1 & Btr		1720	1975	1,800,000	
No.1	2x6	1495	1720	1,700,000	
No.2		1310	1505	1,600,000	
No.3	7 Γ	750	860	1,400,000	
Stud	7 · · r	775	895	1,400,000	
Select Structural		2000	2300	1.900.000	WCLIB
No.1 & Str	-1 F	1585	1825	1.800.000	WWPA
No 1	2x8	1380	1585	1 700 000	
Nio 2	240	1010	1300	1,700,000	
N0.2	┩┝	1210	1390	1,600,000	
No.3	_	690	795	1,400,000	· •
Select Structural	-	1835	2110	1,900,000	
No.I & Btr	┥⊢	1455	1675	1,800,000	
No.I	2x10	1265	1455	1,700,000	1
No.2	1 L	1105	1275	1,600,000	. 1
No.3		635	725	1,400,000	
Select Structural	L L	1670	1920	1,900,000	
No.1 & Btr	J · E	1325	1520	1,800,000	
No.1	2x12	1150	1325	1,700,000	
No.2		1005	1155	1,600,000	
No.3		575	660	1,400,000	
					and the second se

 TABLE F- 2

 FLOOR JOISTS WITH L/360 DEFLECTION LIMITS

DESIGN CRITERIA:

Deflection – For 40 psf live load. Limited to span in inches divided by 360. Strength – Live load of 40 psf plus dead load of 10 psf determines the required bending design value.

Joist					3 E.			Modulu	is of Elastic	ity, E, in 1	,000,000 ps	si						
Size	Spacing (in)	g								• • •	•							
(III)	(m)	0.8	0.9	10	1.1	12	i 3	i 4	15	14	17	1.0		• •				
	414 2		0.2			1.2	1.5	1.4	1.5	1.0	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4
	12.0	8-6	8-10	9-2	9-6	9-9	10-0	10-3	10-6	10-9	10-11	11-2	11-4	11-7	11_9	11	12 1	12 2
	16.0	7-9	8-0	8-4	8-7	8-10	9 ∸ i	9-4	9-6	9-9	9-11	10-2	10-4	10-6	10-8	10-10	14-1	12-5
2x 6	19.2	73	77	710	8-1	8-4	8-7	8-9	9-0	9-2	9-4	9-6	9-8	9-10	10 - 0	10-10	10 4	10 6
	24.0	6-9	7-0	7-3	7-6	79	7-11	8-2	8-4	8-6	8 8	8-10	90	9-2	9-4	9-6	0_7	0_0
																J = 0	<i>y</i> - <i>i</i>	9-9
	12.0	11-3	11-8	12- 1	12-6	12-10	13-2	13 6	13-10	i4-2	14-5	14.8	15 0	15 2	15 6	15 0	15 11	16.0
	16.0	10-2	10 7	11-0	11-4	11-8	12 - 0	12-3	12-7	12-10	13-1	13-4	13-0	13-5	13-0 14 1	11 2	13-11	10~2
2x 8	19.2	9-7	10-0	10-4	10-8	11-0	11-3	11-7	11-10	12-1	12-4	12-7	12-10	13-10	13_3	14-5	14-0	14-8
	24.0	8-11	9-3	9-7	9-11	10-2	10-6	10-9	11-0	11-3	11-5	11-8	11-11	13=0 12 i	12-3	13 - 3 12 - 6	13-0	13-10
									1 - 1 - 1 - 1						12 5	12-0	12-0	12-10
	12.0	14-4	14-11	15-5	15-11	16-5	16-10	17-3	17-8	18-0	18-5	18 0	10 i	10 5	10 0	20 1		
	16.0	13-0	13-6	i4-0	14-6	14-11	15-3	15-8	16-0	16-5	16_9	17_0	19-1	19-5	19-9	20-1	20-4	20-8
2x10	19.2	12-3	12-9	13-2	13-7	14-0	14-5	14-9	15-1	15 5	15-9	17 = 0 16 = 0	16_4	16_7	16-11	10-3	10-0	18-9
	24.0	11-4	11-10	12-3	12-8	13-0	13-4	13-8	14-0	14-4	14-7	14-11	15-2	15-5	15-8	17-2	17-3	1/- 0
				and the					e a di ji					10 0	15 0	1511	10-2	10- 5
	12.0	17-5	18- i	18-9	19-4	19-11	20-6	21-0	21-6	21-11	22-5	22-10	23-3	23-7	24-0	24 5	24- 9	25 1
	16.0	15-10	16-5	17-0	17-7	18 i	187	19-1	19-6	19-11	20-4	20-9	21-1	21-6	21-10	22-2	22-6	23 - 1 22 - 10
2x12	19.2	14-11	15-6	16-0	16-7	17-0	17-6	17-11	18-4	18-9	19-2	19-6	19-10	20-2	20-6	20 - 10	21 - 2	21 - 6
	24.0	13-10	14-4	14-11	15-4	15-10	16-3	16-8	17-0	17-5	17-9	18-1	18 5	18-9	19– i	19-4	19-8	19-11
							:-1					and the second	el a tile					
	1																	
Fb	12.0	718	777	833	888	941	993	1043	1092	1140	1187	1233	1278	1323	1367	1410	i452	i 494
F	16.0	790	855	917	977	1036	1093	1148	1202	1255	1306	1357	1407	1456	1504	1551	1598	1644
F.	19.2	840	909	975	1039	1101	1161	1220	1277	1333	1388	1442	1495	1547	1598	1649	1698	1747
F	24.0	905	979	1050	1119	1186	1251	1314	1376	1436	1496	1554	1611	1667	1722	1776	1829	1882

Note: The required bending design value, F_b, in pounds per square inch is shown at the bottom of each table and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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 TABLE C-1

 CEILING JOISTS WITH L/240 DEFLECTION LIMITS

DESIGN CRITERIA:

Deflection – For 10 psf live load. Limited to span in inches divided by 240. Strength – Live Load of 10 psf plus dead load of 5 psf determines the required fiber stress value.

Joist Size	Snacin							Modulu	s of Elastic	ity, E, in 1	,000,000 ps	i						
(in)	(in)	5																
()	()	0.8	0.9	1.0	1.1	1.2	1.3	i.4	1.5	1.6	1.7	1.8	i.9	2.0	2.1	2.2	2.3	2.4
	12.0	9–10 8–11	10-3	10-7 9-8	10-11	11-3	11-7	11-10	12-2	12-5	12 8	12-11	13-2	13-4	13-7	13-9	14-0	14-2
2x 4	19.2 24.0	8-5 7-10	8-9 8-1	9- i 8- 5	9-4 8-8	9– 8 8–11	9-11 9-2	10-2 9-5	10-4 9-8	10-7 9-10	10–10 10– 0	11-0 10-3	11–3 10–5	12-2 11-5 10-7	12-4 11-7 10-9	12-0 11-9 10-11	12-9 12-0 11-1	12-11 12-2 11-3
								5										
	12.0 16.0	15-6 14-1	16- i 14- 7	16– 8 15– 2	17– 2 15– 7	17– 8 16– i	18-2 16-6	18-8 16-11	19– 1 17– 4	196 178	19–11 18– 1	20 3 18 5	20- 8 18- 9	21-0 19-1	21–4 19–5	21 8 19 8	22- 0 20- 0	22- 4 20- 3
2x 6	19.2 24.0	13-3 12-3	13-9 12-9	14-3 13-3	14-8 13-8	15-2 14-1	15–7 14–5	15-11 14-9	16-4 15-2	16– 8 15– 6	17-0 15-9	17– 4 16– 1	17- 8 16- 4	17-11 16- 8	18– 3 16–11	18– 6 17– 2	18–10 17– 5	19- i 17- 8
	12.0 16.0	20-5 18-6	21-2 19-3	21–11 19–11	22-8 20-7	23-4 21-2	24-0 21-9	24– 7 22– 4	25-2 22-10	25-8 23-4	23-10	24-3	24-8	25-2	25-7	25-11		
2x 8	19.2 24.0	17-5 16-2	18–1 16–10	18–9 17–5	19– 5 18– 0	19–11 18– 6	20– 6 19– 0	21-0 19-6	21-6 19-11	21–11 20– 5	22-5 20-10	22–10 21–2	23-3 21-7	23-8	24 0 22 4	24- 5 22- 8	24– 9 23– 0	25-2 23-4
				·														
	12.0	26-0	04.7	05 F														
2x10	10.0 19.2 24.0	23 - 8 22 - 3 20 - 8	247 `23-1 21-6	23-5 23-11 22-3	24-9 22-11	25-5 23-8	24_ 3	24-10	25_5	26-0								
	24.0	20-0	2,1-0	44-5	22-11	25-0	2 1 − 5	2710	25-5	20-0								
F. F.	12.0 16.0	711 783	769 847	825 909	880 968	932 1026	983 1082	1033 1137	1082 1191	1129 1243	1176 1294	1221 1344	1266 1394	1310 1442	1354 1490	1396 1537	1438 1583	1480 1629
F. F.	19.2 24.0	832 896	900 969	965 1040	1029 1108	1090 1174	1150 1239	1208 1302	1265 1363	1321 1423	1375 1481	1429 1539	1481 1595	1533 1651	1583 1706	1633 1759	1682 1812	1731 1864

The required bending design value, Fe, in pounds per square inch is shown at the bottom of each table and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are

limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'

Register, February, 1999, No :

TABLE C-2 CEILING JOISTS WITH L/240 DEFLECTION LIMITS

DESIGN CRITERIA:

Register, February, 1999, No. 518

Deflection – For 20 psf live load.
Limited to span in inches divided by 240.
Strength – Live Load of 20 psf plus
dead load of 10 psf determines the required bending design value.

2							Jois	Modulus o	f Elasticity	, E, in 1,0	00,000 psi					2		
Size	Spacin	Ig																
(111)	(m)	0.8	0.9	1.0	- 1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4
2x 4	12.0	7-10	8-1	8-5	8-8	8-11	9-2	9 5	9-8	9-10	10 0	10-3	10-5	10- 7	10-9	10–11	11- 1	11-3
	16.0	7-1	7-5	7-8	7-11	8- 1	8-4	8 7	8-9	8-11	9 1	9-4	9-6	9- 8	9-9	9–11	10- 1	10-3
	19.2	6-8	6-11	7-2	7-5	7- 8	7-10	8 1	8-3	8-5	8 7	8-9	8-11	9- i	9-3	9– 4	9- 6	9-8
	24.0	6-2	6-5	6-8	6-11	7- 1	7-3	7 6	7-8	7-10	8 0	8-1	8-3	8- 5	8-7	8– 8	8-10	8-11
2x 6	12.0	12-3	12-9	13-3	13-8	14-1	14-5	14-9	15-2	15-6	15-9	16- 1	16-4	16-8	16–11	17-2	17- 5	17– 8
	16.0	11-2	11-7	12-0	12-5	12-9	13-1	13-5	13-9	14-1	14-4	14- 7	14-11	15-2	15– 5	15-7	15-10	16– 1
	19.2	10-6	10-11	11-4	11-8	12-0	12-4	12-8	12-11	13-3	13-6	13- 9	14-0	14-3	14– 6	14-8	14-11	15– 2
	24.0	9-9	10-2	10-6	10-10	11-2	11-5	11-9	12-0	12-3	12-6	12- 9	13-0	13-3	13– 5	13-8	13-10	14– 1
2x 8	12.0	16-2	16-10	17-5	18– 0	18-6	19-0	19– 6	19–11	20- 5	20-10	21-2	21-7	21–11	22 4	22- 8	23- 0	23 4
	16.0	14-8	15-3	15-10	16– 4	16-10	17-3	17– 9	18– 1	18- 6	18-11	19-3	19-7	19–11	20 3	20- 7	20-11	21 2
	19.2	13-10	14-5	14-11	15– 5	15-10	16-3	16– 8	17– 1	17- 5	17- 9	18-1	18-5	18– 9	19 1	19- 5	19- 8	19-11
	24.0	12-10	13-4	13-10	14– 3	14-8	15-1	15– 6	15–10	16- 2	16- 6	16-10	17-2	17– 5	17 9	18- 0	18- 3	18 6
2x10	12.0 16.0 19.2 24.0	20- 8 18- 9 17- 8 16- 5	21 6 19 6 18 4 17 0	22-3 20-2 19-0 17-8	22-11 20-10 19-7 18-3	23-8 21-6 20-2 18-9	24-3 22-1 20-9 19-3	24–10 22– 7 21– 3 19– 9	25- 5 23- 1 21- 9 20- 2	26-0 23-8 22-3 20-8	24- 1 22- 8 21- 1	24- 7 23- 1 21- 6	25-0 23-7 21-10	25- 5 23-11 22- 3	25–10 24–4 22–7	24-9 22-11	25- í 23- 4	25-5 23-8
Fb	12.0	896	969	1040	1108	1174	1239	1302	1363	1423	1481	1539	1595	1651	1706	1759	1812	1864
Fb	16.0	986	1067	1145	1220	1293	1364	1433	1500	1566	1631	1694	1756	1817	1877	1936	1995	2052
Fb	19.2	1048	1134	1216	1296	1374	1449	1522	1594	1664	1733	1800	1866	1931	1995	2058	2120	2181
Fb	24.0	1129	1221	1310	1396	1480	1561	1640	1717	1793	1866	1939	2010	2080	2149	2217	2283	2349

Note:

The required bending design value, F₈, in pounds per square inch is shown at the bottom of each table and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

TABLE R-2 RAFTERS WITH L/240 DEFLECTION LIMITATION

DESIGN CRITERIA:

Strength – Live Load of 30 psf plus Dead Load of 10 psf determines the required bending design value. Deflection – For 30 psf live load. Limited to span in inches divided by 240.

									RafB	ending l	Design V	alue, F,,	(psi)										
Size	Spacing																	1					
(in)	(11)	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400
	12.0	6-2	7-1	7-11	8-8	9-5	10-0	10 8	11-3	11-9	12-4	12-10	13-3	13-9	14-2	14 8	15-1	15-6	15_11				
	16.0	5-4	6-2	6-10	7-6	82	8-8	9-3	9-9	10-2	10-8	11-1	11-6	11-11	12-4	12 8	13-1	13-5	13-9	14-1	14-5		
2x 6	19.2	4-10	5-7	6-3	6-10	7-5	7-11	8-5	8-11	9-4	9-9	10– i	10-6	10-10	11-3	11-7	11-11	12-3	12 7	12-10	13-2	13-6	
	24.0	4-4	5-0	57	6-2	6-8	7- I	7-6	7-11	8 4	8 8	9– 1	9- 5	9-9	10-0	10-4	10-8	10-11	11-3	11-6	11-9	12-0	12 4
	12.0	· 8 1	0_1	10.6	11_6	12.5	12_3	14 0	14 10	15 6	16 3	16 10	17 6	10 î	10 0	10 4	10 10	20 5	20, 11				
	16.0	0-1 7-0	8-1	9_1	9-11	12-5	11-6	17 - 2	14-10	13-5	i40	10-10	15-2	15-1	16-3	15-4	17-2	17-8	18-1	18-7	19-0		
2x 8	19.2	6-5	7-5	8-3	9-1	9-9	10-6	11-1	11-8	12-3	12-10	13-4	13-10	14-4	14-10	15-3	15-8	16-2	16-7	16-11	17-4	17-9	
	24.0	5-9	6-7	7-5	8 1	8-9	9-4	9-11	10-6	11-0	11-6	11-11	12 5	12-10	13-3	13-8	14-0	14-5	14-10	15-2	15-6	15-10	16-3
	12.0	10-4	11-11	13-4	14-8	15-10	16-11	17-11	18-11	19-10	20- 8	21-6	22– 4	23-1	23-11	24 7	25-4	26-0		-			
	16.0	8-11	10-4	11-7	12-8	13-8	14 8	15-6	16-4	17-2	17-11	18-8	19-4	20-0	20-8	21-4	21-11	22-6	23-1	23-8	24-3		;
2x10	19.2	8-2	9-5	10-7	11-7	12-6	13-4	14-2	14-11	15-8	16-4	17-0	17-8	18-3	18-11	19-6	20-0	20-7	21– I	21-8	22-2	22-8	
	24.0	7-4	8 5	9– 5	10-4	11-2	11-11	12-8	13-4	14-0	i4 8	15-3	15–10	16 4	16-11	17-5	17–11	18-5	18-11	19-4	19-10	20-3	20 8
	12.0	12-7	14-6	16-3	17-9	19-3	20-6	21-9	23-0	24-1	25-2												
	16.0	10-11	12-7	14-1	15-5	16 8	17-9	18-10	19-11	20-10	21-9	22-8	23-6	24-4	25-2	25-11							
2x12	19.2	9-11	11-6	12-10	14-1	15-2	16-3	17-3	18-2	19-0	19-11	20-8	21-6	22-3	23-0	23-8	24-4	25-0	25-8				
	24.0	8-11	10-3	11-0	12-7	13 7	14-6	15-5	16-3	17-0	17-9	18-6	19–3	1911	20 6	21-2	21-9	22-5	23-0	23-6	24 1	24-8	25-2
						. :						•											
E	12.0	0.15	0.23	0.32	0.43	0.54	0.66	0.78	0.92	1.06	1.21	1.36	1.52	1.69	1.86	2.04	2.22	2.41	2.60	0.40	0.00		
E E	16.0	0.13	0.20	0.28	0.37	0.47	0.57	0.68	0.80	0.92	1.05	1.18	1.32	1.40	1.01	1.76	1.92	2.08	2.25	2.42	2.60	1 52	
E	24.0	0.12	0.16	0.20	0.34	0.38	0.52	0.55	0.75	0.75	0.85	0.96	1.20	1.19	1.47	1.44	1.75	1.90	2.05 1.84	1.98	2.12	2.33	2.41
-	- ···		5	5.20	0.00	5100	5	5100	5.00	0.70	0100	0.20										2.27	2
			с. ¹	t Satu																			

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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TABLE R-3 RAFTERS WITH L/240 DEFLECTION LIMITATION

DESIGN CRITERIA:

Strength – Live Load of 40 psf plus
Dead Load of 10 psf determines the required bending design value.
Deflection - For 40 psf live load.
Limited to span in inches divided by 240.

Cino	C masture								Rate	ending 1	Design V	Value, F	, (psi)										
Size	Spacing																						
(11)	(111)	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400
2x 6	12.0 16.0 19.2 24.0	5-6 4-9 4-4 3-11	6-4 5-6 5-0 4-6	7-1 6-2 5-7 5-0	7-9 6-9 6-2 5-6	85 73 68 5-11	9- 0 7- 9 7- 1 6- 4	9- 6 8- 3 7- 6 6- 9	10- 0 8- 8 7-11 7- 1	10- 6 9- 1 8- 4 7- 5	11-0 9-6 8-8 7-9	11-5 9-11 9-1 8-1	11-11 10-3 9-5 8-5	12- 4 10- 8 9- 9 8- 8	12-8 11-0 10-0 9-0	13- 1 11- 4 10- 4 9- 3	13- 6 11- 8 10- 8 9- 6	13-10 12-0 10-11 9-9	14-2 12-4 11-3 10-0	12-7 11-6 10-3	12–11 11– 9 10– 6	12 0 10 9	12–4 11–0
2x 8	12.0 16.0 19.2 24.0	7-3 6-3 5-9 5-2	8-4 7-3 6-7 5-11	9-4 8-1 7-5 6-7	10-3 8-11 8-1 7-3	11- 1 9- 7 8- 9 7-10	11–10 10– 3 9– 4 8– 4	12-7 10-10 9-11 8-11	13-3 11-6 10-6 9-4	13-11 12-0 11-0 9-10	14-6 12-7 11-6 10-3	15– 1 13– 1 11–11 10– 8	15– 8 13– 7 12– 5 11– 1	16-3 14-0 12-10 11-6	16– 9 14– 6 13– 3 11–10	17-3 14-11 13-8 12-2	17– 9 15– 5 14– 0 12– 7	18-3 15-10 14-5 12-11	18-9 16-3 14-10 13-3	16- 7 15- 2 13- 7	17-0 15-6 13-11	15–10 14– 2	16– 3 14– 6
2x10	12.0 16.0 19.2 24.0	9 3 8 0 7 4 6 6	10- 8 9- 3 8- 5 7- 7	11–11 10– 4 9– 5 8– 5	13- 1 11- 4 10- 4 9- 3	14-2 12-3 11-2 10-0	15– 1 13– 1 11–11 10– 8	16-0 13-10 12-8 11-4	16-11 14- 8 13- 4 11-11	17- 9 15- 4 14- 0 12- 6	18 6 16- 0 14- 8 13- 1	19-3 16-8 15-3 13-7	20-0 17-4 15-10 14-2	20- 8 17-11 16- 4 14- 8	21–4 18–6 16–11 15– i	22- 0 19- 1 17- 5 15- 7	22- 8 19- 7 17-11 16- 0	23-3 20-2 18-5 16-6	23-11 20-8 18-11 16-11	21–2 19–4 17–4	21- 8 19-10 17- 9	20-3 18-1	20 8 18 6
2x12	12:0 16:0 19:2 24:0	11-3 9-9 8-11 7-11	13-0 11-3 10-3 9-2	14-6 12-7 11-6 10-3	15–11 13–9 12–7 11–3	17– 2 14–11 13– 7 12– 2	18-4 15-11 14-6 13-0	19- 6 16-10 15- 5 13- 9	20- 6 17- 9 16- 3 14- 6	21-7 18-8 17-0 15-3	22- 6 19- 6 17- 9 15-11	23- 5 20- 3 18- 6 16- 7	24-4 21-1 19-3 17-2	25-2 21-9 19-11 17-9	26- 0 22- 6 20- 6 18- 4	23-2 21-2 18-11	23-10 21-9 19-6	24- 6 22- 5 20- 0	25-2 23-0 20-6	25– 9 23– 6 21– 1	24– 1 21– 7	24 8 22 0	25- 2 22- 6
E E E E	12.0 16.0 19.2 24.0	0.14 0.12 0.11 0.10	0.22 0.19 0.18 0.16	0.31 0.27 0.24 0.22	0.41 0.35 0.32 0.29	0.51 0.44 0.41 0.36	0.63 0.54 0.50 0.44	0.75 0.65 0.59 0.53	0.88 0.76 0.69 0.62	1.01 0.88 0.80 0.71	1.15 1.00 0.91 0.81	1.30 1.12 1.03 0.92	i.45 i.26 i.15 i.03	1.61 1.39 1.27 1.14	1.77 1.54 1.40 1.25	1.94 1.68 1.54 1.37	2.12 1.83 1.67 1.50	2.30 1.99 1.81 1.62	2.48 2.15 1.96 1.75	2.31 2.11 1.89	2.48 2.26 2.02	2.42 2.16	2.58 2.30

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

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TABLE R-10 RAFTERS WITH L/240 DEFLECTION LIMITATION

DESIGN CRITERIA:

Register, February, 1999, No :

Strength – Live Load of 30 psf plus Dead Load of 20 psf determines the required bending design value. Deflection – For 30 psf live load. Limited to span in inches divided by 240.

Size	Spacing																							·····	· · ·	
(in)	(in)											Raf	ter Bendi	ig Design	Value, Fb	, (psi)										
		300	400	500	600	700	800	900	1000	T100	1200	1300	1400	1500	1600	1700	18(X)	1900	2000	2100	2200	2300	2400	2500	2600	27(0)
									- A																	
	12.0	5-6	6-4	7-1	7-9	8-5	9-0	9-6	10-0	9-7	10-0	10-5	10-10	11-3	11-7	11-11	124	12-8	13-0	13-3	13-7	13-11	142			l
	16.0	4-9	5-6	6-2	6-9	7-3	7-9	83	8-8	8-4	8-8	9-1	9-5	9-9	10-0	10-4	10-8	10-11	11-3	11-6	11-9	12-0	12-4	12-7	12-10	13-1
2x6	19.2	4-4	5-()	5-7	6-2	68	7-1	7-6	7-11	7_7	7-11	83	8-7	8-11	9-2	9-5	9-9	10-0	10-3	10-6	10-9	11-0	11-3	11-5	118	11-11
	24.0	3-11	4-6	5-0	5-6	5-11	6-4	6-9	7-1	6-10	7-1	7-5	7-8	7-11	8-2	8-5	8-8	8-11	92	9-5	9-7	9-10	10-0	10-3	10-5	10-8
		1.18		1.1.1				to the state]			1	·		1.1			 			1.1	1.11		
	12.0	7-3	8-4	9-4	10-3	11-1	11-10	12-7	13-3	12-8	13-3	13-9	14-4	14-10	15-3	15-9	16-3	16-8	17-1	176	17-11	18-4	189			
	16.0	6-3	73	8-1	8-11	9-7	10-3	10-10	11-6	11-0	11-6	11-11	12-5	12-10	13-3	13-8	14-0	14-5	14-10	15-2	15-6	15-10	16-3	16-7	16-10	172
2x8	19.2	5-9 : •	67	7-5	8-1	8-9	9-4	9-11	10-6	10-0	10-6	10-11	11-4	11-8	12-1	125	12-10	13-2	13-6	1310	142	14-6	14-10	15-1	15-5	15-8
	24.0	5-2	5-11	6-7	7-3	7-10	8-4	8-11	9-4	9-0	9–4	9-9	10-1	10-6	10-10	11-2	11-6	11-9	12-1	12-5	128	12-11	13-3	13-6	13-9	14-0
					1.1.1.1										1							1 · ·				
	12.0	9-3	10-8	11-11	13-1	14-2	15-1	16-0	16-11	162	16-11	17-7	18-3	18-11	19-6	20-1	20-8	21-3	21-10	22-4	22-10	235	23-11			
	16.0	8-0	9-3	10-4	11-4	12-3	13-1	13-10	14-8	14-0	14-8	15-3	15-10	16-4	16-11	17-5	17-11	18-5	18-11	19-4	1910	20-3	20-8	21-1	21-6	21-11
2x10	19.2	7-4	8-5	9-5	10-4	11-2	11-11	12-8	13-4	129	13-4	13-11	14-5	14-11	15-5	15-11	16-4	16-10	17-3	17-8	<u>18-1</u>	18-6	18-11	19-3	19-8	20-0
	24.0	6-6	7-7	8-5	9-3	10-0	10-8	11-4	н-п	11-5	11-11	12-5	12-11	13-4	13-9	14-3	14-8	15-0	15-5	15-10	16-2	16-6	16-1	17-3	17-7	17-11
																1	- · ·		1	[1		1.		
	12.0	113	13-0	14-6	15-11	17-2	18-4	19-6	20-6	198	206	21-5	222	23-0	23-9	24-5	25-2	25-10			- · · ·			<u> </u>		
	16.0	9-9	11-3	12-7	13-9	14-11	15-11	16-10	17-9	17-0	17-9	18-6	19-3	19-11	20-6	21-2	21-9	22-5	23-0	23-6	24-1	24-8	25-2	25-8	1	
2x12	19.2	8-11	10-3	11-6	12-7	13-7	14-6	15-5	16-3	15-7	16-3	16-11	17-6	18-2	18-9	19-4	19-11	20-5	21-0	21-6	22-0	22-6	23-0	23-5	23-11	24-4
	24.0	7-11	9-2	10-3	11-3	12-2	13-0	13-9	14-6	13-11	14-6	15-1	15-8	16-3	16-9	17-3	17-9	18-3	18-9	19-3	19-8	20-1	20-6	21-0	21-5	21-9
		1	1		1		1								1	1	1		1	1	1	1			1	1
Е	12.0	0.11	0.17	0.23	0.31	0.38	0.47	0.56	0.66	0.77	0.88	0.99	1.10	1.22	1.35	1.48	1.61	1.75	1.89	2.03	2.18	2.33	2.48	1		
E	16.0	0.09	0.14	0.20	0.26	0.33	0.41	0.49	0.57	0.67	0.76	0.86	0,96	1,06	1.17	1.28	1.39	1.51	1.63	1.76	1.88	2.01	2.15	2.28	2.42	2.56
E	19.2	0.09	0.13	0.18	0.24	0.30	0.37	0.44	0.52	0.61	0.69	0.78	0.87	0.97	1.07	1.17	1.27	1.38	1.49	1.60	1.72	1.84	1.96	2.08	2.21	2.34
Е	24.0	0.08	0.12	0.16	0.22	0.27	0.33	0.40	0.46	0.54	0.62	0.70	0.78	0.87	0.95	1.04	1.14	1.23	1.33	1.43	1.54	1.64	1.75	1.86	1.98	2.09

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

TABLE R-11 **RAFTERS WITH L/240 DEFLECTION LIMITATION**

DESIGN CRITERIA:

Register February, 1999 No 518

Strength – Live Load of 40 psf plus Dead Load of 20 psf determines the required bending design value. Deflection - For 40 psf live load. Limited to span in inches divided by 240.

Size	Spacing																									
(in)	(in)											Raft	er Bendir	g Design	Value, F _b ,	(psi)										
	1. A.	300	400	500	600	700	800	900	1000	1100	1200	1300	14(8)	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700
										·									<u> </u>	 					2000	2700
	12.0	5-()	5-10	6-6	7-1	7-8	82	8-8	9–2	10-6	11-0	11-5	11-11	12-4	12-8	13-1	13-6	13-10	14-2	14-7	14-11	15-3	15-7	15-11	ł	- I
- 80 - 80 ¹	-16.0	4-4	5-0	5-7	6-2	6-8	7-1	7-6	7-11	9-1	9-6	9-11	10-3	10~8	-11-0	11-4	11-8	12-0	12-4	12-7	12-11	13-2	13-6	13-9	14-0	14-3
2x6	19.2	4-0	4-7	5-1	5-7	6-1	6-6	6-10	7-3	8~4	88	9-1	9-5	99	10-0	10-4	10-8	10-11	11-3	11-6	11-9	12-0	12-4	12-7	12-10	13-1
	24.0	3-7	4-1	4-7	5-0	5-5	5-10	6-2	6-6	7-5	7-9	8-1	8-5	8-8	90	9-3	9-6	9-9	10-0	10-3	10-6	10-9	11-0	11-3	11-5	11-8
1		- 						1												1						
	12.0	6-7	7-8	8-7	9-4	10-1	.10-10	11-6	121	13-11	14-6	15-1	15-8	16-3	16-9	17-3	17-9	18-3	18-9	19-2	19-8	201	20-6	20-11		
	16.0	5-9	6-7	7-5	8-1	8-9	9-4	9-11	10-6	12-0	12-7	13-1	137	14-0	14-6	14-11	15-5	15-10	16-3	16-7	17-0	17-5	17-9	181	18-6	18-10
2x8	19.2	5-3	6-0	6-9	7-5	8-0	8-7	9-1	9-7	11-0	11-6	11-11	12-5	12-10	13-3	13-8	14-0	14-5	14-10	15-2	15-6	15-10	16-3	16-7	16-10	17-2
	24.0	48	55	6-0	6-7	7-2	7-9	8-1	8-7	9-10	10-3	10-8	11-1	11-6	11-10	12-2	12-7	12-11	13-3	13-7	13-11	14-2	14-6	14-10	15-1	15-5
						:	1						1		10.00		-			<u> </u>						
	12.0	8-5	9_9	10-11	11-11	12-11	13-9	14-8	15-5	17-9	18-6	19-3	.20-0	20-8	21-4	22-0	22-8	23-3	23-11	24-6	25-1	25-7				
	16.0	7-4	8-5	9-5	10-4	11-2	11-11	12-8	13-4	15-4	16-0	16-8	17-4	17-11	18-6	19-1	19-7	20-2	20-8	21-2	21-8	22-2	22-8	23-1	23-7	24.0
2x10	19.2	6-8	7-8	87	9-5	10-2	10-11	11-7	12-2	14-0	14-8	:15-3	15-10	16-4	16-11	17-5	17-11	18-5	18-11	19-4	19-10	20-3	20-8	21-1	21-6	21 11
	24.0	6~0	6-11	7-8	8-5	9-1	9-9	10-4	10-11	12-6	13-1	137	14-2	14-8	15-1	15-7	16-0	16-6	16-11	17-4	17-9	18-1	18-6	18-11	19_3	10_7
											· · · · ·			<u>.</u>			1									10=1
	12.0	10-3	11-10	13-3	14-6	15-8	16-9	17-9	18-9	217	22-6	23-5	24-4	25-2	26-0				2. 						ļ	
	16.0	8-11	10-3	11-6	12-7	13-7	14-6	15-5	16-3	18-8	19-6	20-3	21-1	21-9	22-6	23-2	23-10	24-6	25-2	25-9	<u> </u>				 	
2x12	19.2	8-1	9-4	10-6	11-6	12-5	13-3	14-1	14-10	17-0	17-9	18-6	19-3	19-11	20-6	21-2	21-9	22-5	23-0	23-6	24-1	25-2	25_8			
	24.0	7-3	85	9-4	10-3	11-1	11-10	12-7	13-3	15-3	15-11	16-7	17-2	17-9	18-4	18-11	19-6	20-0	20-6	21-1	21-7	22-0	23-6	23.0	23.5	22.10
							<u> </u>	<u> </u>				<u> </u>	<u> </u>	1					20 0			22 0	22-0	25-47	23-5	23-10
Е	12.0	0.11	0.17	0.24	0.31	0.39	0.48	0.57	0.67	0.76	0.86	0.97	1.09	1.21	1.33	1.46	1.59	1.72	1.86	2.00	214	2.20	2 44	2.60		
Е	16.0	0.09	0.15	0.20	0.27	0.34	0.41	0.49	0.58	0.66	0.75	0.84	0.94	1.05	1.15	1.26	1.37	1.49	1.61	1.73	1.86	1 99	2.17	2.00	2 30	253
E	19.2	0.09	0.13	0.19	0.24	0.31	0.38	0.45	0.53	0.60	0.68	0.77	0.86	0.95	1.05	1.15	1.25	1.36	1.47	1.58	1.70	1.81	193	2.05	2.57	231
Е	24.0	0.08	0.12	0.17	0.22	0.28	0.34	0.40	0.47	0.54	0.61	0.69	0.77	0.85	0.94	2.03	1.12	1.22	1.31	1.41	1.52	1.62	1.73	1.84	1.95	2.06
Note	The requ	ired m	odulus o	ofelasti	city, E,	in 1,000	0,000 pc	ounds pe	er square	inch is	shown	at the b	ottom o	f each ta	ble, is l	imited t	o 2.6 mi	llion ps	and les	s. and	is applic	able to	all lum	her size	s shown	Spans

WISCONSIN ADMINISTRATIVE CODE

are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'

TABLE R-14 RAFTERS WITH L/180 DEFLECTION LIMITATION

DESIGN CRITERIA:

Register, February, 1999, No.:

Strength – Live Load of 30 psf plus Dead Load of 10 psf determines the required bending design value. Deflection – For 30 psf live load. Limited to span in inches divided by 180.

Size	Spacing																													
(in)	(in)													Raf	ter Bendi	ng Design	Value, F _b ,	(psi)												
		200	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700	2800	2900	3000
						1										[<u> </u>	<u> </u>				
	12.0	3-2	3-11	4-6	5-1	5-6	6-0	6-5	6-9	7-2	7-6	7-10	82	8-5	8-9	9-0.	9-4	9-7	9-10	10-1	10-4	15-3	10-10	11-1		1	1		1	
	16.0	2-9	3-5	3-11	44	4-10	5-2	5-6	5-10	6-2	6-6	6-9	7-1	7-4	7-7	7-10	8-1	8-40	8-6	8-9	9-0	13-2	9-5	9-7	9-9	10-0	┠		┣	
2x4	19.2	2-6	3-1	3-7	4-0	4-4	4-9	5-1	5-4	5-8	3-11	6-2	6-5	6-8	6-11	7-2	7-4	7_7	7-9	8-0	8-2	12-0	8-7	8-9	8-11	9-1	9-3	9-5	<u> </u>	<u> </u>
	24.0	2~3	2-9	3-2	3-7	3-11	4-3	4-6	4-10	5-1	5-4	5-6	5-9	6-0	6-2	6-5	6-7	6-9	7-0	7-2	7-4	10-9	7-8	7-10	8-0	8-2	8-4	8-5	8-7	8-9
24					1.1													-			-						<u> </u>		<u> </u>	{
10	12.0	5-0	6-2	7-1	7-11	8-8	9-5	10-0	10-8	11.3	11-9	12-4	12-10	13.3	13_0	14-2	14-8	15-1	15-6	15-11	16-3	15.3	17_0	17.5		Ι.	1 1			
	16.0	4-4	5-4	6-2	6-10	7-6	8-2	8-8	9-3	9-9	10-2	10-8	11-1	11-6	11-11	12-4	12-8	13-1	13-5	13-9	14.1	13-2	12.0	15-1	15.4	13_0			┢	<u> </u>
2x6	19.2	4-0	4-10	5-7	6-3	6-10	7-5	7-11	8-5	8-11	9-4	9-9	10-1	10-6	10-10	11.3	11-7	11-11	12-3	12-7	12-10	12-0	13-6	13-9	14-0	14-4	14-7	14-10	┢	┼──┤
	24.0	3-7	4.4	3-0	5-7	6-2	6-8	7-1	7-6	7-11	8-4	8-8	9-1	9-5	9-9	10-0	10-4	10-8	10-11	11-3	11-6	10-9	12-0	12-4	12-7	12-10	13-1	13-3	13-6	13-9
												·				<u> </u>		_								<u> </u>				
	12.0	67	81.	04.	10.6	11.6	12.5	12.2	14.0	14.10	15.6	16.2	16 10	17.6	10 1	10 0	10.4	10.10	20.5	20.11	21.6	20.1	1			1		Į	1	1 1
	16.0	5_0	7_0	8-1	9_1	9_11	10.9	10-6	12_2	12-10	13-0	14_0	14-7	15-2-	15-8	16-3	19-4	17.3	17 8	18 1	21-3	17.5	10 5	2211	10.2	30 0			Ļ	
2x8	10.0	5-3	6-5	7-5	8-3	9-1	9_9	10-6	11-1	11-8	12-3	12-10	13_4	13-10	13-0	14-10	10-7	15-8	17-3	16-7	10-7	15-10	17-0	18-1	18.6	18 10	10 3	16.7	<u> </u>	
	24.0	4-8	5-9	6-7	7.5	8-1	8-9	9.4	9-11	10-6	11-0	11-6		12-5	12-10	13-3	13_8	14_0	14-5	14-10	15.2	14_2	15-10	16-1	16-7	16-10	12-2	17.6	17-10	+
		<u> </u>				• •		<u> </u>			<u></u>	<u> </u>											13=10	10-5	10-1	10-10	1.1-2	1.2	17-10	
																							1		1 .	1	ł	1	1.1	
	12.0	8-5	10-4	11-11	13-4	14-8	15-10	16-11	17-11	18-11	19-10	20-8	21-6	22-4	23-1	23-11	24-7	25-4	.26-0			25-7				<u> </u>		<u> </u>	1	
2.10	16.0	1-4	8-11	10-4	11-7	12-8	13-8	14-8	15-6	10-4	17-2	1/-11	18-8	19-4	20-0	20-8	21-4	21-11	22-6	23-1	23-8	22-2	24-10	25-4	25-10					
2310	23.0	6-0	3-2	9-3	10-7	10.7	12-0	13-4	14-2	14-11	13-8	14 0	17-0	17-8	16-3	18-11	19-0	20-0	20-7	21-1	21-8	20-3	22-8	23-1	23-1	24-1	24-0	25-0	I	
	24.0	0-0	7	o-J	3-3	10-4	11-2		12-6	13-4	14-0	14-8	13~.)	13-10	10-4	10-11	17-3	17-11	10-3	10-11	19-4	10-1	20,	20-8	21-1	21-0	21-11	22-4	22-9	23-1
		· ·						Į.					1	ŀ									÷ .							
E	12.0	0.06	0.11	0.17	0.24	0.32	0,40	0.49	0.59	0.69	0.79	0.91	1.02	1.14	1.27	1.39	1.53	1.66	1.80	1.95	2.10	2.29	2.40	2.56		L	L	I	L	
E	16.0	0.05	0,10	0.15	0.21	0.28	0.35	0.43	0.51	0.60	0.69	0.78	0.88	0.99	1.10	1.21	1.32	1.44	1.56	1.69	1.82	1.99	2.08	2.22	2.36	2.50				
E	19.2	0.05	0.09	0.14	0.19	0.25	0.32	0.39	0.47	0.54	0.63	0.72	0.81	0.90	1.00	1.10	1.21	1.32	1.43	1.54	1.66	1.78	1.90	2.03	2.15	2.28	2.42	2.55		
E	24.0	0.04	0.08	0.12	0.17	0.23	0.29	0.35	0.42	0.49	0.56	0.64	0.72	0.81	0.89	0.99	1.08	1.18	1.28	1.38	1.48	1.59	1.70	1.81	1.93	2.04	2.16	2.28	2.41	2.53

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'

 TABLE R-15

 RAFTERS WITH L/180 DEFLECTION LIMITATION

DESIGN CRITERIA:

Register, February, 1999, No. 518

Strength – Live Load of 40 psf plus Dead Load of 10 psf determines the required bending design value. Deflection – For 40 psf live load. Limited to span in inches divided by 180.

Size	Spacing																												· · · · ·	<u> </u>
(in)	(in)							1.1			· · ·			Raf	ter Bendir	ig Design	Value, F _b ,	(psi)												
		200	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	2500	2600	2700	2800	2900	3000
			1.																											·
	12.0	2-10	3-6	4-0	4-6	4-11	5-4	5.9	6-1	6-5	6-8 ·	7-0	7-3	7_7	7–10	-8i	8-4	8-7	8-10	9-i	9-3	9-6	9-8	9-11	10-1			1		
	16.0	2-6	3-0	3-6	3-11	4-3	4-8	4-11	3-3	5-6	5-10	6-1	6-4	6-7	6-9	7-0	7-3	7-5	7-8	7-10	8-0	8-2	8-5	8-7	8-9	8-11	9_1	<u> </u>		
2x4	19.2	23	2-9	3-2	3-7	3-11	4-3	4-6	4-10	5-1	5-4	5-6	5-9	6-0	6-2	6-5	6-7	6-9	7-0	72	7-4	7-6	7-8	7-10	8-0	8-2	8-4	8-5	8-7	⊢ −−1
	24.0	2-0	2-6	2-10	3-2	3-6	3-9	4-0	4-3	4-6	4-9	4-11	5-2	5-4	5-6	5-9	5-11	6-1	6-3	6-5	6-7	6-8	6-10	7-0	7-2	7-3	7-5	7-7	7-8	7-10
					1		1.00				4	÷	1				1. A. A.				1.1	·						1		
	12.0	4-6	5-6	6-4	7-1	7_9	8-5	9-0	9-6	10-0	10-6	11-0	11-5	11-11	12-4	128	13-1	13-6	13-10	14-2	14-7	14-11	15-3	15-7	15-11			i i	1.1	
	16.0	3-11	4-9	5-6	6-2	6-9	7-3	7-9	8-3	88	9_i	9-6	9-11	10-3	10-8	11-0	11-4	11-8	12-0	12-4	12-7	12-11	13-2	13-6	13-9	14-0	14-3			
2x6	19.2	3-7	4-4	5-0	3-7	6-2	6-8	7-1	7-6	7-11	8-4	8-8	9-1	9-5	9.9	10-0	10-4	10-8	10-11	11-3.	11-6	11-9	12-0	12-4	12-7	12-10	13-1	13-3	13-6	
	24.0	3-2	3-11	4-6	5-0	5-6	5-11	6-4	6-9	7-1	7-5	7-9	8-1	8-5	8-8	9-0	9=3	9-6	9-9	10-0	10-3	10-6	10-9	11-0	11-3	11-5	11-8	11-11	12-1	12-4
	12.0	5-11	7-3	8-4	9-4	10-3	11-1	1110	12-7	13-3	13-11	14-6	15-i	15-8	16-3	16-9	17-3	17-9	18-3	18-9	19-2	19-8	20-1	20-6	20-11		1.1	ŀ		
	16.0	5-2	6-3	7-3	8-1	8-11	9-7	10-3	10-10	11-6	12-0	12-7	13-1	13-7	14-0	14-6	14-11	15-5	15-10	16-3	16-7	17-0	17-5	17-9	18-1	18-6	18-10			
2x8	19.2	4-8	5-9	6-7	7-5	8-1	8-9	9-4	9-11	10-6	11-0	11-6	11-11	12-5	12-10	13-3	13-8	14-0	14-5	14-10	15-2	13-6	15-10	16-3	16-7	16-10	17-2	17-6	17-10	
	24.0	4-2	5-2 -	3-11	6-7	7-3	7-10	8-4	8-11	9-4	9-10	10-3	10-8	11-1	11-6	11-10	12-2	12-7	12-11	13-3	13-7	13-11	14-2	14-6	14-10	13-1	13-3	15-8	15-11	16-3
						r																								I
· ·	120	7-7	9-3	10-8	11-11	13-1	14.2	15-1	16-0	16-11	17-9	18-6	19-3	20-0	20-8	21-4	22_0	22_8	23-3	23-11	24-6	25-1	25-7				1.1			
	16.0	6-6	8-0	9-3	10-4	11-4	12-3	13-1	13-10	14-8	13-4	16-0	16-8	17-4	17-11	18-6	19-1	19-7	20-2	20-8	21-2	21-8	22-2	22-8	23-1	23-7	24-0			
2x10	19.2	6-0	7-4	8-5	9-5	10-4	11-2	11-11	12-8	13-4	14-0	14-8	15-3	15-10	16-4	16-11	17-5	17-11	18-5	18-11	19-4	19-10	20-3	20-8	21-1	21-6	21-11	22-4	22-9	
	24.0	3-4	6-6	7-7	8-5	9-3	10-0	10-8	71-4	11-11	12-6	13-1	13-7	14-2	14-8	15-1	15-7	16-0	16-6	16-11	17-4	17-9	18-1	18-6	18-11	19-3	19-7	20-0	20-4	20-8
		-			<u> </u>	<u>├</u> ──		1													<u> </u>							<u> </u>		I
F	12.0	0.06	011	0.17	0.23	0.31	0.18	0.47	0.56	0.66	0.76	0.86	0.97	1.09	1.21	1.13	146	1.50	1 72	1.86	200	214	2.24	2.44	260					
E	16.0	0.05	0.09	0.14	0.20	0.26	0.33	0.41	0.49	0.57	0.66	0.75	0.84	0.94	1.05	LIS	1.26	1.37	1.49	1.61	1.73	1.86	1.99	2.12	2.25	2.39	2.53			
E	19.2	0.03	0.09	0.13	0.18	0.24	0.30	0.37	0.44	0.52	0.60	0.68	0.77	0.86	0.95	1.05	1.15	1.25	1.36	1.47	1.58	1.70	1.81	1.93	2.05	2.18	2.31	2.43	2.57	
E	24.0	0.04	0.08	0.12	0.16	0.22	0.27	0.33	0.40	0,46	0.54	0.61	0.69	0.77	0.85	0.94	1.03	1.12	1.22	1.31	1.41	1.52	1.62	1.73	1.84	1.95	2.06	2.18	2.30	2.41
	6734	*				_		0.000			_												4							-

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'

TABLE R-22 RAFTERS WITH L/180 DEFLECTION LIMITATION

DESIGN CRITERIA:

Register, February, 1999, No.:

Strength – Live Load of 30 psf plus Dead Load of 20 psf determines the required bending design value. Deflection – For 30 psf live load. Limited to span in inches divided by 180.

Size	Spacing	1							-																					
(in)	(in)													Ra	fter Bendi	ng Design	Value, Fr.	(nsi)												
		200	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1 1700	1 1800	1 1900	1 2000	2100	1 2200	1 2300	1.2400	1 2500	1 2600	1 2200	1 1000	1 2000	
1.0					[1					<u> </u>	<u> </u>	t						2.00	2400	2300	2000	2100	2800	2900	.3000
	12.0	2-10	3-6	4-0	4-6	4-11	5-4	5-9	6-1	6-5	6-8	7-0	7-3	7-7	7-10	8-i	8-4	87	8-10	9-0	9-3	9-6	9-8	9-11	10-1	10-4	10-6	10-8	10-11	1.1
	16.0	2-6	3-0	3-6	3-11	4-3	4-8	4-11	5-3	5-6	5-10	6-1	6-4	6-7	6-9	7-0	7-3	7-5	7-8	7-10	8-0	8-2	8-5	8-7	8-9	8-11	9-1	9_3	9-5	19_7
ZX4	19.2	2-3	2-9	3-2	3-7	3-11	4-3	4-6	4-10	5-1	5-4	5-6	5-9	6-0	6-2	6-5	6-7	6-9	7-0	7-2	7-4	7-6	7-8	7-10	8-0	8-2	8-4	8.5	8-7	100
	24.0	2-0	2-6	2-10	3-2	3-6	3-9	4-0	4-3	4-6	4-9	4-11	5-2	5-4	5-6	5-9	5-11	6-1	6-3	6-5	6-?	6-8	6-10	7-0	7-2	7-3	7-5	7_7	1-7_8	7-10
			100				2.11							î —					1		<u> </u>			-			<u> </u>	<u> </u>		<u> </u>
	12.0	4-6	5-6	6-4	7-1	7-9,	8-5	9-0	9-6	10-0	10-6	11-0	11-5	11-11	12-4	12-8	13-1	13-6	13-10	14-2	14-7	14_11	15_3	167	1.5.1.	14.2	100			
	16.0	3-11	4-9	5-6	6-2	6-9	7-3	7-9	8-3	8-8	9-1	9-6	9-11	10-3 -	10-8	11-0	11-4	11-8	12-0	12-4	12.7	12-11	13.2	13-1	12 0	10-2	10-0	10-10	17-1	17-5
2x6	19.2	3-7	4-4	5-0	5-7	6-2	6-8	7-1	7-6	7-11	8-4	8-8	9-1	9-5	9_9	10-0	10-4	10-8	10-11	11-3	11-6	11-9	12-0	13-0	13-9	14-0	14-3	14-7	14-10	15-1
	24.0	3-2	3-11	4-6	5-0	5-6	5-11	6-4	6-9	7-1	7-5	7-9	8-1	8-5	8-8	9-0	9-3	9-6	9_9	10-0	10-3	10-6	10.9	11.0	11 2	12-10	11 0	13-3	13-0	13-9
								1	1				<u> </u>			1		l	<u> </u>			<u> </u>	<u> </u>		11-5	11-5	11-0	11-11	12-1	12-4
	12.0	S-11	7-3	8-4	9-4	10-3	11-1	11-10	12-7	13-3	13-11	14-6	15-i	15-8	46-3	16-9	17-3	17-9	18-3	18-9	19-2	19-8	20-1	20-6	20-11	21_4	21-0		22.6	
	16.0	5-2	6-3	7-3	8-1	8-11	9-7	10-3	10-10	11-6	12-0	12-7	13-1	137	14-0	14-6	14-11	15-5	13-10	16-3	16-7	17-0	17-5	17-9	18-1	18.6	18 10	10.2	10.6	10.10
2x8	19.2	4-8	5-9	6-7	7-5	8-1	8-9	9-4	9-11	10-6	11-0	11-6	11-11	12-5	12-10	13-3	13-8	14-0	14-5	14-10	15-2	15-6	15-10	16-3	16-7	16-10	10-10	17-2	12.10	19-10
	24.0	42	5-2	5-11	6-7	7-3	7-10	8-4	8-11	9-4	9-10	10-3	10-8	11-1-	11-6	11-10	12-2	12-7	12-11	13-3	13-7	13-11	14-2	14-6	14-10	15-1 -	15-5	15-8	17-10	10-1
									<u> </u>				<u> </u>			1			<u> </u>	<u>├</u> ──			<u> </u>		<u> </u>		1,5=5	1,0-0		10-3
	12.0	7-7	9-3	10-8	,n−n	13-1	14-2	15-1	16-0	16-11	179	18-6	19-3	20-0	208	21-4	22-0	22-8	23-3	23-11	24-6	25-1	25-7							1
	16,0	6-6	8-0	9-3	10-4	11-4	12-3	13-1	13-10	14-8	15-4	16-0	16-8	17-4	17-11	18-6	19-1	19-7	20-2	20-8	21-2	21-8	22-2	22-8	23-1	23-7	24-0	24-6	24-11	25-4
2x10	19.2	<u>6-0</u>	7-4	8-5	9-5	10-4	11-2	11-11	12-8	13-4	14-0	14-8	15-3	15-10	16-4	16-11	17-5	17-11	18-5	18-11	19-4	19-10	20-3	20-8	21-1	21-6	21-11	22-4	22-9	23-1
	24.0	5-4	6-6	7-7	8-5	9-3	10-0	10-8	11-4	11-11	12-6	13-1	13-7	14-2	14-8	13-1	15-7	16-0	16-6	16-11	17-4	17-9	18-1	18-6	18-11	19-3	19-7	20-0	20-4	20-8
													1			1			l	1	l	l	ł		t		<u> </u>		<u> </u>	
Е	12.0	0.04	0.08	0.12	0.17	0.23	0.29	0.35	0.42	0.49	0.57	0.65	0.73	0.82	0.91	1.00	1.09	1.19	1.29	1.39	1.50	1.61	1.72	1.83	1.95	2.07	2.19	2.31	2.43	256
Е	16.0	0.04	0.07	0.11	0.15	0.20	0.25	0.31	0.36	0.43	0.49	0.56	0.63	0.71	0.78	0.86	0.95	1.03	1.12	1.21	1.30	1.39	1.49	1.59	1.69	1.79	1.89	2.00	211	222
E	19.2	0.03	0.06	0.10	.0.14	0.18	0.23	0.28	0.33	0.39	0.45	0.51	0.58	0.65	0.72	0.79	0.86	0.94	1.02	1.10	1.19	1.27	1.36	1.45	1.54	1.63	1.73	183	192	203
E	24.0	0.03	0.06	0.09	0.12	0.16	0.20	0.25	0.30	0.35	0.40	0.46	0.52	0.58	0.64	0.71	0.77	0.84	0.91	1.99	1.06	1.14	1.22	1.30	1.38	1.46	1.55	163	1 22	1181

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

TABLE R-23 RAFTERS WITH L/180 DEFLECTION LIMITATION

DESIGN CRITERIA:

Register, February, 1999, No 518

Strength – Live Load of 40 psf plus Dead Load of 20 psf determines the required bending design value. Deflection – For 40 psf live load. Limited to span in inches divided by 180.

Size	Spacing															÷ .	**												<u> </u>	
(in)	(in)													Ra	fter Bendi	ng Design	Value, Fr.	(psi)												
		.200	300	400	500	600	700	800	900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	2400	1 2500	2600	2700	1.2800	2000	2000
								1		· · · · ·			1			<u> </u>							2,00	2100	2.500	2000	2100	2000	2900	
	12.0	2-7	3-2	3-8	4-1	4-6	1.11	5.3	5.6	5-10	61	6.5	<u>د ب</u>	4 11	1	1.6		7.10												1 1
	16.0	2-3	2-9	3-2	3-7	3-11	4-3	4.6	4.10	3.1	57	56	5 0	6-11	1-2	1-3	1-1	/-10	8-0	8-3	8-5	8-8	8-10	9-0	9-3	9-5	9-7	9-9	9-11	10-1
2x4	19.2	2-1	2-6	2-11	3-3	3-7	3-10	4-1	4-4	4-7	4.10	5-0	39	5.5	0-2	0-0	0-1	6-9	7-0	1-2	1-4	7-6	7-8	710	8-0	8-2	8-4	85	8-7	8-9
	24.0	1-10	2-3	2-7	2-11	3-2	3-5	3-8	3-11	4-1	4-10	4.6	4_8	7.0	3-0	5-10	0-0	0-2	6-4	6-6	6-8	6-10	7-0	7-2	7-3	7-5	7-7	7-9	7-10	8-0
		1997 - 1997 1997 - 1997 - 1997	100									<u> </u>		3-11	J	J.,	5-5	5-0	3-6	5-10	0-0	0-1	6-3	6-5	6-6	6-8	6-9	6-11	7-0	72
l .	120			1								1.0	1.1	12							1.1		1.1	· · · · ·						
	12.0	4-1	5-0	5-10	0-0	7-1	7-8	8-2	8-8	9-2	9-7	10-0	10-5	10-10	. 11–3	11-7	11-11	12-4	12-8	13-0	13-3	13-7	13-11	14-2	14-6	14-9	15-i	15-4	15-7	15-11
216	10.0	3-1	4-4	3-0	5-1	6-2	0-8	1~1	7-6	7-11	8-4	8-8	9-1	9-5	9_9	10-0	10-4	10-8	10-11	11-3	11-6	11-9	12-0	12-4	12-7	12-10	13-1	13-3	13-6	13-9
280	23.0	2-3	2.7	4-7	3-1	3-7	0-1	0-0	6-10	7-3	1-1	7-11	8-3	8-7	8-11	9-2	9-5	9-9	10-0	10-3	10-6	10-9	11-0	11-3	11-3	11-8	11-11	12-2	12-4	12-7
	24.0	2-11	3-7.	4-1	· · · ·	3-0	3-3	3-10	0-2	0-0	0-10	7-1	1-3	7-8	7-11	8-2	8-5	8-8	8-11	9-2	9-5	9-7	9-10	10-0	10-3	10-5	10-8	10-10	11-0	11-3
1.1	12.0	5-5	6-7	78	8-7	9-4	10-1	10-10	11-6	12-1	12-8	13-3	13-9	14-4	14-10	15-3	15-9	16-3	16-8	17-1	17-6	17-11	18-4	18-9	19-1	19-6	19-10	20-3	20-7	20-11
	16.0	4-8	5-9	6-7	7-5	8-1	8-9	9-4	9-11	10-6	11-0	11-6	11-11	125	12-10	13-3	13~8	14-0	145	14-10	15-2	15-6	15-10	16-3	16-7	16-10	17-2	17-6	17-10	18-1
2x8	19.2	4-3	5-3	6-0	6-9	7-5	8-0	8-7	9-1	9-7	10-0	10-6	10-11	11-4	118	12-1	12-5	12-10	13-2	13-6	13-10	14-2	14-6	14-10	15-1	15-5	15-8	16-0	16-3	16-7
	24.0	3-10	4-8	5-5	6-0	6-7	7-2	7-8	8-1	8-7	9-0	9-4	9-9	10-1	10-6	10-10	11-2	11-6	11-9	12-1	12-5	12-8	12-11	13-3	13-6	13-9	14-0	14-4	14-7	14-10
															[-				-		I	I
]	12.0	6-11	8-5	9-9	10-11	11-11	12-11	13-9	14-8	15-5	16-2	16-11	17-7	18-3	18-11	19-6	20-1	20-8	21-3	21-10	22_4	22-10	22.5	22.11	24.5	21.10	26.4	25.10	1	
	16.0	6-0	7-4	8-5	9-5	10-4	11-2	11-11-	12-8	13-4	14-0	14-8	15-3	15-10	16-4	16-11	17-5	17-11	18-5	18-11	19_4	19-10	20-3	20-8	24-5	24-10	23-24	23-10	33.0	
2x10	19.2	35	6-8	7-8	8-7	9-5	10-2	10-11	11-7	12-2	12-9	13-4	13-11	14-5	14-0	15-5	15-11	16-4	16-10	17-3	17-8	18-1	18-6	18-11	10.3	10-2	20.0	22-4	22-9	23-1
	24.0	4-11	6-0	6-11	7-8	8-5	9-1	9-9	10-4	10-11	11-5	11-11	12-3	12-11	13-4	13-9	14-3	14-8	15-0	15-5	15-10	16-2	16-6	16-11	17-3	17_7	12.11	10 2	10 9	10 11
																	····						10 0				17-11	16,	10-/	10-11
R	12.0	0.04	0.08	013	0.18	0.23	0.10	0.16	0.42	0.50	0.50		0.74	0.00			l	l	l										1	
E	16.0	0.04	0.07	011	015	0.20	0.29	0.30	0.43	0.30	0.58	0.00	0.14	0.83	0.92	1.01	1.11	1,21	1.31	1.41	1.52	1.63	1.74	1.86	1.98	2.10	2.22	2.34	2.47	2.60
E	19.2	0.04	0.06	0.10	0.14	0.18	023	0.28	0.37	0.40	0.30	0.37	0.04	0.12	0.80	0.88	0.96	1.05	1.13	1.22	1.32	1.41	1.51	1.61	1.71	1.82	1.92	2.03	2.14	2.25
E	24.0	0.03	0.06	0.09	0.13	0.16	0.21	025	0.30	0.10	0.40	0.52	0.59	0.05	0.75	0,80	0.88	0.95	1.04	1.12	1.20	1.29	1.38	1.47	1.56	1.66	1.75	1.85	1.95	2.05
لنسيب	(T)			1	<u> </u>				L	Louis	0.41	0.40	0.56	0.39	0.05	0.72	0.78	0.03	0.93	1.00	1.08	1.15	1.23	1.31	1.40	1.48	1.57	1.66	1.75	1.84

Note: The required modulus of elasticity, E, in 1,000,000 pounds per square inch is shown at the bottom of each table, is limited to 2.6 million psi and less, and is applicable to all lumber sizes shown. Spans are shown in feet-inches and are limited to 26' and less. Check sources of supply for availability of lumber in lengths greater than 20'.

DEPARTMENT OF COMMERCE

Design Values for Joists and Rafters These "Fb" values are for use where repetitive members are spaced not more than 24 inches. Values for surfaced dry or surfaced green lumber apply at 19% maximum moisture content in use.

		Design Value in 1	Bending, "Fb"	· · ·	· · · ·
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Cottonwood	·····				
Select Structural		1510	1735	1,200,000	
No.1		1080	1240 .	1,200,000	
No.2		1080	1240	1,100,000]
No.3	2x4	605	695	1,000,000	
Stud		600	690	1,000,000	
Construction		805	925	1,000,000	
Standard		460	530	900,000	
Utility		200	230	900,000	
Select Structural		1310	1505	1,200,000	
No.1		935	1075	1,200,000]
No.2	2x6	935	1075	1,100,000	
No.3		525	600	1,000,000]
Stud		545	630	1,000,000]
Select Structural		1210	1390	1,200,000	NSLB
No 1	2x8	865	990	1,200,000	1
No.2		865	990	1,100,000	
No.3		485	555	1,000,000	
Select Structural		1105	1275	1,200,000	
No.1	2x10	790	910	1,200,000	
No.2		790	910	1,100,000	
No 3		445	510	1,000,000	
Select Structural		1005	1155	1,200,000	
No 1	2xl2	720	825	1,200,000	
No.2		720	825	1,100,000	
No 3		405	465	1,000,000	
Douglas Fir-Larch	··	· · · · · · · · · · · · · · · · · · ·			
Select Structural		2500	2875	1,900,000	
No 1 & Btr		1985	2280	1,800,000	
No.1		1725	1985	1,700,000	
No.2		1510	1735	1,600,000	
No.3	2x4	865	990	1,400,000	
Stud		855	980	1,400,000	
Construction		1150	1325	1,500,000	
Standard		635	725	1.400.000	
Utility		315	365	1,300,000	
Select Structural		2170	2495	1.900.000	
No.1 & Btr		1720	1975	1.800.000	
No.1	2x6	1495	1720	1.700.000	
No.2		1310	1505	1,600,000	
No.3		750	860	1,400,000	а.
Stud		775	895	1,400,000	
Select Structural		2000	2300	1,900,000	WCLIB
No 1 & Str	—	1585	1825	1.800.000	WWPA
No 1	2x8	1380	1585	1,700,000	
No 2		1210	1390	1,600,000	
No.3		690	795	1,000,000	
Select Structural		1835	2110	1,400,000	
No.1 & Btr	-	1455	1675	1,500,000	
No.1	2x10	1265	1455	1,000,000	
No 2		1105	1975	1,700,000	
No.3		625	725	1,000,000	
Select Structural	··· ···· /·	1670	1.23	1,400,000	
No 1 & Btr		1225	1920	1,900,000	
No 1		1525	1320	1,800,000	
No 2		1005	1323	1,700,000	
No 3		575	1155	1,600,000	
		3/3	000	1,400,000	

		Design Value in	Bending, "Fb"	· · · · · · · · · · · · · · · · · · ·	1
			~ ~ ~		Grading Rul
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Agency
Soloot Structural		0045	0.500	1 000 000	7
No. L Mo.2		2245	2580	1,900,000	
No.17No.2		1425	1635	1,600,000	
110.3		820	940	1,400,000	
Stud	2x4	820	945	1,400,000	
Construction		1095	1255	1,500,000	
Standard		605	695	1,400,000	
Ounty		290	330	1,300,000	
Select Structural		1945	2235	1,900,000	
No.1 /NO.2	2x6	1235	1420	1,600,000	
IN0.3	<u> </u>	710	815	1,400,000	
Stud		750	860	1,400,000	NLGA
Select Structural	<u></u>	1795	2065	1,900,000	
No.1 /No.2	2x8	1140	1310	1,600,000	
No.3		655	755	1,400,000	
Select Structural		1645	1890	1,900,000	
No 1 /No-2	2x10	1045	1200	1,600,000	
No.3		600	690	1,400,000	
Select Structural		1495	1720	1,900,000	
No.1 /No.2	2xl2	950	1090	1,600,000	
No.3		545	630	1,400,000	
Douglas Fir-South					
Select Structural		2245	2580	1,400,000	
No 1		1555	1785	1,300,000	
No.2		1425	1635	1,200,000	
No.3	2x4	820	940	1,100,000	
Stud		820	945	1,100,000	· · · · · · · · · · · · · · · · · · ·
Construction		1065	1225	1,200,000	
Standard		605	695	1,100,000	
Utility		290	330	1,000,000	
Select Structural		1945	2235	1,400,000	
No 1		1345	1545	1,300,000	
No.2	2x6	1235	1420	1,200,000	
No.3		710	815	1,100,000	
Stud		750	860	1,100,000	WWPA
Select Structural		1795	2065	1,400,000	
No 1	2x8 .	1240	1430	1,300,000	
No.2		1140	1310	1,200,000	
No.3		655	755	1,100,000	
Select Structural		1645	1890	1,400,000	
No 1	2x10	1140	1310	1,300,000	
No.2		1045	1200	1,200,000	
No 3		600	690	1,100,000	
Select Structural		1495	1720	1,400,000	
No.1	2x12	1035	1190	1,300,000	
No 2		950	1090	1,200,000	
No.3		545	630	1,100,000	

DEPARTMENT OF COMMERCE

		Design Value in	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Spow Loading	Madulus of Flastisity "F?	Grading Rules
Eastern Hemlock–Tamarack	5120	Normal Duration	Show Loading	Modulus of Elasticity E	Agency
Select Structural	······································	2155	2480	1 200 000	Г
No.1		1335	1535	1,200,000	-
No 2		990	1355	1,100,000	-
No.3		605	695	900,000	-
Stud		570	655	900,000	-
Construction		775	895	1 000,000	4
Standard		//30	405	1,000,000	4
Utility		200	230	800,000	-
Select Structural		1870	2150	1 200,000	4
No 1		1160	1220	1,200,000	
No 2	2x6	860	1550	1,100,000	
No 3		525	600	1,100,000	-
Stud		520	505	900,000	NITT NAA
Select Structural		1725	1005	900,000	NELVIA
No 1		1/25	1220	1,200,000	INSER
No 2		10/0	1230	1,100,000	4
No.2		/95	915	1,100,000	
Select Structurel	-	485	555	900,000	- ·
No.1		1580	1820	1,200,000	4 .
No.2		980	1125	1,100,000	
No.2		/25	835	1,100,000	
Foloot Structurel		445	510	900,000	
No.1		1440	1655	1,200,000	
No.2	2x12	890	1025	1,100,000	
No.2		660	760	1,100,000	
No.5		405	465	900,000	
Salaat Structural	······	0155	0400	1 000 000	1
No.1		2155	2480	1,200,000	
No.1		1333	1555	1,100,000	
No.2		990	1140	1,100,000	
Stud	2X4	605	695	900,000	
Construction	·	570	655	900,000	
Standard			895	1,000,000	
Standard Httility		430	495	900,000	
Select Structural		200	230	800,000	
		18/0	2150	1,200,000	
No.1		1160	1330	1,100,000	
No.2	2xo	860	990	1,100,000	· · · · · · · · · · · · · · · · · · ·
1N0.5	·	525	600	900,000	
Siud		520	595	900,000	NELMA
Select Structural		1725	1985	1,200,000	NSLB
No.1	2x8	10/0	1230	1,100,000	
NO 2		795	915	1,100,000	
N0.3		485	555	900,000	
		1580	1820	1,200,000	
NU.1	2x10	980	1125	1,100,000	
N0.2		725	835	1,100,000	
N0.5	-	445	510	900,000	
select Structural		1440	1655	1,200,000	
No.1	2xl2	890	1025	1,100,000	
NO.2		660	760	1,100,000	
No.3		405	.465	900,000	· · · · ·

Species and Grade		Design Value in Bending, "Fb"			()
	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rul Agency
Eastern White Pine					
Select Structural		2155	2480	1,200,000]
No.1		1335	1535	1,100,000	
No.2		990	1140	1,100,000	-
No.3	2x4	605	695	900,000	
Stud		570	655	900,000	
Construction	· · ·	775	895	1,000,000	
Standard		430	495	900,000	
Utility		200	230	800,000	
Select Structural		1870	2150	1,200,000	-
No.1		1160	1330	1,100,000	
No.2	2x6	860	990	1,100,000	1
No 3		525	600	900,000	1
Stud		520	595	900,000	NELMA
Select Structural		1725	1985	1.200.000	NSLB
No.1	2x8	1070	1230	1.100.000	
No.2		795	915	1 100 000	1
No 3		485	555	900 000	{
Select Structural		1520	1820	1 200,000	
No 1	2-10	1.300	1020	1,200,000	
No 2		700	1125	1,100,000	
No.2		125	655	1,100,000	· · ·
Select Structurel		445	510	900,000	
Select Structural	0.10	1440	1655	1,200,000	
No.1	2xi2	890	1025	1,100,000	
No 2		660	760	1,100,000	
No.3		405	465	900,000	
Hem Fir		· · · · · · · · · · · · · · · · · · ·			
Select Structural		2415	2775	1,600,000	No. 1
No 1 & Btr		1810	2085	1,500,000	
No 1	2	1640	1885	1,500,000	
No.2		1465	1685	1,300,000	
No.3	2x4	865	990	1,200,000	
Stud	· · · ·	855	980	1,200,000	
Construction		1120	1290	1,300,000	
Standard		635	725	1,200,000	·
Utility		290	330	1,100,000	
Select Structural		2095	2405	1,600,000	
No 1 & Btr		1570	1805	1,500,000	
No 1	2x6	1420	1635	1,500,000	
No 2		1270	1460	1,300,000	
No 3		750	860	1.200.000	
Stud		775	895	1.200.000	
Select Structural		1930	2220	1,600,000	WCLIB
No 1 & Btr		1450	1665	1 500 000	WWPA
Nol		1450	1510	1,500,000	** ******
No 2		1310	1250	1,500,000	
No 3	[·]		1550	1,000,000	
Salaat Structurel		070		1,200,000	
		1770	2035	1,000,000	
		1.530	15/25	1,500,000	
INO I	2x10	1200	1380	1,500,000	
No.2	[1075	1235	1,300,000	
No.3		635	725	1,200,000	
Select Structural		1610	1850	1,600,000	
No.1 & Btr		1210	1390	1,500,000	(
No 1	2x12	1095	1255	1,500,000	
No 2		980	1125	1,300,000	1 m 1
No.3		575	660	1,200,000	
		Design Value in	Bending, "Fb"		1
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Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "F"	Grading Rules
Hem-Fir (North)			g		ingeney
Select Structural		2245	2580	1.700.000	7
No.1 /No.2		1725	1985	1.600.000	
No.3		990	1140	1.400.000	
Stud	2x4	980	1125	1,400,000	-
Construction		1325	1520	1.500.000	4
Standard		720	825	1.400.000	-
Utility		345	395	1,300,000	
Select Structural		1945	2235	1.700.000	-
No.1 /No.2	2x6	1495	1720	1,600,000	
No.3		860	990	1,400,000	1
Stud		890	1025	1,400,000	NLGA
Select Structural		1795	2065	1.700.000	
No.1 /No.2	2x8	1380	1585	1.600.000	
No.3		795	915	1.400.000	1
Select Structural		1645	1890	1,700,000	
No.1 /No.2	2xl0	1265	1455	1,600,000	
No.3	· · · · · · · · · · · · · · · · · · ·	725	835	1,400,000	
Select Structural		1495	1720	1,700,000	
No.1 /No.2	2x12	1150	1325	1,600,000	
No.3		660	760	1.400.000	
Mixed Maple		I			
Select Structural		1725	1985	1,300,000	
No.1		1250	1440	1,200,000	
No.2		1210	1390	1,100,000	
No.3	2x4	690	795	1,000.000	
Stud		695	Boo	1,000,000	
Construction		920	1060	1,100,000	
Standard		520	595	1,000,000	
Utility		260	300	900,000	
Select Structural		1495	1720	1,300,000	
No 1		1085	1245	1,200,000	
No.2	2x6	1045	1205	1,100,000	
No.3		600	690	1,000,000	
Stud		635	725	1,000,000	NELMA
Select Structural		1380	1585	1,300,000	
No.1	2x8	1000	1150	1,200,000	
No.2		965	1110	1,100,000	
No.3		550	635	1,000,000	
Select Structural		1265	1455	1,300,000	1
No.1	2x10	915	1055	1,200,000	
No.2		885	1020	1,100,000	
No.3		505	580	1,000,000	
Select Structural		1150	1325	1,300,000	
No.1	2xl2	835	960	1,200,000	-
No.2		805	925	1,100,000	
No.3		460	530	1,000,000	and the second second

		Design Value in	Bending, "Fb"		1 7
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rule Agency
Mixed Oak					-
Select Structural		1985	2280	1,100,000	
No 1		1425	1635	1,000,000	
No.2		1380	1585	900,000	
No.3	2x4	820	940	800,000	1
Stud		790	910	800,000	
Construction		.1065	1225	900,000	
Standard		605	695	800,000	
Utility		290	330	800,000	
Select Structural		1720	1975	1,100,000	
No 1		1235	1420	1,000,000	
No.2	2x6	1195	1375	900,000	1
No.3		710	815	800,000	
Stud		720	825	800,000	NELMA
Select Structural		1585	1825	1,100,000	i
No.1	2x8	1140	1310	1,000.000	1
No 2		1105	1270	900.000	
No.3		655	755	800,000	1
Select Structural		1455	1675	1,100,000	1
No.1	2x10	1045	1200	1.000.000	• .
No 2		1010	1165	900.000	
No.3		600	690	800.000	
Select Structural		1325	1520	1 100 000	
No 1	2x12	950	1090	1,000,000	
No 2		920	1050	900,000	
No 3		545	630	800.000	
Mixed Southern Pine		<u></u>	050		<u> </u>
Select Structural	· · · · · · · · · · · · · · · · · · ·	2360	2710	1 600 000	1 (
No 1		1670	1920	1,500,000	
No 2		1500	1720	1 400 000	
No 3	2x4	865	990	1 200 000	
Stud		800	1020	1,200,000	
Construction		1150	1320	1 300 000	
Standard		635	725	1,300,000	
Itility	·	315	365	1,200,000	
Select Structural		2130	2450	1,100,000	
No I	{	1490	1720	1,000,000	
No 2	2×6	1320	1720	1,500,000	
No 3		775	805	1,400,000	
Stud		775	805	1,200,000	CDID
Salact Structural		, 773	2210	1,200,000	SFIB
No 1		1280	1500	1,500,000	
No 2	220	1380	1390	1,500,000	
No.2	<u> </u>	720	1390	1,400,000	
NO.5 Salaat Structural		1720	823	1,200,000	
		1/30	1980	1,000,000	
	^{2XIO}	1210	1390	1,500,000	
NU.2		1060	1220	1,400,000	
NU.D	·····	605	695	1,200,000	
select Structural		1610	1850	1,600,000	
N0.1	2x12	1120	1290	1,500,000	
No.2		1010	1160	1,400,000	
No.3		575	660	1,200,000	

		Design Value in	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Snow Londing	Madulus of Floatisity (F?	Grading Rules
Northern Red Oak		Itorinal Duration	Show Loading	Modulus of Elasticity 'E'	Agency
Select Structural		2415	2775	1 400 000	
No 1		1725	1985	1,400,000	-
No 2		1680	1905	1,400,000	4
No.3	2x4	950	1090	1,300,000	4
Stud		950	1090	1,200,000	4
Construction		1265	1455	1,200,000	-
Standard		720	825	1,200,000	-
Utility		345	305	1,100,000	
Select Structural		2095	2405	1,000,000	-
No 1		1495	1720	1,400,000	-
No 2		1450	1720	1,400,000	-
No 3		820	945	1,300,000	-
Stud		865	000	1,200,000	
Select Structural		1930	2220	1,200,000	
No 1	2x8	1380	1585	1,400,000	- · ·
No 2		1345	1585	1,400,000	-
No 3		760	075	1,500,000	4
Select Structural		1770	2025	1,200,000	
No 1	2×10	1770	2033	1,400,000	4
No 2		1205	1455	1,400,000	
No 3		695	1420	1,300,000	
Select Structural		1610	1850	1,200,000	
Nol		1150	1325	1,400,000	
No 2		1130	1323	1,400,000	
No 3		635	725	1,500,000	
Northern Species			125	1,200,000	<u> </u>
Select Structural		1640	1885	1 100 000	1
No.1 /No.2		990	1140	1,100,000	
No.3		605	695	1,100,000	
Stud	2x4	570	655	1,000,000	
Construction		775	895	1,000,000	
Standard		430	495	900,000	
Utility		200	230	900,000	
Select Structural		1420	1635	1 100 000	
No. 1 / No.2		860	990	1,100,000	
No.3		525	600	1,100,000	1997) 1997)
Stud		520	595	1,000,000	NI GA
Select Structural		1310	1510	1,000,000	NEOA
No.1/No 2	2x8	795	915	1,100,000	
No 3		485		1,100,000	
Select Structural		1200	1380	1 100 000	
No.1 /No.2	2x10	725		1,100,000	
No.3		445	510	1,100,000	
Select Structural		1095	1255	1,000,000	
No 1 /No 2	2x12	660	760	1,100,000	
No.3		405	465	1,100,000	
				1,000,000	

		Design Value in	Bending, "Fb"		T
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Northern White Cedar					
Select Structural		1335	1535	800,000]
No 1		990	1140	700,000	
No.2		950	1090	700,000	
No.3	2x4	560	645	600,000	1
Stud		540	620	600,000	1
Construction		720	825	700,000	
Standard		405	465	600,000	
Utility		200	230	600,000	1
Select Structural		1160	1330	800,000	1
No 1		860	990	700,000	1
No.2	2x6	820	945	700.000	-
No.3		485	560	600,000	1
Stud		490	560	600,000	NELMA
Select Structural		1070	1230	800,000	
No.1	2x8	795	915	700.000	
No 2		750	875	700,000	- ·
No 3		150	515	600,000	
Select Structural		430	1125	800,000	-
Nol	2210	700	925	300,000	-
No 2	2XIU	123	833	700,000	4
No.2		093	800	/00,000	
No.5		410	4/5	600,000	
Select Structural		890	1025	800,000	
No.1	2X12	660	760	700,000	
No.2		6.35	/25	700,000	
No.3	<u> </u>	3/5	430	600,000	L
Red Maple			2.700		, (
Select Structural		2245	2580	1,700,000	1
No.1	· · ·	1595	1835	1,600,000	
INO.2		1555	1785	1,500,000	
No.3	2x4	905	1040	1,300,000	
Stud		885	1020	1,300,000	
Construction		1210	1390	1,400,000	
Standard		660	760	1,300,000	
Utility		315	365	1,200,000	
Select Structural		1945	2235	1,700,000	
No.1		1385	1590	1,600,000	-
No 2	2x6	1345	1545	1,500,000	
No.3		785	905	1,300,000	
Stud		805	925	1,300,000	NELMA
Select Structural		1795	2065	1,700,000	
No.1	2x8	1275	1470	1,600,000	
No.2		1240	1430	1,500,000	
No.3		725	835	1,300,000	. •
Select Structural		1645	1890	1.700.000	
No 1	2x10	1170	1345	1.600.000	
No.2		1140	1310	1.500.000	
No.3		665	765	1 300 000	
Select Structural		1495	1720	1 700 000	
No.1		1065	1720	1,700,000	
No 2		1005	1100	1,000,000	
No 3		<u> </u>	£05	1,000,000	
	1	0000	0731	1.500.000	

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Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Red Oak		· · · · · · · · · · · · · · · · · · ·		L en,	
Select Structural		1985	. 2280	1,400,000	1
No.1		1425	1635	1,300,000	
No.2		1380	1585	1,200,000	4
No.3	2x4	820	940	1,100,000	
Stud		790	910	1,100,000	-
Construction		1065	1225	1,200,000	
Standard		605	695	1,100,000	
Utility		290	330	1,000,000	
Select Structural		1720	1975	1,400,000	
No 1		1235	1420	1,300,000	
No.2	2x6	1195	1375	1,200,000	
No.3	-	710	815	1,100,000	
Stud		720	825	1,100,000	NELMA
Select Structural		1585	1825	1,400,000	
No.1	2x8	1140	1310	1,300,000	
No 2		1105	1270	1,200,000	
No.3		655	755	1,100,000	
Select Structural		1455	1675	1,400,000	
No.1	2x10	1045	1200	1,300,000	
No 2		1010	1165	1,200,000	
No.3		600	690	1,100,000	
Select Structural		1325	1520	1,400,000	
No.1	2xl2	950	1090	1,300,000	1
No.2		920	1060	1,200,000	
No.3		545	630	1,100,000	

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		Design value in Bending, "Fb"		1	\\
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Redwood	······································		v		
Clear Structural		3020	3470	1,400,000	7
Select Structural		2330	2680	1,400,000	1
Select Structural, open grain		1900	2180	1,100,000	
No.1		1680	1935	1,300,000	- · ·
No 1, open grain		1335	1535	1,100,000	
No.2		1595	1835	1,200,000	
No.2, open grain	2x4	1250	1440	1,000,000	1
No.3		905	1040	1,100,000	
No.3, open grain		735	845	900,000	4
Stud		725	835	900.000	
Construction		950	1090	900.000	1
Standard		520	595	900.000	-
Utility		260	300	800.000	-
Clear Structural		2615	3010	1,400,000	
Select Structural	<u>.</u>	2020	2320	1.400.000	
Select Structural, open grain		1645	1890	1,100,000	-
No.1		1460	1675	1,300,000	
No.1. open grain	2x6	1160	1330	1 100 000	
No.2		1385	1590	1 200 000	
No.2. open grain		1085	1245	1,000,000	
No 3		785	905	1 100 000	
No.3. open grain		635	730	900.000	
Stud	[·]	660	760	900,000	
Clear Structural		2415	2775	1 400 000	
Select Structural		1865	2140	1 400 000	RIS
Select Structural, open grain		1520	1745	1,100,000	
No.1		1345	1545	1,300,000	`.
No.1, open grain	2x8	1070	1230	1,100,000	
No.2		1275	1470	1 200 000	
No.2, open grain		1000	1150	1.000.000	
No.3		725	835	1,100,000	
No 3, open grain		585	675	900,000	
Clear Structural		2215	2545	1 400 000	
Select Structural		1710	1965	1,400,000	
Select Structural, open grain		1390	1600	1,100,000	
No.1		1235	1420	1,300,000	
No.1, open grain	2x10	980	1125	1,100,000	
No 2		1170	1345	1,200,000	
No.2, open grain		915	1055	1,000,000	
No.3		665	765	1,100,000	
No 3, open grain		540	620	900.000	
Clear Structural		2015	2315	1,400,000	
Select Structural		1555	1785	1 400 000	
Select Structural, open grain		1265	1455	1,100,000	
No.1		1120	1290	1 300 000	
No.1, open grain	2x12	890	1025	1 100 000	
No.2		1065	1225	1 200 000	
No.2, open grain	}	835	960	1,000,000	
No.3	━ ŀ	605	695	1 100 000	
No.3, open grain		490	560	900 000	
			200	200,000	

		Design Value in	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Southern Pine	·	······································		· · · · · · · · · · · · · · · · · · ·	
Dense Select Structural		3510	4030	1,900,000	
Select Structural		3280	3770	1,800,000	1
Non-Dense Select Structural		3050	3500	1,700,000	1.
No 1 Dense		2300	2650	1,800,000	1
No.1		2130	2450	1,700,000	-
No.1 Non-Dense		1950	2250	1,600,000	-
No.2 Dense	2x4	1960	2250	1,700,000	
No 2		1720	1980	1,600,000	4
No 2 Non-Dense		1550	1790	1,400,000	
No.3		980	1120	1,400,000	-
Stud		1010	1160	1,400,000	-
Construction		1270	1450	1 500 000	-
Standard	-	720	825	1 300 000	- · ·
Utility		345	395	1 300 000	· ·
Dense Select Structural		3100	3570	1 900 000	
Select Structural		2930	3370	1,500,000	- · ·
Non-Dense Select Structural		2700	3110	1,000,000	
No.1 Dense		2010	2310	1,700,000	
No.1		1900	2180	1,300,000	
No 1 Non-Dense	2x6	1720	1980	1,700,000	
No 2 Dense		1670	1920	1,000,000	
No 2		1440	1520	1,700,000	
No.2 Non-Dense		1320	T520	1,000,000	
No 3		865	1920	1,400,000	
Stud	-	800	1020	1,400,000	
Dense Select Structural		2820	3240	1,400,000	
Select Structural	-	2620	3040	1,900,000	
Non-Dense Select Structural		2030	2780	1,800,000	CDID
No 1 Dense		1900	2180	1,700,000	SFID
No.1		1730	1080	1,800,000	
No.1 Non-Dense		1550	1700	1,700,000	
No 2 Dense		1610	1850	1,000,000	
No.2	·	1380	1590	1,700,000	
No 2 Non-Dense		1360	1350	1,000,000	
No 3		805	025	1,400,000	
Dense Select Structural		2470	2840	1,400,000	
Select Structural		2470	2040	1,900,000	
Non-Dense Select Structural		2,300	2/10	1,800,000	
No 1 Dense		1670	1020	1,700,000	
No 1	2×10	1500	1920	1,800,000	
No.1 Non-Dense	- 2/10	1380	1720	1,700,000	
No 2 Dense	- -	1380	1590	1,000,000	
No 2		1380	1390	1,700,000	
No 2 Non-Dense		1210	1390	1,600,000	
No 3	-	600	705	1,400,000	1
Dense Select Structural			795	1,400,000	
Select Structural	-1 -	2360	2/10	1,900,000	
Non-Dense Select Structure!	- -	2190	2510	1,800,000	
No 1 Dense	- -	2010	2310	1,700,000	· ·
No.1		1550	1790	1,800,000	
No.1 Non Deres	2x12	1440	1650	1,700,000	
No.2 Dance	- -	1320	1520	1,600,000	
No.2 Dense		1320	1520	1,700,000	
No.2 Non Deces	-l -	1120	1290	1,600,000	
No.2 Non-Dense	4 6	1040	1190	1,400,000	
INO.3	1	660	760	1,400,000	

-		Design Value in	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
Spruce-Pine-Fir					
Select Structural		2155	2480	1,500,000	7
No 1 /No 2		1510	1735	1,400,000	1
No.3		865	990	1,200,000	
Stud	2x4	855	980	1,200,000	-
Construction		1120	1290	1,300,000	-
Standard		635	725	1,200,000	
Utility		290	330	1,100,000	
Select Structural		1870	2150	1,500,000	-
No.1 /No.2	2x6	1310	1505	1,400,000	-
No.3		750	860	1,200,000	· ·
Stud		775	895	1,200,000	NLGA
Select Structural		1725	1985	1,500,000	-
No. 1 / No.2	2x8	1210	1390	1,400,000	1
No.3		690	795	1,200,000	-
Select Structural	2x10	1580	1820	1,500,000	1
No 1/No 2		1105	1275	1,400,000	
No.3		635	725	1,200,000	
Select Structural		1440	1655	1,500,000	
No.1 /No.2	2xl2	1005	1155	1,400,000	
No.3		575	660	1.200.000	
Spruce-Pine-Fir (South)	·····	1			1
Select Structural		2245	2580	1,300,000]
No.1	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	1465	1685	1,200,000	
No 2		1295	1490	1,100,000	
No.3	2x4	735	845	1,000,000	
Stud		725	835	1,000,000	\int
Construction		980	1125	1,000,000	
Standard		. 545	630	900,000	
Utility		260	300	900,000	
Select Structural		1945	2235	1,300,000	
No.1		1270	1460	1,200,000	
No.2	2x6	1120	1290	1,100,000	
No.3		635	730	1000,000	NELMA
Stud		660	760	1,000,000	NSLB
Select Structural		1795	2065	1,300,000	WCLIB
No 1	2x8	1175	1350	1,200:000	WWPA
No 2	······	1035	1190	1,100,000	
No.3		585	675	1,000,000	
Select Structural		1645	1890	1,300,000	
No.1	2x10	1075	1235	1,200,000	
No 2		950	1090	1,100.000	
No.3		540	620	1.000.000	
Select Structural		1495	1720	1.300.000	
No.1	2xl2	980	1125	1,200.000	
No.2		865	990	1.100.000	
No.3		490	560	1.000.000	
				.,,	

		Design Value in	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Snew Loading	Modulus of Elasticity "E"	Grading Rules Agency
Western Cedars			<u> </u>		
Select Structural		1725	1985	1,100,000	· ۲
No 1		1250	1440	1.000.000	
No 2		1210	1390	1.000.000	
No 3	2x4	690	795	900.000	· ·
Stud		695	800	900.000	
Construction		920	1060	900.000	
Standard		520	595	800,000	•
Utility		260	300	800.000	
Select Structural		1495	1720	1,100,000	
No 1		1085	1245	1.000.000	
No 2	2x6	1045	1205	1.000.000	
No 3		600	690	900.000	
Stud		635	725	900.000	WCLIB
Select Structural		1380	1585	1,100,000	WWPA
No I	2x8	1000	1150	1,000,000	
No.2		965	1110	1.000.000	1
No 3		550	635	900,000	
Select Structural		1265	1455	1.100.000	
No 1		915	1055	1,000,000	
No 2		885	1020	1,000,000	
No 3		505	580	900,000	
Select Structural		1150	1325	1 100 000	
No 1		835	960	1,000,000	
No 2		805	925	1,000,000	
No.3		460	530	900.000	
Western Woods	1				[
Select Structural		1510	1735	1.200.000	1
No I		1120	1290	1.100.000	
No 2		1120	1290	1.000.000	
No 3	2x4	645	745	900.000	
Stud		635	725	900.000	
Construction		835	960	1.000.000	
Standard		460	530	900,000	
Utility		230	265	800,000	
Select Structural		1310	1505	1.200.000	
No 1		970	1120	1,100,000	
No 2	2x6	970	1120	1,000,000	
No 3		560	645	900.000	
Stud	- I	575	660	900.000	WCLIB
Select Structural		1210	1390	1.200.000	WWPA
No.1	2x8	895	1030	1,100,000	
No.2		895	1030	1.000.000	
No.3		520	595	900.000	
Select Structural		110	1275	1,200,000	
No 1	2x10	820	945	1,100,000	
No.2		820	945	1.000.000	
No.3	<u> </u>	475	545	900.000	
Select Structural		1005	1155	1.200.000	
No I	2x12	750	860	1,100,000	
No.2		750	860	1.000.000	
No.3		430	495	900.000	

		Design Value in	Bending, "Fb"		
Species and Grade	Size	Normal Duration	Snow Loading	Modulus of Elasticity "E"	Grading Rules Agency
White Oak					·
Select Structural		2070	2380	1,100,000	
No.1		1510	1735	1,000,000	
No.2		1465	1685	900,000	
No.3	2x4	820	940	800,000	
Stud		820	945	800,000	
Construction		1095	1255	900,000	
Standard		605	695	800,000	
Utility		290	330	800,000	
Select Structural		1795	2065	1,100,000	
No 1		1310	1505	1,000,000	
INO.2	2x6	1270	1460	900,000	
No.3		710	815	800,000	
Stud		750	860	800,000	NELMA
Select Structural		1655	1905	1,100,000	
No.1	2x8	1210	1390	1,000,000	
No.2		1175	1350	900,000	
No.3		655	755	800,000	
Select Structural		1520	1745	1,100,000	
No.1	2x10	1105	1275	1,000,000	
No 2	·	1075	1235	900,000]
No.3		600	690	800,000	
Select Structural		1380	1585	1,100,000	
No.1	2x12	1005	1155	1,000,000	
No 2		980	1125	900,000	
NO.3		545	630	800,000	
Select Street	·····			rink - h-normalization (http://www.co.j.org/a	. (
Select Structural		1725	1985	1,500,000	
No.1		1250	1440	1,400,000	
NO Z		1210	1390	1,300,000	
NO.5	2x4	690	/95	1,200,000	
Construction		695	800	1,200,000	
Stondord		920	1060	1,300,000	
		520	595	1,100,000	
Select Structural		230	265	1,100,000	
No.1		1495	1720	1,500,000	
No.2		1055	1245	1,400,000	
No 2	2X0	1045	1205	1,300,000	1
Nu.5		600	690	1,200,000	
Silud Select Structurel		635	725	1,200,000	NSLB
No.1		1380	1585	1,500,000	
No 2	^{2x8}	1000	1150	1,400,000	
No.2		965	1110	1,300,000	
Salaat Structural		550	635	1,200,000	
No 1		1265	1455	1,500,000	
No.2	2XIU	C16	1055	1,400,000	
No.2		288	1020	1,300,000	
Select Structurel		505	580	1,200,000	
		1150	1325	1,500,000	
No 2	^{2x12}	835	960	1,400,000	
No.2		805	925	1,300,000	
1.0.0		460	530	1,200,000	

21.04(2)(a)5. HANDRAIL SHAPES



APPENDIX EROSION CONTROL PROCEDURES EXAMPLES, ILLUSTRATIONS AND GUIDELINES

The following examples and illustrations of some erosion control procedures are provided for your information. Many of these examples can be found in the "Wisconsin Construction Site Best Management Practices Handbook", developed by the Wisconsin department of natural resources. Note: The Handbook is available from Document sales, 202 South Thornton Avenue, P.O. Box 7840, Madison, WI 53707–8480; phone (608) 266–3358.

Figures E-1 to E-11, depict the materials and installation of some erosion control procedures.

Also included in the appendix are examples of plot plans depicting the best management practices that will help meet the requirements of the performance standards in this code.

Figure E – 12 is an example of a site with slopes of 12 % or less and also simple slopes, i.e. all slopes occurring in one general direction. Downslope measures are required, to reduce maintenance of these measures, the upslope diversion is recommended.

Figure E – 13 is an example of a site with complex slopes (slopes occurring in more than one direction). This site also has an area where slopes that are 12-20% are going to be disturbed. The location of the erosion control procedures are clearly indicated on the plot plan, including narratives that indicated methods of permanent stabilization.

Figure E - 14 is an example of a large lot, greater than 5 acres, with slopes greater than 12% and where the area of land disturbing activity is indicated. This plan indicates the use of vegetative barriers.

Figure E -15 explains how to determine and calculate % slopes.

Guidelines for timing the implementation of the erosion control practices and procedures in order to stabilize areas disturbed during construction of one and 2-family dwellings are included in this appendix. Dormant seeding, the guidelines for the use of vege-tative buffers and the recommended maintenance for erosion control practices are also included.

For sites using either straw bales or silt fences as a perimeter control, <u>Table E-1</u> is included as a guide for determining the distance between parallel fences constructed on various slopes. Perimeter measures should be installed at right angles to the direction of flow. Drainage area is to be no more than 1/4 acres (approx. 10,000 square feet) per 100 feet of perimeter control.

TABLE E-1 DISTANCE BETWEEN PARALLEL STRAW BALES OR SILT FENCES

			Slope	
	Slope		Distance	
	Percent		(feet)	
· · · · ·			L.	
	< 2%		100 feet	
	2 to 5%		75 feet	
	5 to 10%		50 feet	
	10 to 20%		25 feet	
	> 20 %		15 feet	i.

VEGETATIVE BARRIERS

Vegetative barriers may be used as a perimeter measure if disturbed areas above consist of slopes no greater than 6% and barriers are on a grade no steeper than 5%. Vegetative barriers are to be a minimum of 10' wide for every 50 feet of open ground draining to them. These barriers must be maintained, i.e. not driven on or destroyed. If the barriers become covered with silt or otherwise destroyed, additional perimeter measures may be required.

TEMPORARY STABILIZATION OR MULCH CROP

It is much easier to control erosion than to control sediment. Temporary stabilization helps to minimize erosion and therefore the need for long term maintenance of silt fences and straw bales. Annual rye grass may be planted as a temporary cover between April 1 and September 15. If seeding is done in the spring or late summer seeding dates and slopes are 6% or less, mulch may not be necessary.

Winter rye may be planted between July 15 and October 15. These seedings should be mulched.

LATE SEASON CONSTRUCTION MULCHING/DORMANT SEEDING

If ground is broken after September 15, mulch should be applied as soon as a rough grade is established, unless final grade and landscaping is to be completed before the next growing season. Mulch will help to reduce the raindrop impact. Seeding should not be done between September 15 and November 1 as the weather is warm enough for the seed to germinate but it will not have an opportunity to establish a root system strong enough to survive the winter. A dormant seeding may be done OVER the mulch after November 1. These seedings are risky. A split application of seed may also be made, using half in November and balance early in spring.

WINTER CONSTRUCTION

In areas with course soils, (sands) if excavation is possible most likely a trencher can be used to install the necessary silt fence. If at all possible leave the perimeter of the site undisturbed (this is assuming the site had vegetation present prior to frost); this may be the easiest erosion control for flat sites (6% or less).

In areas that have heavy soils, (clays) close attention should be paid to the try to get perimeter measures installed prior to frost penetrating greater than 6". If ground is solidly frozen, perimeter measures that need to be trenched may have to wait to be installed when the frost first starts to come out in the spring. This does not eliminate the need to keep sediment from leaving the site. Alternate methods for controlling erosion should be considered such as the use of soil stabilizers.

MAINTENANCE OF THE MOST COMMONLY USED EROSION CONTROL PROCE DURES

SILT FENCES

Repair or replacement should be done within 24 hours if fencing is torn, sagging, overtopped, blown over (laying down), shows a lack of integrity, or in any way is not functioning as designed. Sediment deposits should be removed after each storm event. Sediment deposits shall be removed when deposits reach 0.5 the above ground height of the fence. Silt fence should be removed after upland areas have been stabilized. Any sediment deposits remaining in place after the silt fence is no longer required should be dressed to conform to the existing grade, prepared and stabilized.

STRAWBALES

Replacement of broken or torn bales should be done within 24 hours. Sediment deposits should be removed when deposits reach 0.5 the height of the bales. Strawbales should be removed after upland areas have been stabilized. Any sediment deposits remaining in place after the strawbale barrier is no longer required should be dressed to conform to the existing grade, prepared and stabilized.

MULCHING

Additional mulch or matting should be applied when rills develop (rill - small, eroded ditch measuring 1" or less width).

TEMPORARY DIVERSION

Any breaks or eroded areas of a diversion should be repaired within 24 hours.

SEDIMENT TRAP

Any structural deficiencies should be repaired within 24 hours. Sediment should be removed when it reaches half of the outlet height of trap.

SODDING

Repair or replacement of sod that has been destroyed in an area of channelized flow should be done within 24 hours after the rain event.

INLET PROTECTION BARRIERS

Sediment deposits should be removed when deposits reach 0.5 the height of the fence. Repair or replacement should be made to damaged barriers within 24 hours.

TEMPORARY GRAVEL CONSTRUCTION ENTRANCE

Rock should be maintained to meet the design criteria of 2-3" aggregate stone; 12 feet wide and 50 feet long or the distance to the foundation, whichever is less; and maintained at a depth of 6". Filter fabric (geotextile) should be used as a separation barrier between the rock and soil if soils are mainly clay or silt.





- 1. THE STRAW BALES SHALL BE PLACED ON SLOPE CONTOUR WITH ENDS OF STRAW BALE FENCE TURNED UPSLOPE TO PREVENT FLANKING 2. BALES TO BE PLACED SO THAT BINDINGS ARE ORIENTED AROUND THE SIDES RATHER THAN ALONG THE TOPS AND BOTTOMS OF THE BALES.
- BALES TO BE PLACED IN A ROW WITH THE ENDS TIGHTLY ABUTTING 3.
- KEY IN BALES 4" INTO SOIL TO PREVENT EROSION OR FLOW UNDER BALES 4
- **FIG. E 2** STRAW BALE FENCE



Register, February, 1999, No 518

DEPARTMENT OF COMMERCE















ORIGINAL GROUND ELEVATION -- REMOVE EXISTING VEGETATION

PURPOSE

To divert runoff around disturbed areas to a location where the clean water can be discharged to existing vegetation in such a way as to prevent any negative offsite impacts.

CONDITIONS WHERE PRACTICE APPLIES

1. Where drainage areas do not exceed 3 acres.

2. Upslope of disturbed areas where erosion is likely to occur.

3. Upslope of soil piles.

4. Above steep cut or fill slopes.

STABILIZATION

Diversions side slopes, ridge, downslope side of the berm and channel should be stabilized within 7 days of final grading by:

1. Sodding;

2. seeding and mulching in combination with filter fabric barriers or straw bale barriers;

3. covering with suitable geotextile;

4. covering with 6 mil polyethylene sheeting. (vegetation should be used as the stabilization method if diversion is to be in place 30 days or longer)

FIG. E – 10 TEMPORARY DIVERSION

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Comm 20 APPENDIX





s. Comm 21.16 **Frost Protected Shallow Footings** In lieu of frost walls, the following is an acceptable method.

Minimum Ground Insulation Requirements (1)

		Mea Tempe	n Annua rature (2	11 2, 6)	Minimum (Footing Depth 7, 8)
Air Freezing Index (F- days) (3)	W-Insulation Width from Edge of Footing (4, 5)	38	40	≥41	D- Concrete Depth	G-Granular Base Thickness
2250 or less	63"	NA	NA	2.5"	10"	6"
2251 - 3000	79"	4"	3.5"	3.5"	10"	6"
3001 - 3750	91"	5"	NA	NA	10"	6"

Notes:

1. Recommendations are based on information found in "Design Guide for Frost-Protected shallow Foundations" prepared for the U. S. Department of Housing and Urban Development by NAHB Research Center (Instrument 3: DU100K000005987, dated June 1994

2. Units are degrees Fahrenheit. See estimate provided on Mean Annual Temperature Contour Map.

3. Air freezing index shall be based on maximum year expected for a 100 year return period. See estimate provided on AFI Contour Map.

4. Ground insulation to the building interior can be extended beneath the entire slab where it is desired to protect the entire slab from frost heave action.

5. Ground insulation to the building interior can be in one horizontal plane (as shown in the dctail) and covered with non frost-susceptible fill or the insulation maybe placed directly beneath the slab.

6 Insulation thickness recommendations are for extruded polystyrene (XPS) insulation.

7. The minimum depth of concrete footing and horizontal insulation is 10". A 6" drainage layer is required under the insulation.

8. Insulation placed directly beneath the footing shall be Type IV or Type VI XPS in accordance with ASTM C578. Maximum deadload placed on the Type IV insulation shall be 1200 pounds/square foot. Maximum deadload placed on Type VI shall be 1900 psf.





s. Comm 21.16 **Frost Protected Shallow Footings** In lieu of frost walls, the following is an acceptable method.

Minimum Ground Insulation Requirements (1)

Minimum Footing Depth Mean Annual (7.8)Temperature (2, 6) G-Granular ≥41 D-W-Insulation 38 40 Air Freezing Width from Concrete Base Index (F-Thickness days) (3) Edge of Footing Depth (4, 5) 10" 6' 2250 or less 63" NA NA 2.5" 10' 6' 79" 4" 3.5" 3.5" 2251 - 3000 91" 5" NA NA 10" 6' 3001 - 3750

Notes:

1. Recommendations are based on information found in "Design Guide for Frost-Protected shallow Foundations" prepared for the U. S. Department of Housing and Urban Development by NAHB Research Center (Instrument 3: DU100K000005987, dated June 1994

2. Units are degrees Fahrenheit. See estimate provided on Mean Annual Temperature Contour Map.

3. Air freezing index shall be based on maximum year expected for a 100 year return period. See estimate provided on AFI Contour Map.

4. Ground insulation to the building interior can be extended beneath the entire slab where it is desired to protect the entire slab from frost heave action.

5. Ground insulation to the building interior can be in one horizontal plane (as shown in the dctail) and covered with non frost-susceptible fill or the insulation maybe placed directly beneath the slab.

6. Insulation thickness recommendations are for extruded polystyrene (XPS) insulation.

7. The minimum depth of concrete footing and horizontal insulation is 10". A 6" drainage layer is required under the insulation.

8. Insulation placed directly beneath the footing shall be Type IV or Type VI XPS in accordance with ASTM C578. Maximum deadload placed on the Type IV insulation shall be 1200 pounds/square foot. Maximum deadload placed on Type VI shall be 1900 psf.









Air-Freeze Index Contour Map



Mean Annual Temperature Contour Map

UDC Energy Worksheet

The UDC Energy Worksheet is required to be submitted with building plans for plan review prior to issuance of a building permit. Following is a sample dwelling and completed Energy Worksheet and a blank worksheet after that. The sample completed worksheet has been completed for both the Prescriptive Package and System Design Methods for demonstration purposes. Normally only one method is required to be completed for showing code compliance.

Sample dwelling: Non-Electrically heated single-family dwelling located in Dane County (Zone 3). Has 1,500 square feet and 186 linear feet of perimeter building thermal envelope. Garage is not heated. Estimated infiltration rate is .3 air changes per hour. There will be 170 cfm of installed exhaust ventilation.

Gross Above-Foundation Walls:		
$Wall = 8.09' (97''-1/8'') \times 186$ linear feet = 1,504 sq	juare feet	
Box sill = 0.81 feet (9-3/4 inches deep: sill, header,	, subfloor) x 186 linear feet = 15	1 square feet
Wood 1 x 8-inch drop siding		R = 0.79
1-inch extruded polystyrene sheathing		R = 5
R13 hatt insulation		R = 13
2×4 froming 16 inches ΩC	and the second second	R = 4.4
2×4 framing, to increase 0.2 .		R = 0.56
172-men erywan minsh		
$\mathbf{D}_{\mathbf{r}} = \mathbf{r}_{\mathbf{r}} - 2\mathbf{r}_{\mathbf{r}}$		· · ·
Door area = 30 sq it		U = 0.35
insulated steel doors		0 - 0.25
XX/im Jourse		
Windows:		
Above-Foundation windows - 150 sq it	1/2" air space rated by NFR	C II = 0.35
wood, low-E, argon-illied, double-pane with	11/2 all space, fated by Refe	c
Foundation wall window area = 20 square leet		11 - 0 87
Operable metal w/o thermal break, double p	bane	0 = 0.07
Town Jesten Off Link 1 ft ownood	and the second	
Foundation - 8 it high, 1 it exposed	and the second	$\mathbf{R} = 0.8$
8-inch poured concrete		$\mathbf{R} = 5$
1-inch extruded polystyrene for full neight		R – J
	tood bool)	
Ceiling - 1,500 square feet, standard roof trusses (no ra	lised neel)	P 4 4
2 x 4 trusses, 24 inches O.C.	and the second	$\mathbf{K} = 4.4$
Blown fiberglass insulation		R/inch = 2.5
Insulation in cavity, 16 inches		K = 40
Insulation over framing, 12.5 inches		R = 31.25
5/8-inch drywall finish		R = 0.56
	[1] A. K.	

Heating Plant

Gas-Fired Hot Air, 90% AFUE

High Efficiency



Submit complete	d worksheet pages 3-6 w	ith dwelling plans to local enforcing municipality.
Project Address:	Sample - Zone 3	
Builder:		Owner:
Worksheet Completed By: Does dwelling unit have three kild	watts or more input capacity	Date: of permanently installed electrical space heating equipment? below) INO
You will need to apply the stricter	standards shown for electric	ally-heated homes if you answered "YES" to the above question.
A. Area Calculations Enter appropriate dimensions to ol method. These calculated areas ar	btain area values. Some calc e referenced elsewhere on th	ulations will not be necessary depending on home design or calculations worksheet, for example, "(A.1.)".
1. Window, Skylight & Patio Doc a. In Above- Foundation Walls	or Area (overall unit area) b. In Foundation Walls	2. Opaque Door Area a. In Above- Foundation Walls b. In Foundation Walls
$\frac{150}{\text{c. Total } (a. + b.)} = \frac{\text{sq. ft.}}{170}$	20 sq. ft.	$\frac{38}{\text{c. Total } (a. + b.)} = 38 \qquad 0 \qquad \text{sq. ft.}$
3. Gross Exposed Basement Wall .	Area	4. Basement Wall Area Below Grade
1'x 186'	196	7'x 186'
	sq. ft.	1302 sq.
5. Opaque [1] Basement Wall Area A.2.b.)	a (A.3. + A.4 A.1.b	6. Gross Heated Above-Foundation Wall Area, including boxsill
186 + 1302 - 20 - 0	•	1504 + 151
If the exposed area of A.3 is greater th	1468 sq. ft. an the below grade area of	1655 sq.
A.4., add A.5. to A.7 and cross out the 7. Above Foundation Code Wall A	number in this cell. rea (A.6. + A1.b. + A.2.b.)	8. Opaque [1] Above-Foundation Wall Area (A.6 A1.a A.2.a.)
1655 + 20 + 0	an ann an Stairt an Anna Anna Anna Anna 1919 - Anna Anna Anna Anna Anna Anna Anna 1919 - Anna Anna Anna Anna Anna Anna Anna An	1655 - 150 - 38
	1675 sq. ft.	1467 sq.
9. Floor Area Over Interior Uncone 50°	ditioned Spaces Less Than	 10. Insulated Roof Or Ceiling (less skylights) 28 x 45 = 1260 12 x 20 = 240
	0 sq. ft.	1500 sq.
11. Exterior Floor Area (Overhang	35) A Marian Parti I. A Antonio Atta	12. Crawl Space Wall Area
	0	0
13. Slab On Grade (above or less t	than 12 inches below grade)	14. Total Heated Envelope Area $(A.5 + A.7 + A.9 + A.10 + A.11 + A.12 + (A.13. X 2'))$
0		1468 + 1675 + 0 + 1500 + 0 + 0 + 0 4643
15 Percent Glazing (for Prescripti	ve Package Method.	sq. : 16. Windows Description - Above-Foundation Windows:
Section B, only) (A.1.c. $+$ A.7. X 170 $+$ 1675 x 100%	(100%)	Frame type: Wwood or Wood Clad Vinyl Meta Glazing type: WDual Triple Dual w/storm panel Dual-Glazing Air Space: 1/4' 3/8" W1/2" or mor
an a	10.2 %	Features: XLow-E XArgon-filled Suspended film Foundation Windows: Vinyl X Metal

.

B. Prescriptive Package Method (Skip this section if using the System Design Method of Sections C-F)

The prescriptive package method is the simplest method for determining compliance with the UDC insulation and window requirements. To use the prescriptive package method, enter your actual design values in the "Actual" row below. For a component, with two or more areas of different insulation levels, such as windows, either use the least insulating value for both areas or use the Weighted Average tables below. Multiply your % glazing by the glazing U-value to obtain your "Glazing Factor". Find the Prescriptive Table that applies to your space heating fuel and sheathing type. Select a package from the table that most closely matches the construction indicated on your plans. Do not exceed the package U-values or glazing factor or fall below the package R-values with your design. Transfer the R-Values and U-values to the blank table below in the "Allowed" row. Then proceed to Section F. See page 2 for detailed instructions for this section.

	Package	% glazing	U glazing	Glazing Factor	R wall	R ceiling	R Bsmt Crawl	U door	U	Equip.
	#			(% glazing × U			Space, Slab or	and the	overall	Eff.
		land the second	a an an agus	glazing)			Floor			······································
Actual		10.2% (A.15)	0.41	0.042	R13 + 5	R40	R5	0.35		High
Allowed	45			0.0504 Max	R18, / Min	R40 Min	R5 Min	0.35 Max	0.086	High

(Please go to Section F.)

Optional R-Value/U-Value Weighted Average Table for Component: Windows

Component Cor	struction Description	R Value	U-Value (1÷R Value)	Area (sq ft)	U-Value × Area (UA)
Basement windo	WS		0.87	20	17.4
Above-foundatio	n windows	a statistica de la composición de la c	0.35	150	52.5
69.9	170	0.4	F1	Total Area = 170	Total UA = 69.9
(Total UA)	(Total Area)	(Weighted A	verage U-Value ((for windows or doors))	la de la companya de Reference de la companya de la company
(Total Area)	(Total UA)	(Weighted Average R-Value (for all other components))			

Optional R-Value/U-Value Weighted Average Table for Component:

(Total Area)

Component Construction Description	R Value	U-Value (1÷R Value)	Area (sq ft)	U-Value × Area (UA)
		the second second second	and the second	
			Total Area =	Total UA =

(Total UA)

(Weighted Average U-Value (for windows or doors))

(Total Area) (Total UA) (Weighted Average R-Value (for all other components))

Because the sample house fit a Package, you would normally skip ahead to Section F. For demonstration purposes here. the System Design Method is also completed.

C. Code-Allowed Heat Loss For System Design Method

Enter area values from Section A as notated and temperature differences per footnote 2 into this table and then multiply across by the electric or non-electric code-required U-value. Total the right column to find the total allowed heat loss factor.

Component	Area From Sect A.	× Requi	= Heat Loss UA	
		NON-ELEC	ELECTRIC	
1 Opaque Basement Wall [2]	1468 (A.5.)	0.077 [3]	0.077 [3]	113
2. Above Foundation Code Wall	1675 (A.7.)	0.110	0.080	184
3. Floor Over Interior Unconditioned Space	(A.9.)	0.050	0.050	
4. Roof or Ceiling	1500 (A.10.)	0.026	0.020	39
5. Floor Over Exterior	(A.11.)	0.033	0.033	
6. Crawl Space Wall	(A.12.)	0.060	0.060	
7. Slab On Grade Unheated Heated [3]	(A.13.) Lin. ft.	0.72 'F' 0.70 'F'	0.68 'F' 0.68' F'	
8 Subtotal		•, <u></u>		336
 Oredit for High Efficiency Heating Plant: 1.18 for fu Otherwise use 1.0 	imace or boiler >90% AFUI	E; 1.15 for heat pur	np> 7 8 HPSF,	× 1.18
10	396.5			
D. System Design Method - Actual 'U' Values Of Your Home's Components

D.1. Above-Foundation Components - If applicable, check the appropriate typical component constructions listed below, and use the pre-calculated U values. If your wall construction is not listed, you may obtain a pre-calculated U value from the default U-Value tables in the UDC Appendix. (Note that the default Table 2 Wood Frame U-values assume no insulating sheathing which penalizes you if your wall does have insulating sheathing, then you may need to use the Manual Calculation section below.) If you are using exterior metal framing, then you must use the Metal-Frame Wall U-Values of the UDC Appendix. If your component construction is not listed here or in the default tables, you need to use the Manual Calculation section below.) If you are using either layers of building materials from the Typical Thermal Properties of Building Materials Table of the UDC Appendix, ASHRAE Fundamentals Manual or manufacturer's specifications. Total them across and then obtain the U-value by taking the reciprocal (1/R) of the total R-value.

Above-Foundation	Walls 2X4	. 16" O.C	. R-13 bat	. R-1 board: U	- 079	D 2X4	. 16" O.C., R-1	3 batt. R-	5 board: 1	J - :061	
and Maria and Araba	2X6, 16" O.C., R-19 batt, R-1 board: U - 059							9 batt. R-	5 board: 1	J049	
Other - describe:	1					and the second	U	-	from D	efault Tab	le
Roof or Ceiling	• 🗆 2X4	truss, 24'	O.C., with	n R-38 insulatio	n: U03	0 🗆 2X4	truss, 24" O C	with R-1	52 insulati	on: U02	25
	🗆 2X1	2 cathedra	al ceiling, 1	6" O.C., with R	-38 insula	ation U02	7				
Other - describe:	R40 with regu	ilar truss	ies				ι) - 0.029	from E	Default Tab	le 1
Floor Over Exterior	or Unconditio	ned Spac	e	2X10 joists,	16" O.C.,	R-19 batt: L	J047				
Other - describe:							I	J -	from I	Default Tab	le
-		Optio	nal Manua	al U-Value Calo	ulation (i	f assembly i	not listed abov	re)	1		
	Cavity Or	Ext.	Ext.	Insulation	Shea-	Framing	Insulation	Inter-	Int.	Total	U-Value
Component	Solid If	Air	Finish	Over	thing	Or Solid	Within	ior	Air	R -	
Name	Applicable	Film*		Framing			Cavity	Finish	Film*	Value	
Above Foundation	Cavity	.17	0.79	5.0	l		13	0.56	.68	20.2	.050
Wall	Solid	.17	0.79	5.0		4.4		0.56	.68	11.6	.086
	Cavity										
	Solid										L

		* Air Film R	R-Values	and the second second second			
Location		Heat Flow Direction					
	Upwar	rds	Horizontal	Downwards			
Exterior	.17		.17	.17			
Interior	.61		.68	.92			

D.2. Foundation And Slab-On-Grade Components - Check appropriate boxes for planned type of construction to determine precalculated overall 'U-value' including air films, wall, insulation, soil and cavity/solid differences. Slab on grade F-values are per lineal foot of slab perimeter.

Component Type	U-Va	lue
Foundation Wall	Basement	Crawl Space
□ Masonry or concrete wall without insulation	0.360	0.477
Masonry or concrete wall with R-5 insulation board for full height	0.115	0.136
☐ Masonry or concrete wall with R-10 insulation board or R-11 insulation batt and 2X4's for full height	0.072	0.081
Permanent wood foundation with R-19 batt for full height	0.054	0.059
Basement or crawl space floor without insulation	0.025	0.025
Basement floor with R-5 insulation	0.022	0.022
Slab-On-Grade (or within 12" of grade)	F-Val	lue
□ Slab-on-grade without insulation	1.04	1
□ Slab-on-grade with R-5 insulation for 48" total horizontal and vertical application	0.74	4
□ Slab-on-grade with R-10 insulation board for 48" total application	0.68	3

D.3. Windows And Doors - Use manufacturer's specifications for window and glazed door values, if they were determined per NFRC Std 100, to enter into Table E. Otherwise see default tables of UDC s. Comm 22.05 for U-values.

E. System Design Method - Calculated Envelope Heat Loss Factor Of Your Home

Enter values into table from elsewhere on this worksheet and multiply across to find the actual heat loss factor of each component. If using pre-calculated component U-values, do not calculate separate cavity and solid figures or apply wood frame factors. Total component heat loss factors in right column to find total envelope heat loss factors.

	Cavity Or	Area	× .	X A stual (IP Volue	= Heat Loss Factor
Component	Applicable	Sect. A	Factor**	From Sect. D	(UA)
Above-Foundation Windows		150 (A.1.a.)		0.35	52.5
Foundation Windows		20 (A.1.b)		0.87	17.4
Doors		38 (A.2.c)		0.35	13.3
Opaque Basement Wall		1468 (A.5.)	eeservertee : :	0.115	168.8
Opaque Above-Foundation Wall	Cavity		.75	.050	55
Opuque : rect - caracter - and	Solid	1467 (A.8.)	.25	.086	31.5
Floor Over Unconditioned Spaces	Cavity				a sa
	Solid	(A.9.)			
Roof or Ceiling	Cavity				
5	Solid	1500 (A.10.)		0.029	43.5
Floor Over Exterior	Cavity				
	Solid	(A.11.)			
Crawl Space Wall		(A.12.)			
				the second second	· · · · · · · · · · · · · · · · · · ·
					the second s
Slab On Grade		(A.13.)Lin. ft.		F-Value	
Total Calculated Envelope H Factor of line 10 of Section C	Heat Loss Factor. (Enter here:	or- Not to exceed 396.5)t	Total Code All by more than 19	owed Heat Loss	382

** Adjustment Factors For Wood-Framed Components - Do not apply if your are using a pre-calculated or default U-Value.

	Spacing Of Framing	Stud	Walls	Joists/Rafters			
12	Members	Cavity	Solid	Cavity	Solid		
	12"	.70		.86	14		
1.00	16"	75	25	.90	10		
	24"	.78	.22	.93	.07		

F. Heat Loss Factor Due to Air Infiltration (for heating equipment sizing) Enter appropriate values. A maximum infiltration air change rate of 0.5 per hour is allowed in addition to ventilation losses.

Floor Level	Area (sq ft)	× Height (ft)	Fan Capacity (cfm)	× Constant	× Air Changes Per Hour	= Heat Loss Factor(UA)
Basement	1500	8		.018	0.3	64.8
Level 1	1500	8		.018	0.3	64.8
Level 2				.018		
Level 3				.018		
Ventilation			170	.432		73.4
		Tota	Infiltration &	& Ventilation	Heat Loss Factor	203

G. Heating Equipment Sizing

Enter appropriate value to determine the maximum and minimum allowable heating equipment capacity in BTUs/HR. A more detailed calculation may be submitted to the local code official. [4]

Prescriptive 0.086 4643		
Method: U overall from selected Prescriptive Total Envelope Area Package of Section B (A.14.)		399.3
OR System Design Method: Calculated Heat Loss Factor from Sect. E.		
Infiltration & Ventilation Heat Loss Factor (from Sect. F.)	+	203
Total Heat Loss Factor (UA)	=	602.3
Temperature Difference from Zone Table on page 1	×	85
Minimum Heating Equipment Output	=	51,196
Allowable Heating Equipment Size Margin Multiplier	×	1.15
Maximum Allowable Heating Equipment Output [5]	=	58,875
Planned Furnace Output Or Boiler IBR Rating		60,000
Make & Model if High Efficiency Credit has been taken: Acme XLH60K		

Package	Glazing Factor	R wall	R ceiling	R basement	U door	U overall	HVAC Equipment Efficiency
1	0.0370	R21	R42	R7	0.35	0.073	Normal
2	0.0264	R21	R51, RT	R5	0.35	0.073	Normal
3	0.0333	R15	R42	R10	0.35	0.073	Normał
4	0.0440	R19	R33	R10	0.35	0.073	Normal
5	0.0330	R13	R42	R11	0.35	0.073	Normal
6	0.0480	R19	R33	R11	0.35	0.073	Normal
7	0.0600	R21	R47	R11	0.35	0.073	Normal
8	0.0407	R13	R44	R13	0.35	0.073	Normal
9	0.0600	R19	R42	R13	0.35	0.073	Normal
10	0.0680	R21	R38, RT	R13	0.35	0.073	Normal
11	0.0296	R13	R49	R5	0.35	0.086	High
12	0.0440	R19	R30	R5	0.35	0.086	High
13	0.0520	R21	R33	R5	0.35	0.086	High
14	0.0720	R13	R47	R10	0.35	0.086	High
15	0.0784	R19	R38	R10	0.47	0.086	High
16	0.0640	R13	R33	R11	0.47	0.086	High
17	0.0896	R19	R49	R11	0.35	0.086	High
18	0.0896	R21	R34	R11	0.35	0.086	High
19	0.0920	R19	R34	RII	0.47	0.086	High
20	0.0840	R13	R49	R13	0.35	0.086	High
21	0.0840	R19	R30	R13	0.47	0.086	High
22	0.0896	R21	R31	R13	0.47	0.086	High
Package	Glazing Factor	R wall	R ceiling	R crawl	U door	U overall	HVAC Equipment Efficiency
23	0.0520	R19	R34	R19	0.47	0.070	Normal
24	0.0672	R13	R36	R19	0.47	0.083	High
25	0.0720	R13	R33	R19	0.47	0.083	High
Package	Glazing Factor	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency
26	0.0560	R21	R36	R5	0.47	0.103	Normal
27	0.0728	R13	R36	R5	0.47	0.121	High
28	0.0760	R13	R34	R5	0.47	0.121	High
Package	Glazing Factor	R wall	R ceiling	R heated-slab	U door	U overall	HVAC Equipment Efficiency
29	0.0560	R21	R47	R5	0.47	0.101	Normal
30	0.0728	R13	R42	R5	0.47	0.120	High
31	0.0760	R13	R38	R5	0.47	0.120	High
Package	Glazing Factor	R wall	R ceiling	R floor	U door	U overall	HVAC Equipment Efficiency
32	0.0480	R19	R47	R19	0.35	0.065	Normal
33	0.0728	R19	R36	R19	0.47	0.077	High
34	0.0560	R13	R34	R19	0.47	0.077	High

Prescriptive Package Tables (Corrected) (See notes on page 2 of Energy Worksheet; 1 = insulating sheathing, RT = raised heel roof truss)

alastnia Host In Joting Sheathin Table P 2 D . . . -1-No

			abic D-4	rescriptive pa	ckages, inon-elec	tric neat, In	8		
- 1	Package	Glazing Factor	R wall	R ceiling	R basement	U door	U overall	HVAC Equipment Efficiency	
- [35	0.0370	R20, 1	R42	R7	0.35	0.073	Normal	
- I	36	0.0363	R28, 1	R38, RT	R5	0.35	0.073	Normal	
· • •	37	0.0552	R18, I	R44	R10	0.35	0.073	Normal	
	38	0.0560	R20, 1	R47	R10	0.35	0.073	Norma]	
1	39	0.0560	R23, 1	R34	R10	0.35	0.073	Normal	
1	40	0.0560	R18, I	R47	R11	0.35	0.073	Normai	
Ì	41	0.0616	R23, 1	R42	R11	0.35	0.073	Normal	
	42	0.0546	R18, 1	R44	R11	0.35	0.073	Normal	
1	43	0.0672	R23, 1	R40	R13	0.35	0.073	Normal	
	44	0.0720	R25, 1	R36	R13	0.35	0.073	Normal	
	45	0.0504	R18, J	R40	R5	0.35	0.086	High	
1	46	0.0560	R19, 1	R47	R5	0.35	0.086	High	
Ī	47	0.0560	R23, 1	R38	R5	0.47	0.086	High	
. I	48	0.0600	R25, I	R38	R5	0.47	0.086	High	
ſ	49	0.0680	R26, 1	R42	R5	0.35	0.086	High	
1	50	0.0680	R28, 1	R47	R5	0.47	0.086	High	
t	51	0.0672	R26, I	R47	R5	0.35	0.086	High	
- 1	52	0.0672	R28, I	R38	R5	0.35	0.086	High	
ſ	53	0.0720	R20, I	R42	R7	0.47	0.086	High	
f	54	0.0855	R18, I	R36	R11	0.35	0.086	High	

Wisconsin Uniform Dwelling Code Energy Worksheet

Instructions: This worksheet is a Safety & Buildings Division (S&BD)-approved method of manually showing compliance with the energy conservation and heating equipment sizing requirements of the Uniform Dwelling Code (UDC), for new dwelling permits **submitted on or after February 1, 1999**. It may be necessary for the user to purchase a copy of the UDC from State Document Sales, (608)266-3358. Additional information is printed in the UDC Commentary, which is available for a fee, as are blank copies of this form, from S&BD at POB 2509, Madison, WI 53701, Tel. 608-267-4405. **Earlier editions of this worksheet may NOT be used**. Numbers in brackets, [1], refer to the footnotes printed on page 2.

You may also submit completed worksheets from the computer program WIScheck, which is available for free download from http://www.energycodes.org/ on the Internet.

A required U-value is the **maximum** acceptable heat transmittance for an element. A required insulation R-value is the **minimum** acceptable level of resistance to heat transmittance. (U-values and R-values are reciprocals of each other.) If a component includes two or more areas of different insulation levels, either use the less insulating value for both areas, or use the Optional Weighted Average table in the **Prescriptive Package Method** section or enter separate areas and insulation values in the **System Design Method**. All "U" values must be carried to four places after the decimal point, rounded to three places. Other values may be rounded to the whole number.

Window and door U-values must be tested and documented by the manufacturer in accordance with the National Fenestration Rating Council (NFRC) test procedures or be taken from the glazing U-value table in s. Comm 22.05. Center-of-glass U-values cannot be used. If a door contains glass and an aggregate U-value rating for that door is not available, include the glass area of the door with your windows and use the opaque door U-value to determine compliance of the door.

The code gives credit for high-efficiency heating equipment. "High-Efficiency" means a furnace with an AFUE of 90% or more, or a heat pump with an HSPF of 7.8 or more without the use of electric resistance backup heat of greater than 3 kilowatts. If you plan to install more than one piece of heating equipment, the equipment with the lowest efficiency must exceed the efficiency required by the selected package.

Choice of Method: You have the choice of using the Prescriptive Package Method or the System Design Method to show code compliance. For the simpler **Prescriptive Package Method**, which is recommended for standard designs, complete Sections A., B., F., and G. Instructions are on page 2. You will be first calculating component areas, then comparing your planned insulation levels to the required insulation levels of the Prescriptive Packages. You will then calculate infiltration and ventilation heat losses to size your heating equipment. If you cannot comply with one of the prescriptive packages, you may be able to show compliance by the System Design Method.

For the System Design Method, which is recommended for alternative designs in which more insulation is installed in one component to offset less in another, complete Sections A., C., D., E., F. and G. You will be first calculating component areas, then a code-allowed heat loss factor, then component U- and R-values and then your calculated heat loss factor which you will compare to the code-allowed heat loss factor. You will then calculate infiltration and ventilation heat losses to size your heating equipment.

The **County Zone Table** below is use for determining the temperature difference for sizing your heating plant in Section G. You may submit to your local code official more exact calculations to size your heating equipment.

Zone 1 - 95 degrees	Zone 2 - 90 degrees	Zone 3 - 85 degrees	Zone 4 - 80 degrees
Ashland, Barron, Bayfield,	Adams, Buffalo, Clark, Eau Claire,	Brown, Calumet, Columbia, Crawford,	Jefferson, Kenosha.
Burnett, Chippewa, Douglas,	Jackson, Juneau, LaCrosse, Langlade,	Dane, Dodge, Door, Fond du Lac.	Milwaukee, Ozaukee,
Dunn, Florence, Forest, Iron,	Marathon, Marinette, Menominee,	Grant, Green, Green Lake, Iowa,	Racine, Rock,
Lincoln, Oneida, Pierce, Polk,	Monroe, Portage, Shawano, Oconto,	Kewaunee, LaFayette, Manitowoc,	Walworth
Price, Rusk, Saint Croix,	Pepin, Trempeleau, Vernon,	Marquette, Outagamie, Richland, Sauk,	Washington,
Sawyer, Taylor, Vilas, Washburn	Waupaca, Wood	Sheboygan, Waushara, Winnebago	Waukesha

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Detailed Instructions for Section B. Prescriptive Package Method:

R-value requirements are for insulation only and do not include structural components.

For a component with two or more areas of different insulation levels, either use the least insulating value for both areas or use the Weighted Average tables on page 4.

Wall R-values represent the sum of the wall cavity insulation plus insulating sheathing, if used. Do not include exterior siding, structural sheathing or interior drywall. For example, an R-20 requirement could be met *EITHER* by R-15 cavity insulation plus R-5 sheathing *OR* R-13 cavity insulation plus R-7 sheathing. Note that there are separate tables for walls with structural sheathing only and for walls with insulating sheathing. To use a table for insulating sheathing, the sheathing used must be at least R-4, except that at least R-2 insulation may be provided over corner bracing. Table wall R-Values apply to wood-frame or mass (concrete, masonry, log) wall assemblies, but not to metal-frame construction. If metal frame is planned, use the adjusted R-Values from the Metal-Frame Wall Tables of the UDC Appendix. Table wall values apply to boxsills.

Ceiling R-values represent the sum of the cavity insulation plus insulating sheathing, if used. For ventilated ceilings, any insulating sheathing must be placed between the conditioned space and the ventilated portion of the roof. Ceiling R-values with "**RT**" indicates that a raised-heel truss or oversized truss construction must be used so that the insulation achieves the full insulation thickness over the exterior walls.

Floor requirements apply to floors over unconditioned spaces (such as un-insulated crawlspaces, basements and garages). Floors over outside air shall have a Uoverall = 0.033 or R-30 added insulation.

"Heated-Slab" requirements apply to slabs that contain heat ducts or pipes. All slab insulation must extend at least 48 inches either 1) down from the top of the slab, or 2) down from the top of the slab to the bottom of the slab and then horizontally underneath the slab, or 3) down from the top of the slab to the bottom of the slab and then horizontally away from the slab, with pavement or at least 10 inches of soil covering the horizontal insulation.

Walls of basements below un-insulated floors must be insulated from the top of the basement wall to the level of the basement floor. Conditioned basement windows and glass doors must be included with the other glazing. Exterior basement doors must meet the door U-value requirements. If more than 50% of the basement is exposed, then all of the basement walls must instead meet the above-foundation wall requirements.

Crawl space wall R-value requirements are for walls of unventilated crawlspaces. The crawlspace wall insulation must extend from the top of the wall (including the sill plate) to at least 12 inches below the outside finished grade. If the distance from the outside finished grade to the top of the footing is less than 12 inches, the insulation must extend a total vertical plus horizontal distance of 24 inches from the outside finished grade.

Footnotes for worksheet:

- [1] Opaque wall area is wall area minus opening areas of doors and windows.
- [2] These below-grade U-values have the insulating value of the soil added to the code-required U-values which apply to the building materials only. See Sect. D.2. for typical insulated component U-values.
- [3] These slab-on-grade F-values are derived from the code-required U-values and include the heat loss through the edge and body of the slab. See Sect. D.2. Temperature difference is the same as for above-grade spaces.
- [4] For building additions, show that the existing heating equipment, if used to heat the addition, is large enough. To do so, you must calculate the heat loss of the whole building.
- [5] If desired manufacturer does not have a furnace of this size, then a designer may select the manufacturer's next larger size.

Submit completed worksheet pages 3-6 with dwelling plans to local enforcing municipality.

Project Address:

Owner: Builder:

Worksheet Completed By: Date: Does dwelling unit have three kilowatts or more input capacity of permanently installed electrical space heating equipment? □ YES (see below) D NO

You will need to apply the stricter standards shown for electrically-heated homes if you answered "YES" to the above question.

A. Area Calculations

Enter appropriate dimensions to obtain area values. Some calculations will not be necessary depending on home design or calculation method. These calculated areas are referenced elsewhere on this worksheet, for example, "(A.1.)".

1. Window, Skylight & Patio Door Area (overall unit area)	2. Opaque Door Area
a. In Above-Foundation Walls b. In Foundation Walls	a. In Above- Foundation Walls b. In Foundation Walls
and the second	and a second
	and the second second state of the second
sq. ft sq. ft.	sq. ft sq. ft.
c. Total $(a. + b.) =$	c. Total $(a. + b.) =$
3. Gross Exposed Basement Wall Area	4. Basement Wall Area Below Grade
	and the second
Sq. II.	Sq. it.
(A.5. + A.4 A.1.0 A.2.)	o. Gross rieated Above-roundation wan Area, including boxsin
A.2.0.) a set of the top of the set of the s	and the second
and the state of the second	
 A segment of the second second by spice 	(1) The second s second second s second second sec second second sec
So ff	and the second secon
If the exposed area of A.3. is greater than the below grade area of	so ft.
A.4., add A.5. to A.7 and cross out the number in this cell.	
7. Above Foundation Code Wall Area (A.6. + A1.b. + A.2.b.)	8. Opaque [1] Above-Foundation Wall Area (A.6 A1.a A.2.a.)
[1] A set of a set of a set of a set of the set of t	
and the second secon	
statute the statute of the statute of the statute of sq. ft.	sq. ft.
9. Floor Area Over Interior Unconditioned Spaces Less Than	10. Insulated Roof Or Ceiling (less skylights)
1 50° in production (1), internet of the cost of the	e de la companya de l
[1] A. C. M. A. R. Martine, A. Martine, A. M. Martine, A. M. Martine, and A. M. Martine, A. M. Martine, A. M. Martine, A. M. Martine, and A. M Antonio, and A. Martine, and A Antonio, and A. Martine, a Antonio, and A. Martine, a	the state of the s
sq. n.	
11. Exterior Floor Area (Overnangs)	12. Crawl Space wall Area
	na se en
	a series and a series of the series of th The series of the series of
in a standard and the standard standard and the standard standard standard standard standard standard standard s	and the second
13 Slab On Grade (above or less than 12 inches below grade)	14 Total Heated Envelope Area (A.5 + A.7 + A.9 + A.10 + A.11 +
15. One on Grade (above or less and 12 ments over grade)	$A = 12 + (A = 13 \times 2^{i}))$
en e	
Alternation of the second s	$\sigma = M_{\rm eff} + e_{\rm eff} f_{\rm eff} + e_{\rm eff} + e_{\rm eff} f_{\rm eff} + e_{\rm eff} + $
lineal feet of slab perimeter	sa. ft.
15. Percent Glazing (for Prescriptive Package Method.	16. Windows Description - Above-Foundation Windows:
Section B. only) (A.1.c. \div A.7 \times 100%)	Frame type: 🛛 Wood or Wood Clad 🖾 Vinyl 🗆 Metal
	Glazing type: Dual DTriple Dual w/storm panel
	Dual-Glazing Air Space: 🗆 1/4' 🗆 3/8" 🗖 1/2" or more
%	Features: Low-E Argon-filled Suspended film
	Foundation Windows: Vinv) [] Metal

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B. Prescriptive Package Method (Skip this section if using the System Design Method of Sections C-F)

The prescriptive package method is the simplest method for determining compliance with the UDC insulation and window requirements. To use the prescriptive package method, enter your actual design values in the "Actual " row below. For a component, with two or more areas of different insulation levels, such as windows, either use the least insulating value for both areas or use the Weighted Average tables below. Multiply your % glazing by the glazing U-value to obtain your "Glazing Factor". Find the Prescriptive Table that applies to your space heating fuel and sheathing type. Select a package from the table that most closely matches the construction indicated on your plans. Do not exceed the package U-values or glazing factor or fall below the package R-values with your design. Transfer the R-Values and U-values to the blank table below in the "Allowed" row. Then proceed to Section F. See page 2 for detailed instructions for this section.

	Package	% glazing	U glazing	Glazing Factor	R wall	R ceiling	R Bsmt, Crawl	U door	U	Equip.
	#		1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	(% glazing × U	1.1	$(1,1) \in \mathcal{F}(T)$	Space, Slab or		overall	Eff.
				glazing)			Floor			
Actual		% (A.15)	1 A.							l
Allowed				Max	Min	Min	Min	Max		

(Please go to Section F.)

Optional R-Value/U-Value Weighted Average Table for Component:

Component Construction Description	R Value	U-Value (1÷R Value)	Area (sq ft)	U-Value × Area (UA)
			Total Area =	Total UA =

Optional R-Value/U-Value Weighted Average Table for Component:

Component Construction Description	R Value	U-Value (1÷R Value)	Area (sq ft)	U-Value × Area (UA)
		1		
		l The second second	Total Area =	Total UA =
(Total UA) (Total Area)	= (Weighted)	Average U-Value	(for windows or doors))	

(Total Area) (Total UA) (Weighted Average R-Value (for all other components))

C. Code-Allowed Heat Loss For System Design Method

Enter area values from Section A as notated and temperature differences per footnote 2 into this table and then multiply across by the electric or non-electric code-required U-value. Total the right column to find the total allowed heat loss factor.

		Area			= Heat Loss				
	Component	From Sect A.	× Requi	red U-Value	UA				
			□ NON-ELEC	ELECTRIC					
1.	Opaque Basement Wall [2]	(A.5.)	0.077 [3]	0.077 [3]					
2.	Above Foundation Code Wall	(A.7.)	0.110	0.080					
3.	Floor Over Interior Unconditioned Space	(A.9.)	0.050	0.050					
4.	Roof or Ceiling	(A.10.)	0.026	0.020					
5.	Floor Over Exterior	(A.11.)	0.033	0.033					
6:	Crawl Space Wall	(A.12.)	0.060	0.060					
7	Slab On Grade Unheated Heated [3]	(A.13.) Lin. ft.	0.72 'F' 0.70 'F'	0.68 'F' 0.68' F'					
8.	Subtotal								
9.	 Credit for High Efficiency Heating Plant: 1.18 for furnace or boiler >90% AFUE; 1.15 for heat pump> 7.8 HPSF, Otherwise use 1.0 								
10.	10. Total Code-Allowed Heat Loss Factor								

D. System Design Method - Actual 'U' Values Of Your Home's Components

D.1. Above-Foundation Components - If applicable, check the appropriate typical component constructions listed below, and use the pre-calculated U values. If your wall construction is not listed, you may obtain a pre-calculated U value from the default U-Value tables in the UDC Appendix. (Note that the default Table 2 Wood Frame U-values assume no insulating sheathing which penalizes you if your wall does have insulating sheathing, then you may need to use the Manual Calculation section below.) If you are using exterior metal framing, then you must use the Metal-Frame Wall U-Values of the UDC Appendix. If your component construction is not listed here or in the default tables, you need to use the Manual Calculation section below.) If you are using inter listed here or in the default tables, you need to use the Manual Calculation section below to manually enter R-values for the different layers of building materials from the Typical Thermal Properties of Building Materials Table of the UDC Appendix. ASHRAE Fundamentals Manual or manufacturer's specifications. Total them across and then obtain the U-value by taking the reciprocal (1/R) of the total R-value.

Above-Foundation	Walls 2X4	, 16" O.C	. R-13 bat	t, R-1 board: 1	U079	🗆 2X4	, 16" O.C., R-1	3 batt, R-:	5 board: U	J - 061	
2X6, 16" O.C., R-19 batt, R-1 board: U059 2X6, 16" O.C., R-19 batt, R-5									5 board: l	J049	
Other - describe:		, 1. M.					່ ບ	-	from De	fault Table	2
Roof or Ceiling	🗆 2X4	truss, 24"	O.C., wit	h R-38 insulati	on: U03	0 🗆 2X4	truss, 24" O.C	, with R-	52 insulati	on: U'- 02	25
	🗆 2X I	2 cathedra	al ceiling,	16" O.C., with	R-38 insula	ation U02	7				
Other - describe:			- 1995 - E				U	-	from De	fault Table	
Floor Over Exterior	or Unconditio	ned Space	e	2X10 joists	, 16" O.C.,	R-19 batt: I	J047		1997 - 1997 1997 - 1997 - 1997		
🖸 Other - describe:		-					U	-	from De	fault Table	
		N	Ianual U-	Value Calcula	tion (if ass	embly not li	sted above)		in a shara		
	Cavity Or	Ext.	Ext.	Insulation	Shea-	Framing	Insulation	Inter-	Int.	Total	U-Value
Component	Solid If	Air	Finish	Over	thing	Or Solid	Within	ior	Air	R-	
Name	Applicable	Film*	l .	Framing		1	Cavity	Finish	Film*	Value	
	Cavity	···· ·					e esta la c		<u> </u>		
n Maria an an an	Solid					1					1
	Cavity										
	Solid		-								

and the second	* Air Film R-	Values						
Location	Heat Flow Direction							
	Upwards	Horizontal	Downwards					
Exterior	.17	.17	.17					
Interior	.61	.68	.92					

D.2. Foundation And Slab-On-Grade Components - Check appropriate boxes for planned type of construction to determine precalculated overall 'U-value' including air films, wall, insulation, soil and cavity/solid differences. Slab on grade F-values are per lineal foot of slab perimeter.

Component Type	U-Value			
Foundation Wall	Basement	Crawl Space		
□ Masonry or concrete wall without insulation	0.360	0.477		
□ Masonry or concrete wall with R-5 insulation board for full height	0.115	0.136		
☐ Masonry or concrete wall with R-10 insulation board or R-11 insulation batt and 2X4's for full height	0.072	0.081		
Permanent wood foundation with R-19 batt for full height	0.054	0.059		
Basement or crawl space floor without insulation	0.025	0.025		
Basement floor with R-5 insulation	0.022	0.022		
Slab-On-Grade (or within 12 " of grade)	F-Va	lue		
□ Slab-on-grade without insulation	1.04	4		
□ Slab-on-grade with R-5 insulation for 48" total horizontal and vertical application	0.7	\$		
Slab-on-grade with R-10 insulation board for 48" total application	0.6	3		

D.3. Windows And Doors - Use manufacturer's specifications for window and glazed door values, if they were determined per NFRC Std 100, to enter into Table E. Otherwise see default tables of UDC s. Comm 22.05 for U-values.

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E. System Design Method - Calculated Envelope Heat Loss Factor Of Your Home

Enter values into table from elsewhere on this worksheet and multiply across to find the actual heat loss factor of each component. If using pre-calculated component U-values, do not calculate separate cavity and solid figures or apply wood frame factors. Total component heat loss factors in right column to find total envelope heat loss factors.

Component	Cavity Or Solid If	Area From Sect A	× Wood Frame Factor**	× Actual 'U' Value From Sact D	= Heat Loss Factor
Above-Foundation Windows		(A.1.a.)		Stell B	(0.1.)
Foundation Windows		(A.1.b)			
Doors		(A.2.c)			
Opaque Basement Wall		(A.5.)			
Opaque Above-Foundation Wall	Cavity				
	Solid	(A.8.)			
Floor Over Unconditioned Spaces	Cavity	4 - 1 -			
	Solid	. (A.9.)			
Roof or Ceiling	Cavity				
	Solid	(A.10.)			
Floor Over Exterior	Cavity				
	Solid	(A.11.)			
Crawl Space Wall		(A.12.)			
······································					
Slab On Grade	*******	(A.13.)Lin. ft.	**********	F-Value	
Total Calculated Envelope H	leat Loss Fac	tor-Not to exceed)	Total Code Allore than 1%	owed Heat Loss	-

1.1	Spacing Of Framing	Stud	Walls	Joists/1	Rafters
	Members	Cavity	Solid	Cavity	Solid
	12"	.70	30	.86	.14
	16"	.75	25	.90	.10
	24"	.78	.22	.93	.07

F. Heat Loss Factor Due to Air Infiltration (for heating equipment sizing)

Enter appropriate values. A maximum infiltration air change rate of 0.5 per hour is allowed in addition to ventilation losses.

Floor Level	Area (sq ft)	× Height (ft)	Fan Capacity (cfm)	× Constant	× Air Changes Per Hour	= Heat Loss Factor(UA)
Basement				.018		
Level 1				.018		
Level 2				.018		
Level 3			*******	.018		
Ventilation	***********			.432		
		Tota	l Infiltration &	Ventilation	Heat Loss Factor	

G. Heating Equipment Sizing

Enter appropriate value to determine the maximum and minimum allowable heating equipment capacity in BTUs/HR. A more detailed calculation may be submitted to the local code official. [4]

1

Prescriptive Package ×	
Method: U overall from selected Prescriptive Total Envelope Area	
Package of Section B (A.14.)	
OR System Design Method: Calculated Heat Loss Factor from Sect. E.	
Infiltration & Ventilation Heat Loss Factor (from Sect. F.)	+
Total Heat Loss Factor (UA)	=
Temperature Difference from County Zone Table on page 1	×
Minimum Heating Equipment Output	
Allowable Heating Equipment Size Margin Multiplier	× 1.15
Maximum Allowable Heating Equipment Output [5]	=
Planned Furnace Output Or Boiler IBR Rating	S
Make & Model if High Efficiency Credit has been taken:	

Package	Glazing Factor	R wali	R ceiling	R basement	U door	U overall	HVAC Equipment Efficiency
1	0.0370	R21	R42	R7	0.35	0.073	Normal
2	0.0264	R21	R51, RT	R5	0.35	0.073	Normal
3	0.0333	R15	R42	R10	0.35	0.073	Normal
4	0.0440	R19	R33	RIO	0.35	0.073	Normal
5	0.0330	R13	R42	R11	0.35	0.073	Normal
6	0.0480	R19	R33	RII	0.35	0.073	Normal
7	0.0600	R21	R47	R11	0.35	0.073	Normal
8	0.0407	R13	R44	R13	0.35	0.073	Normal
9	0.0600	R19	R42	R13	0.35	0.073	Normal
10	0.0680	R21	R38. RT	R13	0.35	0.073	Normal
11	0.0296	R13	R49	R5	0.35	0.086	High
12	0.0440	R19	R30	R5	0.35	0.086	High
13	0.0520	R21	R33	R5	0.35	0.086	High
14	0.0720	R13	R47	R10	0.35	0.086	High
15	0.0784	R19	R38	R10	0.47	0.086	High
16	0.0640	R13	R33	R11	0.47	0.086	High
17	0.0896	R19	R49	R11	0.35	0.086	High
18	0.0896	R21	R34	R11	0.35	0.086	High
19	0.0920	R19	R34	RII	0.47	0.086	High
20	0.0840	R13	R49	R13	0.35	0.086	High
21	0.0840	R19	R30	R13	0.47	0.086	High
22	0.0896	R21	R31	R13	0.47	0.086	High
Package	Glazing Factor	R wall	R ceiling	R crawl	U door	U overall	HVAC Equipment Efficiency
23	0.0520	R19	R34	R19	0.47	0.070	Normal
.24	0.0672	R13	R36	R19	0.47	0.083	High
25	0.0720	R13	R33	R19	0.47	0.083	High
Package	Glazing Factor	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency
26	0.0560	R21	R36	R5	0.47	0.103	Normal
27	0.0728	R13	R36	R5	0.47	0.121	High
28	0.0760	R13	R34	R5	0.47	0.121	High
Package	Glazing Factor	R wall	R ceiling	R heated-slab	U door	U overall	HVAC Equipment Efficiency
29	0.0560	R21	R47	R5	0.47	0.101	Normal
30	0.0728	R13	R42	R5	0.47	0.120	High
31	0.0760	R13	R38	R5	0.47	0.120	High
Package	Glazing Factor	R wall	R ceiling	R floor	U door	U overall	HVAC Equipment Efficiency
32	0.0480	R19	R47	R19	0.35	0.065	Normal
33	0.0728	R19	R36	R19	0.47	0.077	High
34	0.0560	R13	R34	R19	0.47	0.077	High

Prescriptive Package Tables (Corrected)

ptive packages, Non-electric Heat, Insulating Sheathing

Package	Glazing Factor	R wall	R ceiling	R basement	U door	U overall	HVAC Equipment Efficiency
35	0.0370	R20, I	R42	R7	0.35	0.073	Normal
36	0.0363	R28, I	R38, RT	R5	0.35	0.073	Normal
37	0.0552	R18, I	R44	R10	0.35	0.073	Normal
38	0.0560	R20, I	R47	R10	0.35	0.073	Normal
39	0.0560	R23, I	R34	R10	0.35	0.073	Normal
40	0.0560	R18, I	R47	R11	0.35	0.073	Normal
41	0.0616	R23, 1	R42	R11	0.35	0.073	Normal
42	0.0546	R18, I	R44	R11	0.35	0.073	Normal
43	0.0672	R23, I	R40	R13	0.35	0.073	Normal
44	0.0720	R25, 1	R36	R13	0.35	0.073	Normal
45	0.0504	R18, I	R40	R5	0.35	0.086	High
46	0.0560	R19, I	R47	R5	0.35	0.086	High
47	0.0560	R23, I	R38	R5	0.47	0.086	High
48	0.0600	R25, 1	R38	R5	0.47	0.086	High
49	0.0680	R26, 1	R42	R5	0.35	0.086	High
50	0.0680	R28.1	R47	R5	0.47	0.086	High
51	0.0672	R26, 1	R47	R5	0.35	0.086	High
52	0.0672	R28, I	R38	R5	0.35	0.086	High
53	0.0720	R20, 1	R42	R7	0.47	0.086	High
54	0.0855	R18. I	R36	RII	0.35	0.086	High

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55	0.0896	R23, 1	R33	R11	0.47	0.086	High
56	0.0861	R18, I	R36	R13	0.47	0.086	High
57	0.1000	R23, 1	R33	R13	0.47	0.086	High
Package	Glazing Factor	R wall	R ceiling	R crawl	U door	U overall	HVAC Equipment Efficiency.
58	0.0546	R18, I	R38	R19	0.47	0.070	Normal
59	0.0784	R15, 1	R30	R19	0.47	0.083	High
60	0.0880	R15, I	R38	R19	0.47	0.083	High
Package	Glazing Factor	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency
61	0.0640	R23, 1	R36	R5	0.47	0.103	Normal
62	0.0896	R15, 1	R36	R5	0.47	0.121	High
63	0.0960	R15, 1	R38	R5	0.47	0.121	High
Package	Glazing Factor	R wall	R ceiling	R heated-slab	U door	U overall	HVAC Equipment Efficiency
64	0.0640	R23, 1	R34	R5	0.47	0.101	Normal
65	0.0840	R15, I	R31	R5	0.47	0.121	High
66	0.0920	R15, 1	R33	R5	0.47	0.121	High
Package	Glazing Factor	R wall	R ceiling	R floor	U door	U overall	HVAC Equipment Efficiency
67	0.0480	R20, I	R44	R19	0.35	0.065	Normal
68	0.0728	R20, I	R36	R19	0.47	0.077	High
69	0.0560	R14, I	R38	R19	0.47	0.078	High

Table B-3 Prescriptive packages, Electric Heat, Structural Sheathing Only

Package	Glazing Factor	R wall	R ceiling	R basement	U door	Uoverall	HVAC Equipment Efficiency
E 70	0.0396	R21	R37, RT	R19	0.35	0.059	Normal
E 71	0.0429	R21	R42. RT	R19	0.35	0.059	Normal
E 72	0.0520	R21	R49	R13	0.35	0.068	High
E 73	0.0640	R19	R42, RT	R19	0.35	0.068	High
E 74	0.0693	R21	R49, RT	R19	0.47	0.068	High
Package	Glazing Factor	R wall	R ceiling	R crawl	U door	U overall	HVAC Equipment Efficiency
E 75	0.0429	R21	R54, RT	R30	0.35	0.054	Normal
E 76	0.0480	R21	R45, RT	R19	0.35	0.062	High
E 77	0.0627	R21	R54, RT	R30	0.47	0.062	High
Package	Glazing Factor	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency
E 78	0.0396	R26	R51, RT	R10	0.35	0.083	Normal
E 79	0.0480	R21	R49	R7	0.35	0.095	High
E 80	0.0528	R21	R49, RT	R5	0.35	0.095	High
Package	Glazing Factor	R wall	R ceiling	R floor	U door	U overall	HVAC Equipment Efficiency
E 81	0.0363	R21	R54, RT	R30	0.35	0.052	Normal
E 82	0.0520	R21	R49	R30	0.35	0.060	High
E 83 .	0.0528	R21	R44. RT	R30	0.47	0.060	High

	Table B-4 Prescriptive packages, Electric Heat, Insulating Sheathing							
Package	Glazing Factor	R wall	R ceiling	R basement	U door	U overall	HVAC Equipment Efficiency	
Ē 84	0.0480	R25.1	R48, RT	R16	0.35	0.059	Normal	
E 85	0.0495	R25, 1	R48, RT	_R16	0.35	0.059	Normal	
E 86	0.0462	R28, I	R40	R16	0.35	0.059	Normal	
E 87	0.0429	R25, I	R36	R18	0.35	0.059	Normal	
E 88	0.0528	R23, I	R58, RT	R18	0.35	0.059	Normal	
E 89	0.0462	R25, I	R42	R18	0.35	0.059	Normal	
E 90	0.0560	R25, 1	R46, RT	R10	0.35	0.068	High	
E 91	0.0640	R23, I	R48, RT	R13	0.35	0.068	High	
E 92	0.0600	R25, 1	R42	R13	0.35	0.068	High	
E 93	0.0600	R23, 1	R37	R18	0.47	0.068	High	
E 94	0.0759	R25, I	R46, RT	R18	0.47	0.068	High	
Package	Glazing Factor	R wall	R ceiling	R crawl	U door	U overall	HVAC Equipment Efficiency	
E 95	0.0429	R25, I	R48, RT	R23	0.35	0.054	Normal	
E 96	0.0520	R23, 1	R38	R23	0.35	0.062	High	
E 97	0.0561	R25, I	R44	R23	0.47	0.062	High	
Package	Glazing Factor	R wall	R ceiling	R slab	U door	U overall	HVAC Equipment Efficiency	
E 98	0.0396	R25,1	R48, RT	R10	0.35	0.083	Normal	
E 99	0.0560	R23.1	R44	R7	0.35	0.095	High	
E 100	0.0594	R25, I	R46, RT	R5	0.47	0.095	High	
Package	Glazing Factor	R wall	R ceiling	R floor	U door	U overall	HVAC Equipment Efficiency	
E 101	0.0429	R25, I	R46, RT	R30	0.35	0.052	Normal	
E 102	0.0560	R23, 1	R44	R30	0.35	0.060	High	
E 103	0.0627	R25, 1	R44, RT	R30	0.47	0.060	High	

Default Assembly R and U Value Tables

(All U-values include framing factors, finish materials and air films.)

Insulation	Standard	Raised	Insulation	Standard	Raised
R –Value	Truss	Truss ^(b)	R-Value	Truss	Truss ^(b)
	U–Value	U-Value		U–Value	U–Value
R-0	0.568	0.568	R-33	0.033	0.029
R-7	0.119	0.119	R-34	0.032	0.028
R8	0.108	0.108	R-35	0.032	0.028
R9	0.098	0.098	R-36	0.031	0.027
R-10	0.089	0.089	R-37	0.031	0.026
R-11	0.082	0.082	R-38	0.030	0.025
R-12	0.076	0.076	R-39	0.030	0.025
R-13	0.070	0.070	R-40	0.029	0.024
R-14	0.066	0.066	R-41	0.029	0.024
R-15	0.062	0.061	R-42	0.028	0.023
R-16	0.059	0.058	R-43	0.028	0.023
R-17	0.056	0.055	R-44	0.027	0.022
R-18	0.053	0.052	R-45	0.027	0.022
R-19	0.051	0.049	R-46	0.027	0.021
R-20	0.048	0.047	R-47	0.026	0.021
R-21	0.047	0.045	R-48	0.026	0.020
R-22	0.045	0.043	R-49	0.026	0.020
R-23	0.043	0.041	R-50	0.026	0.020
R-24	0.042	0.040	R51	0.025	0.019
R-25	0.040	0.038	R-52	0.025	0.019
R-26	0.039	0.037	R-53	0.025	0.019
R-27	0.038	0.035	R–54	0.025	0.018
R-28	0.037	0.034	R-55	0.024	0.018
R-29	0.036	0.033	R-56	0.024	0.018
R-30	0.035	0.032	R–57	0.024	0.018
R-31	0.034	0.031	R-58	0.024	0.017
R-32	0.034	0.030	R-59	0.024	0.017

Table 1. Ceiling U--Values^(a)

(a) R-values represent the sum of the ceiling cavity insulation plus the R-value of insulating sheathing (if used). For example, R-19 cavity insulation plus R-2 sheathing is reported as R-21 ceiling insulation. For ventilated ceilings, insulating sheathing must be placed between the conditioned space and the ventilated portion of the roof (typically applied to the trusses or rafters immediately behind the drywall or other ceiling finish material).

(b) To receive credit for a raised truss, the insulation must achieve its full insulation thickness over the exterior walls.

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Insulation R–Value ^(c)	16-in. O.C. Wall U-Value	24-in. O.C. Wall U-Value
R-0	0.238	0.241
R–7	0.105	0.104
R8	0.099	0.097
R-9	0.094	0.092
R-10	0.090	0.088
R–11	0.089	0.087
R-12	0.085	0.083
R-13	0.082	0.080
R-14	0.079	0.077
R-15	0.077	0.074
R-16	0.066	0.064
R-17	0.064	0.062
R-18	0.062	0.060
, R–19	0.060	0.059
R–20	0.059	0.057
R-21	0.057	0.056
R-22	0.056	0.054
R–23	0.055	0.053
R-24	0.054	0.052
R-25	0.053	0.051
R–26	0.052	0.050
R-27	0.051	0.049
R-28	0.050	0.048

Table 2. Wood-Frame Wall U-Values^(a,b)

(a) U-values are for uncompressed insulation

(b) U-values in this Table were developed for wood-frame walls, but the 16-in. O.C. Wall U-Value column can also be used for above-grade concrete, masonry, and log walls. Mass wall R-value to U-value conversion tables are planned for future versions of the MECcheck Manual. TM

(c) Wall R-values are the sum of the cavity insulation plus insulating sheathing (if used).

Table 3. 16–in. O.C. Metal–Frame Wall U–Values and Equivalent Prescriptive Package Wall R–Values (Use the U–values below for the System Design Method of the Energy Worksheet. Use the equivalent R–value below to choose an Energy Worksheet Prescriptive Package with a wall R–value that is less than or equal to it. If you have an equivalent R–value without an "I" listed after it, then you must use a Package wall R–value without an "I" designation.)

Cavity		·		I	nsulatin	g Sheath	ing R-Va	lue		· · · · · .	
R-Value						-	_				
	R-0	R-1	R-2	R-3	R-4	R-5	R-6	R-7	R8	R-9	R-10
R0	U-0.270	U0.258	U-0.205	U-0.170	U-0.146	U-0.127	U-0.113	U-0.101	U-0.092	U-0.084	U0.078
R-11	U-0.120	U-0.118	U-0.106	U-0.096	U-0.087	U-0.080	U-0.074	U-0.069	U-0.065	U-0.061	U-0.057
		an an thur					R15	R15I	R16I	R18I	R20I
R-13	U-0.114	U-0.111	U-0.100	U-0.091	U-0.084	U0.077	U-0.072	U-0.067	U-0.063	U-0.059	U-0.056
2 10						R15	R15	R15I	R17I	R19I	R22I
R-15	U-0.109	U-0.107	U-0.096	U-0.088	U-0.081	U-0.075	U-0.070	U-0.065	U-0.061	U-0.058	U-0.054
	-					R15	R15	R16I	R18I	R19I	R22I
R-19	U-0.101	U-0.099	U-0.090	U-0.083	U-0.077	U-0.071	U-0.066	U-0.062	U-0.059	U-0.055	U-0.052
					R15	R15	R15I	R17I	R19I	R20I	R22I
R-21	U-0.098	U-0.096	U-0.088	U-0.081	U-0.075	U-0.070	U-0.065	U-0.061	U-0.058	U-0.054	U-0.052
a de la composición d				R13	R15	R15	R16I	R18I	R19I	R20I	R22I
R-25	U-0.094	U-0.093	U-0.085	U0.078	U-0.073	U-0.068	U-0.063	U-0.060	U-0.056	U-0.053	U-0.051
а. 				R13	R15	R151	R17I	R19I	R20I	R20I	R23I

Table 4. 24–in. O.C. Metal–Frame Wall U–Values and Equivalent Prescriptive Package Wall R–Values (Use the U–values below for the System Design Method of the Energy Worksheet. Use the equivalent R–value below to choose an Energy Worksheet Prescriptive Package with a wall R–value that is less than or equal to it. If you have an equivalent R–value without an "I" listed after it, then you must use a Package wall R–value without an "I" designation.)

Cavity R–Value		Insulating Sheathing R-Value									
	R-0	R-1	R-2	R-3	R-4	R-5	R6	R –7	R-8	R-9	R-10
R0	U0.270	U-0.258	U0.205	U-0.170	U-0.146	U-0.127	U-0.113	U0.101	U-0.092	U0.084	U-0.078 R13
R–11	U-0.106	U0.104	U–0.095	U-0.086	U-0.080 R13	U-0.074 R15	U-0.069 R15I	U-0.064 R17I	U-0.060 R18I	U0.057 R20I	U-0.054 R20I
R-13	U–0.100	U-0.098	U-0.090	U-0.082 R13	U–0.076 R15	U–0.071 R15	U-0.066 R15I	U-0.062 R17I	U-0.058 R19I	U-0.055 R20I	U-0.052 R22I
R–15	U-0.094	U-0.093	U0.085	U-0.078 R13	U–0.073 R15	U-0.068 R15I	U-0.063 R17I	U-0.060 R19I	U-0.056 R20I	U-0.053 R20I	U-0.051 R23I
R-19	U-0.088	U-0.086	U-0.080 R13	Ŭ0.074 R15	U-0.069 R15I	U-0.064 R17I	U-0.060 R19I	U-0.057 R20I	U0.054 R20I	U–0.051 R23I	U-0.049 R24I
R–21	U-0-085	U-0.084	U-0.077 R15	U–0.072 R15	U-0.067 R15I	U-0.063 R17I	U0.059 R19I	U-0.056 R20I	U-0.053 R20I	U-0.050 R23I	U-0.048 R24I
R-25	U-0.081 R13	U-0.080 R13	U-0-074 R15	U-0.069 R15	U-0.064 R17I	U-0.060 R19I	U0.057 R20I	U-0.054 R20I	U-0.051 R23I	U-0.049 R23I	U0.046 R24I

Insulation R-Value	Floor U-Value
R-0	0.249
R–7	0.096
R-11	0.072
R-13	0.064
R-15	0.057
R-19	0.047
R-21	0.044
R-26	0.037
R-30	0.033

Table 5. Floor U-Values

Table 6. Basement U-Values^(a)

Insulation	Basement Wall	Insulation	Basement Wall
R-Value	U–Value	R–Value	U–Value
R-0	0.360	R-10	0.072
R-1	0.244	R-11	0.067
R-2	0.188	R-12	0.062
R-3	0.155	R-13	0.059
R-4	0.132	R-14	0.055
R–5	0.115	R-15	0.052
R-6	0.102	R-16	0.050
R–7	0.092	R–17	0.047
R8	0.084	R-18	0.045
R-9	0.077	R-19	0.043
		R-20	0.041

(a) Insulation R-values represent the sum of exterior and/or interior insulation. Basement walls must be insulated from the top of the basement wall to 10 ft below ground level or to the floor of the basement, whichever is less.

Perimeter Insulation	Slab F	–Value
R–Value		
	24-in. Insulation Depth	48-in. Insulation Depth
R-0	1.04	1.04
R-1	0.91	0.89
R-2	0.86	0.83
R-3	0.83	0.79
R4	0.82	0.76
R-5	0.80	0.74
R-6	0.79	0.73
R-7	0.79	0.71
R-8	0.78	0.70
R-9	0.77	0.69
R-10	0.77	0.68
R-11		0.68
R-12		0.67
R-13		0.66
R-14		0.66
R-15		0.65
R-16		0.65
R-17		0.65
R-18	· · · · · · · · · · · · · · · · · · ·	0.64
R-19		0.64
R-20		0.64

Table 7. Slab F-Values

Insulation R-Value	Crawl Space Wall U-Value
R-0	0.477
R-1	0.313
R–2	0.235
R-3	0.189
R4	0.158
R-5	0.136
R-6	0.120
R–7	0.107
R-8	0.096
R–9	0.088
R-10	0.081
R-11	0.075
R-12	0.069
R-13	0.065
R-14	0.061
R-15	0.057
R-16	0.054
R-17	0.051
R-18	0.049
R-19	0.047
R-20	0.045

Table 8. Crawl Space Wall U-Values

Frame/Glazing Features	Single Pane	Double Pane	
Metal Without Thermal Break			
Operable	1.27	0.87	
Fixed	1.13	0.69	
Garden Window	2.60	1.81	
Curtain Wall	1.22	0.79	
Door	1.26	0.80	
Skylight	1.98	1.31	
Site-Assembled Skylight	1.36	0.82	
Metal With Thermal Break	······································		
Operable	1.08	0.65	
Fixed	1.07	0.63	
Curtain Wall	1.11	0.68	
Door	1.10	0.66	
Skylight	1.89	1.11	
Site-Assembled Skylight	1.25	0.70	
Reinforced Vinyl or Metal-Clad Wood			
Operable	0.90	0.57	
Fixed	0.98	0.56	
Door	0.99	0.57	
Skylight	1.75	1.05	
Wood/Vinyl/Fiberglass			
Operable	0.89	0.55	
Fixed	0.98	0.56	
Garden Window	2.31	1.61	
Door	0.98	0.56	
Skylight	1.47	0.84	
Glass Block Assemblies		0.60	

Table 9. U-Values for Windows, Glazed Doors, and Skylights^(a)

(a) The U-values in these tables can be used in the absence of test U-values. The product cannot receive credit for a feature that cannot be clearly detected. Where a composite of materials from two different product types is used, the product must be assigned the higher U-value.

Steel Doors	•			
Without Foam Core	0.60 0.35			
With Foam Core				
Wood Doors	Without Storm	With Storm		
Panel With 7/16-in. Panels	0.54	0.36		
Hollow Core Flush	0.46	0.32		
Panel With 1–1/8–in. Panels	0.39	0.28		
Solid Core Flush	0.40	0.26		

(a) The U-values in these tables can be used in the absence of test U-values. The product cannot receive credit for a feature that cannot be clearly detected. Where a composite of materials from two different product types is used, the product must be assigned the higher U-value.

Resistance (R)

DEPARTMENT OF COMMERCE

Description Per Inch For Thickness Density, lb/ft³ Thickness Listed °F.ft².h SHEATHING Gypsum or plaster board 1/2" 50 0.45 Gypsum or plaster board 5/8" 50 0.56 Plywood (Douglas Fir) 1/2" 34 0.62 Plywood (Douglas Fir) 5/8" 34 0.77 Plywood or wood panels 3/4". 34 0.93 Vegetable fiber board Sheathing, regular density 1/2" 18 1.32 Hardboard Medium density 1 37 50 Particleboard Medium density 50 1.06 FINISH FLOORING MATERIALS Carpet and rubber pad 1.23 ____ ____ INSULATING MATERIALS Blanket and Batt Mineral fiber, fibrous form processed from rock, slag, or glass approx. 3–4 in 04 - 2011 approx. 3.5 in 0.4-2.0 13 approx. 3.5 in 15 12 - 16approx. 5 5-6.5 in 19 04-20 approx. 5.5 in. 06-10 21 approx 6-7.5 in 0.4-2.0 22 approx 8.25-10 in 0.4-2.0 30 approx 10–13 in 0.4-2.0 38 Board and Slabs Glass fiber, organic bonded 4.00 40-90 Expanded polystyrene, extruded (smooth skin surface) 1.8-3.5 5.00 Expanded polystyrene, molded beads 10 3.85 1.25 4.00 1.5 4.17 175 417 2.0 4.35 Cellular polyurethane/polyisocyanurate 6.25-5.56 1.5 Cellular polyisocyanurate (CFC-11 exp.) (gas-impermeable facers) 2.0 7.04 Mineral fiberboard, wet felted Acoustical tile 18.0 2.86 Loose Fill Cellulosic insulation (milled paper or wood pulp) 3.70-3.13 23-32 Perlite, expanded 2.0 - 4.13.7-3.3 4.1-7.4 3.3 - 2.87.4-11.0 2.8 - 2.4Mineral fiber (rock, slag, or glass) approx. 3.75-5 in 0.6-2.0 11.0 approx. 6.5-8.75 in 0.6-2.0 19.0 approx. 7.5–10 in. 22.0 0.6-2.0 approx. 10.25-13.75 in 0.6-2.0 30.0 Mineral fiber (rock, slag, or glass) approx. 3.5 in. (closed sidewall application) 2.0-3.5 12.0-14.0 Vermiculite, exfoliated 2.13 7.0-8.2 4.0-6.0 2.27 Spray Applied Polyurethane foam 1.5-2.5 6.25-5.56 Ureaformaldehyde foam 0.7-1.6 4.55-3.57

Typical Thermal Properties of Building Materials-Design Values^a

Cellulosic fiber

Glass fiber

3.5-6.0

3.5-4.5

3.45-2.94

3.85-3.70

ROOFING			
Asphalt shingles	70		0.44
PLASTERING MATERIALS			
Cement plaster, sand aggregate	116	0.20	·
0.75 in.			0.15
MASONRY MATERIALS			
Masonry Units			
Brick, fired clay	150	0.12-0.10	
Concrete blocks			
Normal weight aggregate (sand and gravel)			
8 in., 33-36 lb, 126-136 lb/ft ³ concrete, 2 or 3 cores		· <u> </u>	1.11-0.97
Same with perlite filled cores	—		2.0
Same with vermiculite filled cores			1.92-1.37
12 in., 50 lb, 125 lb/ft ³ concrete, 2 cores		—	1 23
Concretes			
Sand and gravel or stone aggregate concretes	150	0.10	—
SIDING MATERIALS (on flat surface)			
Siding			
Asphalt roll siding	··		0.15
Hardboard siding, 7/16"		· · · ·	0.67
Wood, drop, 1 by 8 in		<u> </u>	0.79
Aluminum, steel, or vinyl, over sheathing			
Hollow-backed			0.61
Insulating-board backed nominal 3/8"	—	·	1.82
Insulating-board backed nominal 3/8", foil backed	<u> </u>		2.96
WOOD			
Maples, oak and similar materials	45	0.91	
Fir, pine and similar materials	32	1.25	
3/4"	32	0.94	
1-1/2"	32	19	
3-1/2"	32	4.4	
5–1/2"	32	6.9	
7-1/4"	32	9.1	
9–1/4"	32	11.6	
11-1/4"	32	14.1	

^aValues are for a mean temperature of 75°F. Representative values for dry materials are intended as design (not specification) values for materials in normal use. Thermal values of insulating materials may differ from design values depending on their in-situ properties (e.g., density and moisture content, orientation, etc.) and variability experienced during manufacture. For properties of a particular product, use the value supplied by the manufacturer or by unbiased tests in accordance with s. Comm 22 31.



s. Comm 22.26 Slab-On-Grade Insulation Details

Insulation shall extend vertically and horizontally for a total of 48". In all cases the insulation shall insulate to the top edge of the floor perimeter. The last diagram is not an acceptable method.